

Appendix L

**LEVEL OF SERVICE  
CALCULATIONS**



## LEVEL OF SERVICE CALCULATIONS - APPENDIX L

<b>EXISTING CONDITION</b>	<b>Page</b>
Interstate 84 westbound ramp/ Saw Mill Road a.m. peak hour	1
US Route 6 & 202/ Saw Mill Road a.m. peak hour	2
US Route 6 & 202/ Starr Ridge Road a.m. peak hour	3
Argonne Road /Interstate 684 & NYS Route 22 ramps US Route 6 & 202 a.m. peak hour	4
East Main Street/US Route 6 & 202 & 22a.m. peak hour	5
Interstate 84 eastbound ramp/ Saw Mill Road a.m. peak hour	6
Dingle Ridge Road/ US Route 6 & 202 a.m. peak hour	7
Joe's Hill Road/ US Route 6 & 202 a.m. peak hour	8
Peach Lake Road/ US Route 6 & 202 a.m. peak hour	9
Peach Lake Road/ Interstate 84 westbound off-ramp a.m. peak hour	10
Peach Lake Road/ Interstate 84 eastbound on-ramp a.m. peak hour	11
Sodom Road (East)/US Route 6 & 202 & 22 a.m. peak hour	12
Sodom Road (West)/US Route 6 & 202 & 22 a.m. peak hour	13
Interstate 84 westbound ramp/ Saw Mill Road p.m. peak hour	14
US Route 6 & 202/ Saw Mill Road p.m. peak hour	15
US Route 6 & 202/ Starr Ridge Road p.m. peak hour	16
Argonne Road /Interstate 684 / NYS Route 22 ramps US Route 6 & 202 p.m. peak hour	17
East Main Street/US Route 6 & 202 & 22p.m. peak hour	18
Interstate 84 eastbound ramp/ Saw Mill Road p.m. peak hour	19
Dingle Ridge Road/ US Route 6 & 202 p.m. peak hour	20
Joe's Hill Road/ US Route 6 & 202 p.m. peak hour	21
Peach Lake Road/ US Route 6 & 202 p.m. peak hour	22
Peach Lake Road/ Interstate 84 westbound off-ramp p.m. peak hour	23
Peach Lake Road/ Interstate 84 eastbound on-ramp p.m. peak hour	24
Sodom Road (East)/US Route 6 & 202 & 22 p.m. peak hour	25
Sodom Road (West)/US Route 6 & 202 & 22 p.m. peak hour	26
Interstate 84 westbound ramp/ Saw Mill Road Saturday peak hour	27
US Route 6 & 202/ Saw Mill Road Saturday peak hour	28
US Route 6 & 202/ Starr Ridge Road Saturday peak hour	29
Argonne Road /Interstate 684 / NYS Route 22 ramps US Route 6 & 202 Saturday peak hour	30
East Main Street/US Route 6 & 202 & 22Saturday peak hour	31
Interstate 84 eastbound ramp/ Saw Mill Road Saturday peak hour	32
Dingle Ridge Road/ US Route 6 & 202 Saturday peak hour	33
Joe's Hill Road/ US Route 6 & 202 Saturday peak hour	34
Peach Lake Road/ US Route 6 & 202 Saturday peak hour	35
Peach Lake Road/ Interstate 84 westbound off-ramp Saturday peak hour	36
Peach Lake Road/ Interstate 84 eastbound on-ramp Saturday peak hour	37
Sodom Road (East)/US Route 6 & 202 & 22 Saturday peak hour	38
Sodom Road (West)/US Route 6 & 202 & 22 Saturday peak hour	39

**NO BUILD CONDITION****Page**

Interstate 84 westbound ramp/ Saw Mill Road a.m. peak hour	40
US Route 6 & 202/ Saw Mill Road a.m. peak hour	41
US Route 6 & 202/ Starr Ridge Road a.m. peak hour	42
Argonne Road /Interstate 684 & NYS Route 22 ramps US Route 6 & 202 a.m. peak hour	43
East Main Street/US Route 6 & 202 & 22a.m. peak hour	44
Interstate 84 eastbound ramp/ Saw Mill Road a.m. peak hour	45
Dingle Ridge Road/ US Route 6 & 202 a.m. peak hour	46
Joe's Hill Road/ US Route 6 & 202 a.m. peak hour	47
Peach Lake Road/ US Route 6 & 202 a.m. peak hour	48
Peach Lake Road/ Interstate 84 westbound off-ramp a.m. peak hour	49
Peach Lake Road/ Interstate 84 eastbound on-ramp a.m. peak hour	50
Sodom Road (East)/US Route 6 & 202 & 22 a.m. peak hour	51
Sodom Road (West)/US Route 6 & 202 & 22 a.m. peak hour	52
Interstate 84 westbound ramp/ Saw Mill Road p.m. peak hour	53
US Route 6 & 202/ Saw Mill Road p.m. peak hour	54
US Route 6 & 202/ Starr Ridge Road p.m. peak hour	55
Argonne Road /Interstate 684 / NYS Route 22 ramps US Route 6 & 202 p.m. peak hour	56
East Main Street/US Route 6 & 202 & 22p.m. peak hour	57
Interstate 84 eastbound ramp/ Saw Mill Road p.m. peak hour	58
Dingle Ridge Road/ US Route 6 & 202 p.m. peak hour	59
Joe's Hill Road/ US Route 6 & 202 p.m. peak hour	60
Peach Lake Road/ US Route 6 & 202 p.m. peak hour	61
Peach Lake Road/ Interstate 84 westbound off-ramp p.m. peak hour	62
Peach Lake Road/ Interstate 84 eastbound on-ramp p.m. peak hour	63
Sodom Road (East)/US Route 6 & 202 & 22 p.m. peak hour	64
Sodom Road (West)/US Route 6 & 202 & 22 p.m. peak hour	65
Interstate 84 westbound ramp/ Saw Mill Road Saturday peak hour	66
US Route 6 & 202/ Saw Mill Road Saturday peak hour	67
US Route 6 & 202/ Starr Ridge Road Saturday peak hour	68
Argonne Road /Interstate 684 / NYS Route 22 ramps US Route 6 & 202 Saturday peak hour	69
East Main Street/US Route 6 & 202 & 22Saturday peak hour	70
Interstate 84 eastbound ramp/ Saw Mill Road Saturday peak hour	71
Dingle Ridge Road/ US Route 6 & 202 Saturday peak hour	72
Joe's Hill Road/ US Route 6 & 202 Saturday peak hour	73
Peach Lake Road/ US Route 6 & 202 Saturday peak hour	74
Peach Lake Road/ Interstate 84 westbound off-ramp Saturday peak hour	75
Peach Lake Road/ Interstate 84 eastbound on-ramp Saturday peak hour	76
Sodom Road (East)/US Route 6 & 202 & 22 Saturday peak hour	77
Sodom Road (West)/US Route 6 & 202 & 22 Saturday peak hour	78

**BUILD CONDITION****Page**

Interstate 84 westbound ramp/ Saw Mill Road a.m. peak hour	79
US Route 6 & 202/ Saw Mill Road a.m. peak hour	80
US Route 6 & 202/ Starr Ridge Road a.m. peak hour	81
Argonne Road /Interstate 684 & NYS Route 22 ramps US Route 6 & 202 a.m. peak hour	82
East Main Street/US Route 6 & 202 & 22a.m. peak hour	83
Interstate 84 eastbound ramp/ Saw Mill Road a.m. peak hour	84
Dingle Ridge Road/ US Route 6 & 202 a.m. peak hour	85
Joe's Hill Road/ US Route 6 & 202 a.m. peak hour	86
Peach Lake Road/ US Route 6 & 202 a.m. peak hour	87
Peach Lake Road/ Interstate 84 westbound off-ramp a.m. peak hour	88
Peach Lake Road/ Interstate 84 eastbound on-ramp a.m. peak hour	89
Sodom Road (East)/US Route 6 & 202 & 22 a.m. peak hour	90
Sodom Road (West)/US Route 6 & 202 & 22 a.m. peak hour	91
Interstate 84 westbound ramp/ Saw Mill Road p.m. peak hour	92
US Route 6 & 202/ Saw Mill Road p.m. peak hour	93
US Route 6 & 202/ Starr Ridge Road p.m. peak hour	94
Argonne Road /Interstate 684 / NYS Route 22 ramps US Route 6 & 202 p.m. peak hour	95
East Main Street/US Route 6 & 202 & 22p.m. peak hour	96
Interstate 84 eastbound ramp/ Saw Mill Road p.m. peak hour	97
Dingle Ridge Road/ US Route 6 & 202 p.m. peak hour	98
Joe's Hill Road/ US Route 6 & 202 p.m. peak hour	99
Peach Lake Road/ US Route 6 & 202 p.m. peak hour	100
Peach Lake Road/ Interstate 84 westbound off-ramp p.m. peak hour	101
Peach Lake Road/ Interstate 84 eastbound on-ramp p.m. peak hour	102
Sodom Road (East)/US Route 6 & 202 & 22 p.m. peak hour	103
Sodom Road (West)/US Route 6 & 202 & 22 p.m. peak hour	104
Interstate 84 westbound ramp/ Saw Mill Road Saturday peak hour	105
US Route 6 & 202/ Saw Mill Road Saturday peak hour	106
US Route 6 & 202/ Starr Ridge Road Saturday peak hour	107
Argonne Road /Interstate 684 / NYS Route 22 ramps US Route 6 & 202 Saturday peak hour	108
East Main Street/US Route 6 & 202 & 22Saturday peak hour	109
Interstate 84 eastbound ramp/ Saw Mill Road Saturday peak hour	110
Dingle Ridge Road/ US Route 6 & 202 Saturday peak hour	111
Joe's Hill Road/ US Route 6 & 202 Saturday peak hour	112
Peach Lake Road/ US Route 6 & 202 Saturday peak hour	113
Peach Lake Road/ Interstate 84 westbound off-ramp Saturday peak hour	114
Peach Lake Road/ Interstate 84 eastbound on-ramp Saturday peak hour	115
Sodom Road (East)/US Route 6 & 202 & 22 Saturday peak hour	116
Sodom Road (West)/US Route 6 & 202 & 22 Saturday peak hour	117

<b>Improvements</b>	<b>Page</b>
Eastern Site Access/US Route 6 & 202 a.m. peak hour	118
Western Site Access/US Route 6 & 202 a.m. peak hour	119
Center Site Access/US Route 6 & 202 a.m. peak hour	120
Eastern Site Access/US Route 6 & 202 p.m. peak hour	121
Western Site Access/US Route 6 & 202 p.m. peak hour	122
Center Site Access/US Route 6 & 202 p.m. peak hour	123
Eastern Site Access/US Route 6 & 202 Saturday peak hour	124
Western Site Access/US Route 6 & 202 Saturday peak hour	125
Center Site Access/US Route 6 & 202 Saturday peak hour	126
Peach Lake Road/ US Route 6 & 202 a.m. peak hour	127
Peach Lake Road/ US Route 6 & 202 p.m. peak hour	128
Peach Lake Road/ US Route 6 & 202 Saturday peak hour	129
Peach Lake Road/ US Route 6 & 202 a.m. peak hour Sensitivity	130

### HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>I-84 WB ramps/Saw Mill</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>11/16/07</i>	Jurisdiction <i>Danbury, CT</i>
Time Period <i>A.M. Peak Hour</i>	Analysis Year <i>Existing Condition</i>
	Project ID <i>Stateline</i>

#### Volume and Timing Input

	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N <sub>1</sub>				0	1	1	0	2			2	0	
Lane Group					LT	R		LT			TR		
Volume, V (vph)				155	5	388	50	139			90	53	
% Heavy Vehicles, %HV				3	3	3	1	1			1	1	
Peak-Hour Factor, PHF				0.92	0.92	0.92	0.86	0.86			0.92	0.92	
Pretimed (P) or Actuated (A)				A	A	A	A	A			A	A	
Start-up Lost Time, I <sub>1</sub>					2.0	2.0		2.0			2.0		
Extension of Effective Green, e					2.0	2.0		2.0			2.0		
Arrival Type, AT					3	3		3			5		
Unit Extension, UE					3.0	3.0		3.0			3.0		
Filtering/Metering, I					1.000	1.000		1.000			1.000		
Initial Unmet Demand, Q <sub>b</sub>					0.0	0.0		0.0			0.0		
Ped / Bike / RTOR Volumes				0	0	230	0	0		0	0	0	
Lane Width					12.0	12.0		12.0			12.0		
Parking / Grade / Parking				N	-1	N	N	1	N	N	-1	N	
Parking Maneuvers, N <sub>m</sub>													
Buses Stopping, N <sub>b</sub>					0	0		0			0		
Min. Time for Pedestrians, G <sub>p</sub>					3.2			3.2			3.2		
Phasing	WB Only	02	03	04	NB Only	SB Only	07	08					
Timing	G = 13.0	G =	G =	G =	G = 12.0	G = 30.0	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =					
Duration of Analysis, T = 0.25						Cycle Length, C = 70.0							

#### Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					173	172		220			156	
Lane Group Capacity, c					328	293		603			1457	
v/c Ratio, X					0.53	0.59		0.36			0.11	
Total Green Ratio, g/C					0.19	0.19		0.17			0.43	
Uniform Delay, d <sub>1</sub>					25.7	26.0		25.6			12.0	
Progression Factor, PF					1.000	1.000		1.000			0.500	
Delay Calibration, k					0.13	0.18		0.11			0.11	
Incremental Delay, d <sub>2</sub>					1.6	3.1		0.4			0.0	
Initial Queue Delay, d <sub>3</sub>					0.0	0.0		0.0			0.0	
Control Delay					27.3	29.1		26.0			6.0	
Lane Group LOS					C	C		C			A	
Approach Delay				28.2			26.0			6.0		
Approach LOS				C			C			A		
Intersection Delay	22.7			X <sub>c</sub> = 0.28			Intersection LOS			C		

<b>HCS+™ DETAILED REPORT</b>												
<b>General Information</b>						<b>Site Information</b>						
Analyst JAG						Intersection US 6 and Saw Mill Rd						
Agency or Co. TMA						Area Type All other areas						
Date Performed 12/6/2006						Jurisdiction Danbury, CT						
Time Period A.M. Peak Hour						Analysis Year Existing Condition						
						Project ID Stateline						
<b>Volume and Timing Input</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>		2	0	1	1		1		1			
Lane Group		TR		L	T		L		R			
Volume, V (vph)		91	98	46	547		502		39			
% Heavy Vehicles, %HV		1	1	0	0		1		1			
Peak-Hour Factor, PHF		0.89	0.89	0.80	0.80		0.87		0.87			
Pretimed (P) or Actuated (A)		A	A	A	A		A		A			
Start-up Lost Time, I <sub>1</sub>		2.0		2.0	2.0		2.0		2.0			
Extension of Effective Green, e		2.0		2.0	2.0		2.0		2.0			
Arrival Type, AT		3		3	3		5		5			
Unit Extension, UE		3.0		3.0	3.0		3.0		3.0			
Filtering/Metering, I		1.000		1.000	1.000		1.000		1.000			
Initial Unmet Demand, Q <sub>b</sub>		0.0		0.0	0.0		0.0		0.0			
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0			
Lane Width		13.0		13.0	14.0		12.0		12.0			
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N			
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0		0	0		0		0			
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2			3.2				
Phasing	EW Perm	WB Only	03		04		NB Only	06		07		08
Timing	G = 20.0	G = 10.0	G =	G =	G = 30.0	G =	G =	G =	G =	G =	G =	G =
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =	Y =	Y =	Y =	Y =
Duration of Analysis, T = 0.25						Cycle Length, C = 75.0						
<b>Lane Group Capacity, Control Delay, and LOS Determination</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		212		57	684		577		45			
Lane Group Capacity, c		910		772	946		715		640			
v/c Ratio, X		0.23		0.07	0.72		0.81		0.07			
Total Green Ratio, g/C		0.27		0.51	0.47		0.40		0.40			
Uniform Delay, d <sub>1</sub>		21.5		9.6	16.1		19.9		13.9			
Progression Factor, PF		1.000		1.000	1.000		0.556		0.556			
Delay Calibration, k		0.11		0.11	0.28		0.35		0.11			
Incremental Delay, d <sub>2</sub>		0.1		0.0	2.8		6.8		0.0			
Initial Queue Delay, d <sub>3</sub>		0.0		0.0	0.0		0.0		0.0			
Control Delay		21.6		9.6	18.9		17.9		7.8			
Lane Group LOS		C		A	B		B		A			
Approach Delay	21.6			18.1			17.2					
Approach LOS	C			B			B					
Intersection Delay	18.2			X <sub>c</sub> = 0.76			Intersection LOS			B		



SHORT REPORT												
General Information						Site Information						
Analyst	JAG					Intersection	US 6/ Starr					
Agency or Co.	TMA					Area Type	All other areas					
Date Performed	12/7/2006					Jurisdiction	Town of Southeast					
Time Period	A.M. Peak Hour					Analysis Year	Existing Condition					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes	0	2	0	0	2	0	0	1	0	1	1	0
Lane Group	DefL	TR			LTR			LTR		L	TR	
Volume (vph)	124	325	107	40	1112	86	88	24	7	117	94	111
% Heavy Vehicles	5	5	5	5	5	5	5	5	5	5	5	5
PHF	0.74	0.74	0.74	0.91	0.91	0.91	0.80	0.80	0.80	0.91	0.91	0.91
Pretimed/Actuated (P/A)	A	A	A	A	A	A	A	A	A	A	A	A
Startup Lost Time	2.0	2.0			2.0			2.0		2.0	2.0	
Extension of Effective Green	2.0	2.0			2.0			2.0		2.0	2.0	
Arrival Type	3	3			3			3		3	3	
Unit Extension	3.0	3.0			3.0			3.0		3.0	3.0	
Ped/Bike/RTOR Volume	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	12.0	12.0			12.0			15.0		10.0	9.0	
Parking/Grade/Parking	N	5	N	N	-2	N	N	-6	N	N	-5	N
Parking/Hour												
Bus Stops/Hour	0	0			0			0		0	0	
Minimum Pedestrian Time		3.2			3.2			3.2			3.2	
Phasing	EW Perm	EB Only	03	04	NS Perm	06	07	08				
Timing	G = 30.0	G = 4.0	G =	G =	G = 16.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis (hrs) = 0.25						Cycle Length C = 65.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate	168	584			1361			149		129	225	
Lane Group Capacity	420	1019			1461			209		298	378	
v/c Ratio	0.40	0.57			0.93			0.71		0.43	0.60	
Green Ratio	0.65	0.60			0.46			0.25		0.25	0.25	
Uniform Delay $d_1$	17.9	7.9			16.5			22.4		20.7	21.6	
Delay Factor k	0.11	0.17			0.45			0.28		0.11	0.18	
Incremental Delay $d_2$	0.6	0.8			11.0			10.9		1.0	2.5	
PF Factor	1.000	1.000			1.000			1.000		1.000	1.000	
Control Delay	18.5	8.7			27.6			33.3		21.7	24.2	
Lane Group LOS	B	A			C			C		C	C	
Approach Delay	10.9			27.6			33.3			23.3		
Approach LOS	B			C			C			C		
Intersection Delay	22.5			Intersection LOS						C		

## SHORT REPORT

General Information	Site Information
Analyst <i>JAG</i> Agency or Co. <i>TMA</i> Date Performed <i>12/28/2006</i> Time Period <i>A.M. Peak Hour</i>	Intersection <i>Argonne /Rt 22 ramps/US 6</i> Area Type <i>All other areas</i> Jurisdiction <i>Town of Southeast, NY</i> Analysis Year <i>Existing Condition</i>

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes	0	1	0	1	1	0	0	2	0	0	2	0
Lane Group		<i>LTR</i>		<i>L</i>	<i>TR</i>			<i>LTR</i>		<i>DefL</i>	<i>TR</i>	
Volume (vph)	2	78	100	201	6	115	17	740	556	168	265	3
% Heavy Vehicles	5	5	5	5	5	5	5	5	5	5	5	5
PHF	0.74	0.74	0.74	0.85	0.85	0.85	0.97	0.97	0.97	0.88	0.88	0.88
Pretimed/Actuated (P/A)	<i>A</i>	<i>A</i>	<i>A</i>	<i>A</i>	<i>A</i>	<i>A</i>	<i>A</i>	<i>A</i>	<i>A</i>	<i>A</i>	<i>A</i>	<i>A</i>
Startup Lost Time		2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green		2.0		2.0	2.0			2.0		2.0	2.0	
Arrival Type		3		3	3			3		3	3	
Unit Extension		3.0		3.0	3.0			3.0		3.0	3.0	
Ped/Bike/RTOR Volume	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.0		9.0	9.0			12.0		12.0	12.0	
Parking/Grade/Parking	<i>N</i>	0	<i>N</i>	<i>N</i>	-6	<i>N</i>	<i>N</i>	-4	<i>N</i>	<i>N</i>	4	<i>N</i>
Parking/Hour												
Bus Stops/Hour		0		0	0			0		0	0	
Minimum Pedestrian Time		3.2			3.2			3.2			3.2	
Phasing	EW Perm	WB Only	03	04	NS Perm	SB Only	07	08				
Timing	G = 13.0	G = 2.0	G =	G =	G = 33.0	G = 2.0	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis (hrs) = 0.25							Cycle Length C = 70.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate		243		236	142			1354		191	304	
Lane Group Capacity		341		350	411			1472		344	1012	
v/c Ratio		0.71		0.67	0.35			0.92		0.56	0.30	
Green Ratio		0.19		0.33	0.29			0.47		0.61	0.57	
Uniform Delay $d_1$		26.7		25.1	19.8			17.3		22.5	7.8	
Delay Factor k		0.28		0.25	0.11			0.44		0.15	0.11	
Incremental Delay $d_2$		6.9		5.1	0.5			9.7		2.0	0.2	
PF Factor		1.000		1.000	1.000			1.000		1.000	1.000	
Control Delay		33.6		30.2	20.3			26.9		24.5	7.9	
Lane Group LOS		<i>C</i>		<i>C</i>	<i>C</i>			<i>C</i>		<i>C</i>	<i>A</i>	
Approach Delay		33.6		26.5				26.9		14.3		
Approach LOS		<i>C</i>		<i>C</i>				<i>C</i>		<i>B</i>		
Intersection Delay		25.0		Intersection LOS							<i>C</i>	

<b>HCS+™ DETAILED REPORT</b>												
<b>General Information</b>						<b>Site Information</b>						
Analyst <i>WFB + JAG</i>						Intersection <i>Rt 6 (East Main) &amp; 202/22</i>						
Agency or Co. <i>TMA</i>						Area Type <i>All other areas</i>						
Date Performed <i>10/30/2007</i>						Jurisdiction <i>Town of Southeast</i>						
Time Period <i>AM Peak Hour</i>						Analysis Year <i>Existing Condition</i>						
						Project ID <i>Stateline</i>						
<b>Volume and Timing Input</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>	0	1			1	1				1		1
Lane Group		LT			T	R				L		R
Volume, V (vph)	15	162			490	409				494		112
% Heavy Vehicles, %HV	22	22			5	5				7		7
Peak-Hour Factor, PHF	0.75	0.75			0.93	0.93				0.77		0.77
Pretimed (P) or Actuated (A)	A	A			A	A				A		A
Start-up Lost Time, I <sub>1</sub>		2.0			2.0	2.0				2.0		2.0
Extension of Effective Green, e		2.0			2.0	2.0				2.0		2.0
Arrival Type, AT		3			3	3				3		3
Unit Extension, UE		3.0			3.0	3.0				3.0		3.0
Filtering/Metering, I		1.000			1.000	1.000				1.000		1.000
Initial Unmet Demand, Q <sub>b</sub>		0.0			0.0	0.0				0.0		0.0
Ped / Bike / RTOR Volumes	0	0		0	0	0				0	0	0
Lane Width		13.0			11.0	11.0				12.0		11.0
Parking / Grade / Parking	N	0	N	N	0	N				N	0	N
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0			0	0				0		0
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2					3.2		
Phasing	EW Perm	02	03	04	SB Only	06	07	08				
Timing	G = 39.0	G =	G =	G =	G = 36.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 85.0						
<b>Lane Group Capacity, Control Delay, and LOS Determination</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		236			527	440				642		145
Lane Group Capacity, c		698			802	1487				714		618
v/c Ratio, X		0.34			0.66	0.30				0.90		0.23
Total Green Ratio, g/C		0.46			0.46	1.00				0.42		0.42
Uniform Delay, d <sub>1</sub>		14.7			17.8	0.0				22.8		15.7
Progression Factor, PF		1.000			1.000	0.950				1.000		1.000
Delay Calibration, k		0.11			0.23	0.11				0.42		0.11
Incremental Delay, d <sub>2</sub>		0.3			2.0	0.1				14.4		0.2
Initial Queue Delay, d <sub>3</sub>		0.0			0.0	0.0				0.0		0.0
Control Delay		15.0			19.8	0.1				37.2		15.9
Lane Group LOS		B			B	A				D		B
Approach Delay	15.0			10.8						33.3		
Approach LOS	B			B						C		
Intersection Delay	20.2			X <sub>c</sub> = 0.77			Intersection LOS			C		

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	JAG		Intersection	I-84 EB ramps/ Saw Mill Rd				
Agency/Co.	TMA		Jurisdiction	City of Danbury CT				
Date Performed	12/6/2007		Analysis Year	Existing Condition				
Analysis Time Period	A.M. Peak Hour							
Project Description								Stateline
East/West Street: I-84 Eastbound Ramps				North/South Street: Saw Mill Road				
Intersection Orientation: North-South				Study Period (hrs): 0.25				
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		116	47	74	169			
Peak-Hour Factor, PHF	1.00	0.89	0.89	0.96	0.96	1.00		
Hourly Flow Rate, HFR (veh/h)	0	130	52	77	176	0		
Percent Heavy Vehicles	0	--	--	2	--	--		
Median Type	Undivided							
RT Channelized			0				0	
Lanes	0	1	0	0	1	0		
Configuration			TR	LT				
Upstream Signal		0			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	69	0	134					
Peak-Hour Factor, PHF	0.89	0.89	0.89	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	77	0	150	0	0	0		
Percent Heavy Vehicles	1	1	1	0	0	0		
Percent Grade (%)	3			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0				0	
Lanes	0	1	0	0	0	0		
Configuration		LTR						
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT					LTR	
v (veh/h)		77					227	
C (m) (veh/h)		1393					703	
v/c		0.06					0.32	
95% queue length		0.18					1.40	
Control Delay (s/veh)		7.7					12.5	
LOS		A					B	
Approach Delay (s/veh)	--	--					12.5	
Approach LOS	--	--					B	

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	Dingle/ US Route 6 & 202
Agency/Co.	TMA	Jurisdiction	Town of Southeast, NY
Date Performed	12/28/2006	Analysis Year	Existing Condition
Analysis Time Period	A.M. Peak Hour		

Project Description *Stateline*

East/West Street: *US Route 6 & 202 (Danbury Rd)*

North/South Street: *Dingle Ridge Road*

Intersection Orientation: *East-West*

Study Period (hrs): *0.25*

### Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	0	172	18	13	1050	2
Peak-Hour Factor, PHF	0.75	0.75	0.75	0.92	0.92	0.92
Hourly Flow Rate, HFR (veh/h)	0	229	24	14	1141	2
Percent Heavy Vehicles	5	--	--	1	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	2	0	0	2	0
Configuration	<i>LT</i>		<i>TR</i>	<i>LT</i>		<i>TR</i>
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)	36	2	18	0	2	1
Peak-Hour Factor, PHF	0.78	0.78	0.78	0.38	0.38	0.38
Hourly Flow Rate, HFR (veh/h)	46	2	23	0	5	2
Percent Heavy Vehicles	3	3	3	3	3	3
Percent Grade (%)	2			2		
Flared Approach		Y			N	
Storage		1			0	
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration		<i>LTR</i>			<i>LTR</i>	

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>	<i>LT</i>		<i>LTR</i>			<i>LTR</i>	
v (veh/h)	0	14		71			7	
C (m) (veh/h)	590	1317		349			165	
v/c	0.00	0.01		0.20			0.04	
95% queue length	0.00	0.03		0.75			0.13	
Control Delay (s/veh)	11.1	7.8		19.3			27.8	
LOS	<i>B</i>	<i>A</i>		<i>C</i>			<i>D</i>	
Approach Delay (s/veh)	--	--	19.3			27.8		
Approach LOS	--	--	<i>C</i>			<i>D</i>		

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	Joe's Hill Road
Agency/Co.	TMA	Jurisdiction	Town of Southeast, NY
Date Performed	12/4/2006	Analysis Year	Existing Condition
Analysis Time Period	A.M. Peak Hour		

Project Description *Stateline*

East/West Street: *US Route 6 & 202*

North/South Street: *Joe's Hill Road*

Intersection Orientation: *East-West*

Study Period (hrs): *0.25*

### Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	13	180			1060	7
Peak-Hour Factor, PHF	0.73	0.73	1.00	1.00	0.91	0.91
Hourly Flow Rate, HFR (veh/h)	17	246	0	0	1164	7
Percent Heavy Vehicles	5	--	--	0	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	2	0	0	2	0
Configuration	<i>LT</i>	<i>T</i>			<i>T</i>	<i>TR</i>
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)				20		52
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.86	1.00	0.86
Hourly Flow Rate, HFR (veh/h)	0	0	0	23	0	60
Percent Heavy Vehicles	0	0	0	3	0	3
Percent Grade (%)	0			2		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					<i>LR</i>	

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>						<i>LR</i>	
v (veh/h)	17						83	
C (m) (veh/h)	576						281	
v/c	0.03						0.30	
95% queue length	0.09						1.20	
Control Delay (s/veh)	11.4						23.1	
LOS	<i>B</i>						<i>C</i>	
Approach Delay (s/veh)	--	--					23.1	
Approach LOS	--	--					<i>C</i>	

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	US Rte 6/NYS 121
Agency/Co.	TMA	Jurisdiction	Town of Southeast, NY
Date Performed	12/4/2006	Analysis Year	Existing Condition
Analysis Time Period	A.M. Peak Hour		

Project Description *Stateline*East/West Street: *US Route 6 & 202*North/South Street: *NYS Rte 121 Peach Lake*Intersection Orientation: *East-West*Study Period (hrs): *0.25*

### Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)		193	267	233	901	
Peak-Hour Factor, PHF	1.00	0.75	0.75	0.92	0.92	1.00
Hourly Flow Rate, HFR (veh/h)	0	257	356	253	979	0
Percent Heavy Vehicles	0	--	--	5	--	--
Median Type	<i>Undivided</i>					
RT Channelized			1			0
Lanes	0	2	1	0	2	0
Configuration		T	R	LT	T	
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)	313		12			
Peak-Hour Factor, PHF	0.86	1.00	0.86	1.00	1.00	1.00
Hourly Flow Rate, HFR (veh/h)	363	0	13	0	0	0
Percent Heavy Vehicles	4	0	4	0	0	0
Percent Grade (%)	-4			0		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	1	0	1	0	0	0
Configuration	L		R			

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT	L		R			
v (veh/h)		253	363		13			
C (m) (veh/h)		1283	130		892			
v/c		0.20	2.79		0.01			
95% queue length		0.73	33.22		0.04			
Control Delay (s/veh)		8.5	880.3		9.1			
LOS		A	F		A			
Approach Delay (s/veh)	--	--	850.2					
Approach LOS	--	--	F					

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	Route 121/ I-84 off-ramp
Agency/Co.	TMA	Jurisdiction	Town of Southeast
Date Performed	12/6/2006	Analysis Year	Existing Condition
Analysis Time Period	A.M. Peak Hour		

Project Description: <i>Stateline</i>	
East/West Street: <i>I-84 off-ramp</i>	North/South Street: <i>Peach Lake Rd (NYS Rt 121)</i>
Intersection Orientation: <i>North-South</i>	Study Period (hrs): <i>0.25</i>

Vehicle Volumes and Adjustments						
Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)		109			535	
Peak-Hour Factor, PHF	1.00	0.88	1.00	1.00	0.72	1.00
Hourly Flow Rate, HFR (veh/h)	0	123	0	0	743	0
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration		<i>T</i>			<i>T</i>	
Upstream Signal		0			0	

Minor Street	Eastbound			Westbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)				413		194
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.87	1.00	0.87
Hourly Flow Rate, HFR (veh/h)	0	0	0	474	0	222
Percent Heavy Vehicles	0	0	0	5	0	5
Percent Grade (%)	0			-3		
Flared Approach		<i>N</i>			<i>Y</i>	
Storage		0			3	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					<i>LR</i>	

Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration				<i>LR</i>				
v (veh/h)				696				
C (m) (veh/h)				410				
v/c				1.70				
95% queue length				41.97				
Control Delay (s/veh)				347.8				
LOS				<i>F</i>				
Approach Delay (s/veh)	--	--	347.8					
Approach LOS	--	--	<i>F</i>					



## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	NYS Route 121/ I-84 on-ramp
Agency/Co.	TMA	Jurisdiction	Town of Southeast
Date Performed	12/6/2006	Analysis Year	Existing Condition
Analysis Time Period	A.M. Peak Hour		
Project Description <i>Stateline</i>			
East/West Street: <i>I-84 on-ramp</i>		North/South Street: <i>NYS Rt. 121 Peach Lake Rd.</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

### Vehicle Volumes and Adjustments

Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)		109	58	85	859	
Peak-Hour Factor, PHF	1.00	0.91	0.91	0.89	0.89	1.00
Hourly Flow Rate, HFR (veh/h)	0	119	63	95	965	0
Percent Heavy Vehicles	0	--	--	3	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration			<i>TR</i>	<i>LT</i>		
Upstream Signal		0			0	

Minor Street	Eastbound			Westbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)						
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration						

### Delay, Queue Length, and Level of Service

Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		<i>LT</i>						
v (veh/h)		95						
C (m) (veh/h)		1387						
v/c		0.07						
95% queue length		0.22						
Control Delay (s/veh)		7.8						
LOS		<i>A</i>						
Approach Delay (s/veh)	--	--						
Approach LOS	--	--						

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	WFB + JAG		Intersection	Route 6 & Sodom				
Agency/Co.	TMA		Jurisdiction	Town of Southeast				
Date Performed	10/30/2007		Analysis Year	Existing Condition				
Analysis Time Period	A.M. Peak Hour							
Project Description <i>Stateline</i>								
East/West Street: <i>US Route 6/202/22</i>			North/South Street: <i>Sodom Road/Driveway</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	0	275	4	1	819	18		
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.95	0.95	0.95		
Hourly Flow Rate, HFR (veh/h)	0	323	4	1	862	18		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	2	0	0	1	1		
Configuration	<i>LT</i>		<i>TR</i>	<i>LT</i>		<i>R</i>		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	7	3	13	30	0	5		
Peak-Hour Factor, PHF	0.52	0.52	0.52	0.67	0.67	0.67		
Hourly Flow Rate, HFR (veh/h)	13	5	25	44	0	7		
Percent Heavy Vehicles	9	9	9	14	14	14		
Percent Grade (%)	0			0				
Flared Approach		<i>N</i>			<i>N</i>			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	1	1	0	1	0		
Configuration	<i>LT</i>		<i>R</i>		<i>LTR</i>			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound		Southbound			
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>	<i>LT</i>	<i>LT</i>		<i>R</i>		<i>LTR</i>	
v (veh/h)	0	1	18		25		51	
C (m) (veh/h)	777	1244	139		830		173	
v/c	0.00	0.00	0.13		0.03		0.29	
95% queue length	0.00	0.00	0.43		0.09		1.17	
Control Delay (s/veh)	9.6	7.9	34.7		9.5		34.3	
LOS	<i>A</i>	<i>A</i>	<i>D</i>		<i>A</i>		<i>D</i>	
Approach Delay (s/veh)	--	--	20.0			34.3		
Approach LOS	--	--	<i>C</i>			<i>D</i>		

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	WFB + JAG		Intersection	US Route 6 and CR 50				
Agency/Co.	TMA		Jurisdiction	Town of Southeast				
Date Performed	10/30/2007		Analysis Year	Existing Condition				
Analysis Time Period	A.M. Peak Hour							
Project Description <i>Stateline Retail Center</i>								
East/West Street: <i>US Route 6/202/22</i>			North/South Street: <i>County Road 50</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	65	273			831	0		
Peak-Hour Factor, PHF	0.86	0.86	1.00	1.00	0.97	0.97		
Hourly Flow Rate, HFR (veh/h)	75	317	0	0	856	0		
Percent Heavy Vehicles	11	--	--	0	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	1	1	0	0	1	0		
Configuration	L	T				TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				2		144		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.87	1.00	0.87		
Hourly Flow Rate, HFR (veh/h)	0	0	0	2	0	165		
Percent Heavy Vehicles	0	0	0	10	0	10		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	1	0	1		
Configuration				L		R		
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L					L		R
v (veh/h)	75					2		165
C (m) (veh/h)	747					149		346
v/c	0.10					0.01		0.48
95% queue length	0.33					0.04		2.47
Control Delay (s/veh)	10.4					29.5		24.5
LOS	B					D		C
Approach Delay (s/veh)	--	--				24.6		
Approach LOS	--	--				C		

### HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>I-84 WB ramps/Saw Mill</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>11/16/07</i>	Jurisdiction <i>Danbury, CT</i>
Time Period <i>P.M. Peak Hour</i>	Analysis Year <i>Existing Condition</i>
	Project ID <i>Stateline</i>

#### Volume and Timing Input

	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N <sub>1</sub>				0	1	1	0	2			2	0	
Lane Group					LT	R	DefL	T			TR		
Volume, V (vph)				17	3	65	107	94			250	68	
% Heavy Vehicles, %HV				1	1	1	1	1			1	1	
Peak-Hour Factor, PHF				0.89	0.89	0.89	0.88	0.88			0.92	0.92	
Pretimed (P) or Actuated (A)				A	A	A	A	A			A	A	
Start-up Lost Time, I <sub>1</sub>					2.0	2.0	2.0	2.0			2.0		
Extension of Effective Green, e					2.0	2.0	2.0	2.0			2.0		
Arrival Type, AT					3	3	3	3			5		
Unit Extension, UE					3.0	3.0	3.0	3.0			3.0		
Filtering/Metering, I					1.000	1.000	1.000	1.000			1.000		
Initial Unmet Demand, Q <sub>b</sub>					0.0	0.0	0.0	0.0			0.0		
Ped / Bike / RTOR Volumes				0	0	35	0	0		0	0	0	
Lane Width					12.0	12.0	12.0	12.0			12.0		
Parking / Grade / Parking				N	-1	N	N	1	N	N	-1	N	
Parking Maneuvers, N <sub>m</sub>													
Buses Stopping, N <sub>b</sub>					0	0	0	0			0		
Min. Time for Pedestrians, G <sub>p</sub>					3.2			3.2			3.2		
Phasing	WB Only	02	03	04	NS Perm	SB Only	07	08					
Timing	G = 13.0	G =	G =	G =	G = 12.0	G = 30.0	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =					
Duration of Analysis, T = 0.25						Cycle Length, C = 70.0							

#### Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v					22	34	122	107			346	
Lane Group Capacity, c					337	298	175	321			2339	
v/c Ratio, X					0.07	0.11	0.70	0.33			0.15	
Total Green Ratio, g/C					0.19	0.19	0.17	0.17			0.67	
Uniform Delay, d <sub>1</sub>					23.5	23.7	27.3	25.5			4.2	
Progression Factor, PF					1.000	1.000	1.000	1.000			0.152	
Delay Calibration, k					0.11	0.11	0.26	0.11			0.11	
Incremental Delay, d <sub>2</sub>					0.1	0.2	11.5	0.6			0.0	
Initial Queue Delay, d <sub>3</sub>					0.0	0.0	0.0	0.0			0.0	
Control Delay					23.6	23.9	38.8	26.1			0.7	
Lane Group LOS					C	C	D	C			A	
Approach Delay				23.8			32.9			0.7		
Approach LOS				C			C			A		
Intersection Delay	14.4			X <sub>c</sub> = 0.14			Intersection LOS			B		

### HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>US 6 and Saw Mill Rd</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>12/6/2006</i>	Jurisdiction <i>Danbury, CT</i>
Time Period <i>P.M. Peak Hour</i>	Analysis Year <i>Existing Condition</i>
	Project ID <i>Stateline</i>

#### Volume and Timing Input

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>		2	0	1	1		1		1			
Lane Group		TR		L	T		L		R			
Volume, V (vph)		334	185	127	212		94		74			
% Heavy Vehicles, %HV		1	1	1	1		1		1			
Peak-Hour Factor, PHF		0.95	0.95	0.87	0.87		0.98		0.98			
Pretimed (P) or Actuated (A)		A	A	A	A		A		A			
Start-up Lost Time, I <sub>1</sub>		2.0		2.0	2.0		2.0		2.0			
Extension of Effective Green, e		2.0		2.0	2.0		2.0		2.0			
Arrival Type, AT		3		3	3		5		3			
Unit Extension, UE		3.0		3.0	3.0		3.0		3.0			
Filtering/Metering, I		1.000		1.000	1.000		1.000		1.000			
Initial Unmet Demand, Q <sub>b</sub>		0.0		0.0	0.0		0.0		0.0			
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0			
Lane Width		13.0		13.0	14.0		12.0		12.0			
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N			
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0		0	0		0		0			
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2			3.2				
Phasing	EW Perm	WB Only	03	04	NB Only	06	07	08				
Timing	G = 20.0	G = 10.0	G =	G =	G = 30.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 75.0					

#### Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		547		146	244		96		76			
Lane Group Capacity, c		934		612	937		715		640			
v/c Ratio, X		0.59		0.24	0.26		0.13		0.12			
Total Green Ratio, g/C		0.27		0.51	0.47		0.40		0.40			
Uniform Delay, d <sub>1</sub>		23.9		14.4	12.1		14.3		14.2			
Progression Factor, PF		1.000		1.000	1.000		0.556		1.000			
Delay Calibration, k		0.18		0.11	0.11		0.11		0.11			
Incremental Delay, d <sub>2</sub>		1.0		0.2	0.1		0.1		0.1			
Initial Queue Delay, d <sub>3</sub>		0.0		0.0	0.0		0.0		0.0			
Control Delay		24.9		14.6	12.3		8.0		14.3			
Lane Group LOS		C		B	B		A		B			
Approach Delay		24.9			13.2			10.8				
Approach LOS		C			B			B				
Intersection Delay		18.6			X <sub>c</sub> = 0.33			Intersection LOS			B	

### HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>US 6/Starr/22 ramp</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>12/29/2006</i>	Jurisdiction <i>Town of Southeast</i>
Time Period <i>PM Peak Hour</i>	Analysis Year <i>Existing Condition</i>
	Project ID <i>Stateline Retail</i>

#### Volume and Timing Input

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>	0	2	0	0	2	0	0	1	0	1	1	0
Lane Group		<i>LTR</i>			<i>LTR</i>			<i>LTR</i>		<i>L</i>	<i>TR</i>	
Volume, V (vph)	149	477	83	18	442	86	122	40	18	223	34	307
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.95	0.95	0.95	0.92	0.92	0.92	0.90	0.90	0.90	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A
Start-up Lost Time, I <sub>1</sub>		2.0			2.0			2.0		2.0	2.0	
Extension of Effective Green, e		2.0			2.0			2.0		2.0	2.0	
Arrival Type, AT		3			3			3		3	3	
Unit Extension, UE		3.0			3.0			3.0		3.0	3.0	
Filtering/Metering, I		1.000			1.000			1.000		1.000	1.000	
Initial Unmet Demand, Q <sub>b</sub>		0.0			0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0			12.0			15.0		10.0	9.0	
Parking / Grade / Parking	<i>N</i>	5	<i>N</i>	<i>N</i>	-2	<i>N</i>	<i>N</i>	-6	<i>N</i>	<i>N</i>	-5	<i>N</i>
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0			0			0		0	0	
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2			3.2		3.2	3.2	
Phasing	EW Perm	EB Only	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G = 5.0	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 65.0					

#### Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		746			593			200		248	379	
Lane Group Capacity, c		1209			995			327		455	577	
v/c Ratio, X		0.62			0.60			0.61		0.55	0.66	
Total Green Ratio, g/C		0.46			0.31			0.38		0.38	0.38	
Uniform Delay, d <sub>1</sub>		13.2			19.1			16.1		15.6	16.5	
Progression Factor, PF		1.000			1.000			1.000		1.000	1.000	
Delay Calibration, k		0.20			0.18			0.20		0.15	0.23	
Incremental Delay, d <sub>2</sub>		1.0			1.0			3.4		1.4	2.7	
Initial Queue Delay, d <sub>3</sub>		0.0			0.0			0.0		0.0	0.0	
Control Delay		14.1			20.1			19.4		16.9	19.2	
Lane Group LOS		<i>B</i>			<i>C</i>			<i>B</i>		<i>B</i>	<i>B</i>	
Approach Delay		14.1			20.1			19.4		16.9	19.2	
Approach LOS		<i>B</i>			<i>C</i>			<i>B</i>		<i>B</i>	<i>B</i>	
Intersection Delay		17.5			X <sub>c</sub> = 0.84			Intersection LOS		<i>B</i>	<i>B</i>	

**HCS+™ DETAILED REPORT**

<b>General Information</b>				<b>Site Information</b>			
Analyst	JAG			Intersection	Argonne /Rt 22 ramps/US 6		
Agency or Co.	TMA			Area Type	All other areas		
Date Performed	11/7/07			Jurisdiction	Town of Southeast, NY		
Time Period	P.M. Peak Hour			Analysis Year	Existing Condition		
				Project ID	Stateline Retail Center		

**Volume and Timing Input**

	EB			WB			NB			SB				
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT		
Number of Lanes, N <sub>l</sub>	0	1	0	1	1	0	0	2	0	0	2	0		
Lane Group		LTR		L	TR			LTR		DefL	TR			
Volume, V (vph)	7	68	256	120	10	161	27	688	195	138	306	2		
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1		
Peak-Hour Factor, PHF	0.93	0.93	0.93	0.89	0.89	0.89	0.94	0.94	0.94	0.97	0.97	0.97		
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A		
Start-up Lost Time, I <sub>1</sub>		2.0		2.0	2.0			2.0		2.0	2.0			
Extension of Effective Green, e		2.0		2.0	2.0			2.0		2.0	2.0			
Arrival Type, AT		3		3	3			3		3	3			
Unit Extension, UE		3.0		3.0	3.0			3.0		3.0	3.0			
Filtering/Metering, I		1.000		1.000	1.000			1.000		1.000	1.000			
Initial Unmet Demand, Q <sub>b</sub>		0.0		0.0	0.0			0.0		0.0	0.0			
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0		
Lane Width		15.0		9.0	9.0			12.0		12.0	12.0			
Parking / Grade / Parking	N	0	N	N	-6	N	N	-4	N	N	4	N		
Parking Maneuvers, N <sub>m</sub>														
Buses Stopping, N <sub>b</sub>		0		0	0			0		0	0			
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2			3.2			3.2			
Phasing	EW Perm	02		03		04		NS Perm	SB Only		07		08	
Timing	G = 20.0	G =		G =		G =		G = 23.0	G = 2.0		G =		G =	
	Y = 5	Y =		Y =		Y =		Y = 5	Y = 5		Y =		Y =	
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0							

**Lane Group Capacity, Control Delay, and LOS Determination**

	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		356		135	192			968		142	317		
Lane Group Capacity, c		614		259	499			1271		415	921		
v/c Ratio, X		0.58		0.52	0.38			0.76		0.34	0.34		
Total Green Ratio, g/C		0.33		0.33	0.33			0.38		0.55	0.50		
Uniform Delay, d <sub>1</sub>		16.5		16.1	15.3			16.1		15.0	9.1		
Progression Factor, PF		1.000		1.000	1.000			1.000		1.000	1.000		
Delay Calibration, k		0.17		0.13	0.11			0.31		0.11	0.11		
Incremental Delay, d <sub>2</sub>		1.4		1.9	0.5			2.8		0.5	0.2		
Initial Queue Delay, d <sub>3</sub>		0.0		0.0	0.0			0.0		0.0	0.0		
Control Delay		17.9		18.0	15.8			18.9		15.5	9.3		
Lane Group LOS		B		B	B			B		B	A		
Approach Delay		17.9			16.7			18.9			11.2		
Approach LOS		B			B			B			B		
Intersection Delay		16.7			X <sub>c</sub> = 1.00			Intersection LOS			B		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst WFB + JAG						Intersection East Main & 202						
Agency or Co. TMA						Area Type All other areas						
Date Performed 10/30/2007						Jurisdiction Town of Southeast						
Time Period PM Peak Hour						Analysis Year Existing Condition						
						Project ID Stateline Retail Center						
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>	0	1			1	1				1		1
Lane Group		LT			T	R				L		R
Volume, V (vph)	67	339			171	652				490		45
% Heavy Vehicles, %HV	4	4			2	2				3		3
Peak-Hour Factor, PHF	0.93	0.93			0.96	0.96				0.92		0.92
Pretimed (P) or Actuated (A)	A	A			A	A				A		A
Start-up Lost Time, I <sub>1</sub>		2.0			2.0	2.0				2.0		2.0
Extension of Effective Green, e		2.0			2.0	2.0				2.0		2.0
Arrival Type, AT		3			3	3				3		3
Unit Extension, UE		3.0			3.0	3.0				3.0		3.0
Filtering/Metering, I		1.000			1.000	1.000				1.000		1.000
Initial Unmet Demand, Q <sub>b</sub>		0.0			0.0	0.0				0.0		0.0
Ped / Bike / RTOR Volumes	0	0		0	0	0				0	0	0
Lane Width		13.0			11.0	11.0				12.0		11.0
Parking / Grade / Parking	N	0	N	N	0	N				N	0	N
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0			0	0				0		0
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2					3.2		
Phasing	EW Perm	02	03	04	SB Only	06	07	08				
Timing	G = 39.0	G =	G =	G =	G = 36.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 85.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		437			178	679				533		49
Lane Group Capacity, c		794			826	1531				742		642
v/c Ratio, X		0.55			0.22	0.44				0.72		0.08
Total Green Ratio, g/C		0.46			0.46	1.00				0.42		0.42
Uniform Delay, d <sub>1</sub>		16.7			13.8	0.0				20.3		14.6
Progression Factor, PF		1.000			1.000	0.950				1.000		1.000
Delay Calibration, k		0.15			0.11	0.11				0.28		0.11
Incremental Delay, d <sub>2</sub>		0.8			0.1	0.2				3.4		0.1
Initial Queue Delay, d <sub>3</sub>		0.0			0.0	0.0				0.0		0.0
Control Delay		17.5			13.9	0.2				23.7		14.6
Lane Group LOS		B			B	A				C		B
Approach Delay	17.5			3.1						22.9		
Approach LOS	B			A						C		
Intersection Delay	12.6			X <sub>c</sub> = 0.63			Intersection LOS			B		



## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	JAG		Intersection	I-84 EB ramps/Saw Mill				
Agency/Co.	TMA		Jurisdiction	City of Danbury CT				
Date Performed	12/6/2006		Analysis Year	Existing Condition				
Analysis Time Period	P.M. Peak Hour							
Project Description <i>Stateline</i>								
East/West Street: <i>I-84 EB Ramps</i>			North/South Street: <i>Saw Mill Road</i>					
Intersection Orientation: <i>North-South</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		169	94	159	96			
Peak-Hour Factor, PHF	1.00	0.83	0.83	0.87	0.87	1.00		
Hourly Flow Rate, HFR (veh/h)	0	203	113	182	110	0		
Percent Heavy Vehicles	0	--	--	1	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration			TR	LT				
Upstream Signal		0			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	44	0	55					
Peak-Hour Factor, PHF	0.73	0.73	0.73	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	60	0	75	0	0	0		
Percent Heavy Vehicles	1	1	1	0	0	0		
Percent Grade (%)	3			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	1	0	0	0	0		
Configuration		LTR						
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT					LTR	
v (veh/h)		182					135	
C (m) (veh/h)		1250					519	
v/c		0.15					0.26	
95% queue length		0.51					1.03	
Control Delay (s/veh)		8.4					14.4	
LOS		A					B	
Approach Delay (s/veh)	--	--					14.4	
Approach LOS	--	--					B	

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	Dingle /US Rt 6 & 202
Agency/Co.	TMA	Jurisdiction	Town of Southeast, NY
Date Performed	12/28/2006	Analysis Year	Existing Condition
Analysis Time Period	P.M. Peak Hour		

Project Description *Stateline*

East/West Street: *US Route 6 & 202 (Danbury Rd)*

North/South Street: *Dingle Ridge Road*

Intersection Orientation: *East-West*

Study Period (hrs): *0.25*

### Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	1	472	49	28	345	1
Peak-Hour Factor, PHF	0.95	0.95	0.95	0.93	0.93	0.93
Hourly Flow Rate, HFR (veh/h)	1	496	51	30	370	1
Percent Heavy Vehicles	1	--	--	1	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	2	0	0	2	0
Configuration	<i>LT</i>		<i>TR</i>	<i>LT</i>		<i>TR</i>
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)	55	0	24	3	0	0
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.38	0.38	0.38
Hourly Flow Rate, HFR (veh/h)	61	0	26	7	0	0
Percent Heavy Vehicles	1	1	1	1	1	1
Percent Grade (%)	2			2		
Flared Approach		Y			N	
Storage		1			0	
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration		<i>LTR</i>			<i>LTR</i>	

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>	<i>LT</i>		<i>LTR</i>			<i>LTR</i>	
v (veh/h)	1	30		87			7	
C (m) (veh/h)	1191	1025		405			318	
v/c	0.00	0.03		0.21			0.02	
95% queue length	0.00	0.09		0.80			0.07	
Control Delay (s/veh)	8.0	8.6		17.8			16.6	
LOS	A	A		C			C	
Approach Delay (s/veh)	--	--	17.8			16.6		
Approach LOS	--	--	C			C		

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	Joe's Hill Road/US 6
Agency/Co.	TMA	Jurisdiction	Town of Southeast, NY
Date Performed	12/4/2006	Analysis Year	Existing Condition
Analysis Time Period	P.M. Peak Hour		

Project Description *Stateline*

East/West Street: *US Route 6 & 202*

North/South Street: *Joe's Hill Road*

Intersection Orientation: *East-West*

Study Period (hrs): *0.25*

### Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	49	524			324	31
Peak-Hour Factor, PHF	0.95	0.95	1.00	1.00	0.89	0.89
Hourly Flow Rate, HFR (veh/h)	51	551	0	0	364	34
Percent Heavy Vehicles	1	--	--	0	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	2	0	0	2	0
Configuration	<i>LT</i>	<i>T</i>			<i>T</i>	<i>TR</i>
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)				8		22
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.68	1.00	0.68
Hourly Flow Rate, HFR (veh/h)	0	0	0	11	0	32
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			2		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					<i>LR</i>	

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>						<i>LR</i>	
v (veh/h)	51						43	
C (m) (veh/h)	1164						594	
v/c	0.04						0.07	
95% queue length	0.14						0.23	
Control Delay (s/veh)	8.2						11.5	
LOS	<i>A</i>						<i>B</i>	
Approach Delay (s/veh)	--	--					11.5	
Approach LOS	--	--					<i>B</i>	

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	US Rte 6/NYS 121
Agency/Co.	TMA	Jurisdiction	Town of Southeast, NY
Date Performed	12/4/2006	Analysis Year	Existing Condition
Analysis Time Period	P.M. Peak Hour		

Project Description <i>Stateline</i>	
East/West Street: <i>US Route 6 &amp; 202</i>	North/South Street: <i>NYS Rte 121 Peach Lake</i>
Intersection Orientation: <i>East-West</i>	Study Period (hrs): <i>0.25</i>

Vehicle Volumes and Adjustments						
Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)		464	183	56	315	
Peak-Hour Factor, PHF	1.00	0.85	0.85	0.97	0.97	1.00
Hourly Flow Rate, HFR (veh/h)	0	545	215	57	324	0
Percent Heavy Vehicles	0	--	--	1	--	--
Median Type	<i>Undivided</i>					
RT Channelized			1			0
Lanes	0	2	1	0	2	0
Configuration		T	R	LT	T	
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)	245		52			
Peak-Hour Factor, PHF	0.95	1.00	0.95	1.00	1.00	1.00
Hourly Flow Rate, HFR (veh/h)	257	0	54	0	0	0
Percent Heavy Vehicles	1	0	1	0	0	0
Percent Grade (%)	-4			0		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	1	0	1	0	0	0
Configuration	L		R			

Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT	L		R			
v (veh/h)		57	257		54			
C (m) (veh/h)		1027	298		729			
v/c		0.06	0.86		0.07			
95% queue length		0.18	7.58		0.24			
Control Delay (s/veh)		8.7	61.3		10.3			
LOS		A	F		B			
Approach Delay (s/veh)	--	--	52.4					
Approach LOS	--	--	F					

## TWO-WAY STOP CONTROL SUMMARY

General Information				Site Information				
Analyst	JAG			Intersection	Route 121/ I-84 off-ramp			
Agency/Co.	TMA			Jurisdiction	Town of Southeast			
Date Performed	12/6/2006			Analysis Year	Existing Condition			
Analysis Time Period	P.M. Peak Hour							
Project Description <i>Stateline</i>								
East/West Street: <i>I-84 off-ramp</i>				North/South Street: <i>Peach Lake Rd (NYS Rt 121)</i>				
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		240			242			
Peak-Hour Factor, PHF	1.00	0.92	1.00	1.00	0.88	1.00		
Hourly Flow Rate, HFR (veh/h)	0	260	0	0	275	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration		<i>T</i>			<i>T</i>			
Upstream Signal		0			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				75		54		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.83	1.00	0.83		
Hourly Flow Rate, HFR (veh/h)	0	0	0	90	0	65		
Percent Heavy Vehicles	0	0	0	1	0	1		
Percent Grade (%)	0			-3				
Flared Approach		<i>N</i>			<i>Y</i>			
Storage		0			3			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					<i>LR</i>			
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration				<i>LR</i>				
v (veh/h)				155				
C (m) (veh/h)				875				
v/c				0.18				
95% queue length				0.64				
Control Delay (s/veh)				12.1				
LOS				<i>B</i>				
Approach Delay (s/veh)	--	--	12.1					
Approach LOS	--	--	<i>B</i>					

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	NYS Route 121/ I-84 on-ramp
Agency/Co.	TMA	Jurisdiction	Town of Southeast
Date Performed	12/6/2006	Analysis Year	Existing Condition
Analysis Time Period	P.M. Peak Hour		
Project Description <i>Stateline</i>			
East/West Street: <i>I-84 on-ramp</i>		North/South Street: <i>NYS Rt. 121 Peach Lake Rd.</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

### Vehicle Volumes and Adjustments

Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)		238	164	88	229	
Peak-Hour Factor, PHF	1.00	0.97	0.97	0.82	0.82	1.00
Hourly Flow Rate, HFR (veh/h)	0	245	169	107	279	0
Percent Heavy Vehicles	0	--	--	2	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration			<i>TR</i>	<i>LT</i>		
Upstream Signal		0			0	

Minor Street	Eastbound			Westbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)						
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration						

### Delay, Queue Length, and Level of Service

Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		<i>LT</i>						
v (veh/h)		107						
C (m) (veh/h)		1145						
v/c		0.09						
95% queue length		0.31						
Control Delay (s/veh)		8.5						
LOS		<i>A</i>						
Approach Delay (s/veh)	--	--						
Approach LOS	--	--						

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	WFB + JAG		Intersection	Route 6 & Sodom				
Agency/Co.	TMA		Jurisdiction	Town of Southeast				
Date Performed	10/30/2007		Analysis Year	Existing Condition				
Analysis Time Period	P.M. Peak Hour							
Project Description <i>Stateline</i>								
East/West Street: <i>US Route 6/202/22</i>			North/South Street: <i>Sodom Road/Driveway</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	0	354	2	21	867	104		
Peak-Hour Factor, PHF	0.94	0.94	0.94	0.98	0.98	0.98		
Hourly Flow Rate, HFR (veh/h)	0	376	2	21	884	106		
Percent Heavy Vehicles	2	--	--	3	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	2	0	0	1	1		
Configuration	<i>LT</i>		<i>TR</i>	<i>LT</i>		<i>R</i>		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	21	9	23	52	2	0		
Peak-Hour Factor, PHF	0.66	0.66	0.66	0.79	0.79	0.79		
Hourly Flow Rate, HFR (veh/h)	31	13	34	65	2	0		
Percent Heavy Vehicles	8	8	8	2	2	2		
Percent Grade (%)	0			0				
Flared Approach		<i>N</i>			<i>N</i>			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	1	1	0	1	0		
Configuration	<i>LT</i>		<i>R</i>		<i>LTR</i>			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>	<i>LT</i>	<i>LT</i>		<i>R</i>		<i>LTR</i>	
v (veh/h)	0	21	44		34		67	
C (m) (veh/h)	694	1170	105		802		138	
v/c	0.00	0.02	0.42		0.04		0.49	
95% queue length	0.00	0.05	1.76		0.13		2.26	
Control Delay (s/veh)	10.2	8.1	62.0		9.7		53.5	
LOS	<i>B</i>	<i>A</i>	<i>F</i>		<i>A</i>		<i>F</i>	
Approach Delay (s/veh)	--	--	39.2			53.5		
Approach LOS	--	--	<i>E</i>			<i>F</i>		

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	WFB + JAG		Intersection	US Route 6 and CR 50				
Agency/Co.	TMA		Jurisdiction	Town of Southeast				
Date Performed	10/30/2007		Analysis Year	Existing Condition				
Analysis Time Period	P.M. Peak Hour							
Project Description <i>Stateline Retail Center</i>								
East/West Street: <i>US Route 6/202/22</i>			North/South Street: <i>County Road 50</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	117	338			895	0		
Peak-Hour Factor, PHF	0.98	0.98	1.00	1.00	0.98	0.98		
Hourly Flow Rate, HFR (veh/h)	119	344	0	0	913	0		
Percent Heavy Vehicles	3	--	--	0	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	1	1	0	0	1	0		
Configuration	L	T				TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				7		130		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.90	1.00	0.90		
Hourly Flow Rate, HFR (veh/h)	0	0	0	7	0	144		
Percent Heavy Vehicles	0	0	0	1	0	1		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	1	0	1		
Configuration				L		R		
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L					L		R
v (veh/h)	119					7		144
C (m) (veh/h)	742					114		333
v/c	0.16					0.06		0.43
95% queue length	0.57					0.19		2.10
Control Delay (s/veh)	10.8					38.6		23.8
LOS	B					E		C
Approach Delay (s/veh)	--	--				24.5		
Approach LOS	--	--				C		



<b>HCS+™ DETAILED REPORT</b>													
<b>General Information</b>						<b>Site Information</b>							
Analyst JAG Agency or Co. TMA Date Performed 11/16/07 Time Period Saturday Peak Hour						Intersection I-84 WB ramps/Saw Mill Road Area Type All other areas Jurisdiction Danbury, CT Analysis Year Existing Condition Project ID Stateline							
<b>Volume and Timing Input</b>													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N <sub>1</sub>				0	1	1	0	2			2	0	
Lane Group				LT	R		LT			TR			
Volume, V (vph)				27	7	73	54	90			108	31	
% Heavy Vehicles, %HV				0	0	0	1	1			0	0	
Peak-Hour Factor, PHF				0.70	0.70	0.70	0.86	0.86			0.81	0.81	
Pretimed (P) or Actuated (A)				A	A	A	A	A			A	A	
Start-up Lost Time, I <sub>1</sub>					2.0	2.0		2.0			2.0		
Extension of Effective Green, e					2.0	2.0		2.0			2.0		
Arrival Type, AT					3	3		3			5		
Unit Extension, UE					3.0	3.0		3.0			3.0		
Filtering/Metering, I					1.000	1.000		1.000			1.000		
Initial Unmet Demand, Q <sub>b</sub>					0.0	0.0		0.0			0.0		
Ped / Bike / RTOR Volumes				0	0	40	0	0		0	0	0	
Lane Width					12.0	12.0		12.0			12.0		
Parking / Grade / Parking				N	-1	N	N	1	N	N	-1	N	
Parking Maneuvers, N <sub>m</sub>													
Buses Stopping, N <sub>b</sub>					0	0		0			0		
Min. Time for Pedestrians, G <sub>p</sub>					3.2			3.2			3.2		
Phasing	WB Only	02	03	04	NS Perm	SB Only	07	08					
Timing	G = 13.0	G =	G =	G =	G = 12.0	G = 30.0	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =					
Duration of Analysis, T = 0.25						Cycle Length, C = 70.0							
<b>Lane Group Capacity, Control Delay, and LOS Determination</b>													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v					49	47		168			171		
Lane Group Capacity, c					341	301		481			2359		
v/c Ratio, X					0.14	0.16		0.35			0.07		
Total Green Ratio, g/C					0.19	0.19		0.17			0.67		
Uniform Delay, d <sub>1</sub>					23.8	23.9		25.6			4.0		
Progression Factor, PF					1.000	1.000		1.000			0.152		
Delay Calibration, k					0.11	0.11		0.11			0.11		
Incremental Delay, d <sub>2</sub>					0.2	0.2		0.4			0.0		
Initial Queue Delay, d <sub>3</sub>					0.0	0.0		0.0			0.0		
Control Delay					24.0	24.1		26.0			0.6		
Lane Group LOS					C	C		C			A		
Approach Delay				24.1			26.0			0.6			
Approach LOS				C			C			A			
Intersection Delay	15.6			X <sub>c</sub> = 0.09			Intersection LOS			B			

<b>HCS+™ DETAILED REPORT</b>												
<b>General Information</b>						<b>Site Information</b>						
Analyst	JAG					Intersection	US 6/ Saw Mill Rd					
Agency or Co.	TMA					Area Type	All other areas					
Date Performed	12/6/2006					Jurisdiction	Danbury, CT					
Time Period	Saturday Peak Hour					Analysis Year	Existing Condition					
						Project ID	Stateline					
<b>Volume and Timing Input</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>		2	0	1	1		1		1			
Lane Group		TR		L	T		L		R			
Volume, V (vph)		208	85	62	261		88		65			
% Heavy Vehicles, %HV		0	0	0	0		0		0			
Peak-Hour Factor, PHF		0.92	0.92	0.94	0.94		0.83		0.83			
Pretimed (P) or Actuated (A)		A	A	A	A		A		A			
Start-up Lost Time, I <sub>1</sub>		2.0		2.0	2.0		2.0		2.0			
Extension of Effective Green, e		2.0		2.0	2.0		2.0		2.0			
Arrival Type, AT		3		3	3		5		3			
Unit Extension, UE		3.0		3.0	3.0		3.0		3.0			
Filtering/Metering, I		1.000		1.000	1.000		1.000		1.000			
Initial Unmet Demand, Q <sub>b</sub>		0.0		0.0	0.0		0.0		0.0			
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0			
Lane Width		13.0		13.0	14.0		12.0		12.0			
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N			
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0		0	0		0		0			
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2			3.2				
Phasing	EW Perm	WB Only	03		04		NB Only	06		07		08
Timing	G = 20.0	G = 10.0	G =	G =	G = 30.0	G =	G =	G =	G =	G =	G =	G =
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =	Y =	Y =	Y =	Y =
Duration of Analysis, T = 0.25						Cycle Length, C = 75.0						
<b>Lane Group Capacity, Control Delay, and LOS Determination</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		318		66	278		106		78			
Lane Group Capacity, c		954		732	946		722		646			
v/c Ratio, X		0.33		0.09	0.29		0.15		0.12			
Total Green Ratio, g/C		0.27		0.51	0.47		0.40		0.40			
Uniform Delay, d <sub>1</sub>		22.1		10.0	12.4		14.3		14.2			
Progression Factor, PF		1.000		1.000	1.000		0.556		1.000			
Delay Calibration, k		0.11		0.11	0.11		0.11		0.11			
Incremental Delay, d <sub>2</sub>		0.2		0.1	0.2		0.1		0.1			
Initial Queue Delay, d <sub>3</sub>		0.0		0.0	0.0		0.0		0.0			
Control Delay		22.3		10.1	12.5		8.1		14.3			
Lane Group LOS		C		B	B		A		B			
Approach Delay	22.3			12.1			10.7					
Approach LOS	C			B			B					
Intersection Delay	15.6			X <sub>c</sub> = 0.23			Intersection LOS			B		

### HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>US 6, Starr/NYS 22 ramp</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>12/29/2006</i>	Jurisdiction <i>Town of Southeast</i>
Time Period <i>Saturday Peak Hour</i>	Analysis Year <i>Existing Condition</i>
	Project ID <i>Stateline</i>

#### Volume and Timing Input

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>	0	2	0	0	2	0	0	1	0	1	1	0
Lane Group		<i>LTR</i>			<i>LTR</i>			<i>LTR</i>		<i>L</i>	<i>TR</i>	
Volume, V (vph)	218	354	84	30	334	73	100	31	9	62	27	159
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.95	0.95	0.95	0.93	0.93	0.93	0.95	0.95	0.95	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A
Start-up Lost Time, I <sub>1</sub>		2.0			2.0			2.0		2.0	2.0	
Extension of Effective Green, e		2.0			2.0			2.0		2.0	2.0	
Arrival Type, AT		3			3			3		3	3	
Unit Extension, UE		3.0			3.0			3.0		3.0	3.0	
Filtering/Metering, I		1.000			1.000			1.000		1.000	1.000	
Initial Unmet Demand, Q <sub>b</sub>		0.0			0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0			12.0			15.0		10.0	9.0	
Parking / Grade / Parking	N	5	N	N	-2	N	N	-6	N	N	-5	N
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0			0			0		0	0	
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2			3.2		3.2	3.2	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 35.0	G =	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 65.0					

#### Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		690			469			147		82	245	
Lane Group Capacity, c		1261			1684			305		386	466	
v/c Ratio, X		0.55			0.28			0.48		0.21	0.53	
Total Green Ratio, g/C		0.54			0.54			0.31		0.31	0.31	
Uniform Delay, d <sub>1</sub>		9.8			8.1			18.3		16.7	18.6	
Progression Factor, PF		1.000			1.000			1.000		1.000	1.000	
Delay Calibration, k		0.15			0.11			0.11		0.11	0.13	
Incremental Delay, d <sub>2</sub>		0.5			0.1			1.2		0.3	1.1	
Initial Queue Delay, d <sub>3</sub>		0.0			0.0			0.0		0.0	0.0	
Control Delay		10.3			8.2			19.5		16.9	19.7	
Lane Group LOS		B			A			B		B	B	
Approach Delay		10.3			8.2			19.5		16.9	19.7	
Approach LOS		B			A			B		B	B	
Intersection Delay		12.3			X <sub>c</sub> = 0.54			Intersection LOS		B	B	

### HCS+™ DETAILED REPORT

General Information				Site Information			
Analyst	JAG	Intersection	Argonne /Rt 22 ramps/US 6	Area Type	All other areas		
Agency or Co.	TMA	Jurisdiction	Town of Southeast, NY	Analysis Year	Existing Condition		
Date Performed	11/7/07	Project ID	Stateline Retail Center				
Time Period	Saturday Peak Hour						

#### Volume and Timing Input

	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N <sub>l</sub>	0	1	0	1	1	0	0	2	0	0	2	0	
Lane Group		LTR		L	TR			LTR			LTR		
Volume, V (vph)	5	57	211	111	5	154	12	453	156	157	349	19	
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1	
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.84	0.84	0.84	0.86	0.86	0.86	0.91	0.91	0.91	
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A	
Start-up Lost Time, I <sub>1</sub>		2.0		2.0	2.0			2.0			2.0		
Extension of Effective Green, e		2.0		2.0	2.0			2.0			2.0		
Arrival Type, AT		3		3	3			3			3		
Unit Extension, UE		3.0		3.0	3.0			3.0			3.0		
Filtering/Metering, I		1.000		1.000	1.000			1.000			1.000		
Initial Unmet Demand, Q <sub>b</sub>		0.0		0.0	0.0			0.0			0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0	
Lane Width		15.0		9.0	9.0			12.0			12.0		
Parking / Grade / Parking	N	0	N	N	-6	N	N	-4	N	N	4	N	
Parking Maneuvers, N <sub>m</sub>													
Buses Stopping, N <sub>b</sub>		0		0	0			0			0		
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2			3.2			3.2		
Phasing	EW Perm	02	03	04	NS Perm	SB Only	07	08					
Timing	G = 20.0	G =	G =	G =	G = 23.0	G = 2.0	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =					
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0							

#### Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		303		132	189			722			578		
Lane Group Capacity, c		615		293	497			1268			1193		
v/c Ratio, X		0.49		0.45	0.38			0.57			0.48		
Total Green Ratio, g/C		0.33		0.33	0.33			0.38			0.50		
Uniform Delay, d <sub>1</sub>		16.0		15.7	15.3			14.6			9.9		
Progression Factor, PF		1.000		1.000	1.000			1.000			1.000		
Delay Calibration, k		0.11		0.11	0.11			0.16			0.11		
Incremental Delay, d <sub>2</sub>		0.6		1.1	0.5			0.6			0.3		
Initial Queue Delay, d <sub>3</sub>		0.0		0.0	0.0			0.0			0.0		
Control Delay		16.6		16.8	15.8			15.2			10.2		
Lane Group LOS		B		B	B			B			B		
Approach Delay		16.6			16.2			15.2			10.2		
Approach LOS		B			B			B			B		
Intersection Delay		14.1			X <sub>c</sub> = 0.67			Intersection LOS			B		

<b>HCS+™ DETAILED REPORT</b>												
<b>General Information</b>						<b>Site Information</b>						
Analyst <i>WFB + JAG</i>						Intersection <i>East Main &amp; 202</i>						
Agency or Co. <i>TMA</i>						Area Type <i>All other areas</i>						
Date Performed <i>10/30/2007</i>						Jurisdiction <i>Town of Southeast</i>						
Time Period <i>Saturday Peak Hour</i>						Analysis Year <i>Existing Condition</i>						
						Project ID <i>Stateline Retail Center</i>						
<b>Volume and Timing Input</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>	0	1			1	1				1		1
Lane Group		LT			T	R				L		R
Volume, V (vph)	48	171			130	409				426		37
% Heavy Vehicles, %HV	5	5			1	1				2		2
Peak-Hour Factor, PHF	0.91	0.91			0.98	0.98				0.95		0.95
Pretimed (P) or Actuated (A)	A	A			A	A				A		A
Start-up Lost Time, I <sub>1</sub>		2.0			2.0	2.0				2.0		2.0
Extension of Effective Green, e		2.0			2.0	2.0				2.0		2.0
Arrival Type, AT		3			3	3				3		3
Unit Extension, UE		3.0			3.0	3.0				3.0		3.0
Filtering/Metering, I		1.000			1.000	1.000				1.000		1.000
Initial Unmet Demand, Q <sub>b</sub>		0.0			0.0	0.0				0.0		0.0
Ped / Bike / RTOR Volumes	0	0		0	0	0				0	0	0
Lane Width		13.0			11.0	11.0				12.0		11.0
Parking / Grade / Parking	N	0	N	N	0	N				N	0	N
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0			0	0				0		0
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2					3.2		
Phasing	EW Perm	02	03	04	SB Only	06	07	08				
Timing	G = 39.0	G =	G =	G =	G = 36.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 85.0						
<b>Lane Group Capacity, Control Delay, and LOS Determination</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		241			133	417				448		39
Lane Group Capacity, c		782			834	1546				750		648
v/c Ratio, X		0.31			0.16	0.27				0.60		0.06
Total Green Ratio, g/C		0.46			0.46	1.00				0.42		0.42
Uniform Delay, d <sub>1</sub>		14.5			13.4	0.0				18.9		14.5
Progression Factor, PF		1.000			1.000	0.950				1.000		1.000
Delay Calibration, k		0.11			0.11	0.11				0.19		0.11
Incremental Delay, d <sub>2</sub>		0.2			0.1	0.1				1.3		0.0
Initial Queue Delay, d <sub>3</sub>		0.0			0.0	0.0				0.0		0.0
Control Delay		14.7			13.5	0.1				20.2		14.5
Lane Group LOS		B			B	A				C		B
Approach Delay	14.7			3.3						19.8		
Approach LOS	B			A						B		
Intersection Delay	11.7			X <sub>c</sub> = 0.45			Intersection LOS			B		

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	I-84 EB ramps/ Saw Mill Rd
Agency/Co.	TMA	Jurisdiction	City of Danbury
Date Performed	12/6/2006	Analysis Year	Existing Condition
Analysis Time Period	Saturday Peak Hour		

Project Description *Stateline*East/West Street: *I-84 Eastbound Ramps*North/South Street: *Saw Mill Road*Intersection Orientation: *North-South*Study Period (hrs): *0.25*

### Vehicle Volumes and Adjustments

Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)		80	50	75	80	
Peak-Hour Factor, PHF	1.00	0.86	0.86	0.92	0.92	1.00
Hourly Flow Rate, HFR (veh/h)	0	93	58	81	86	0
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration			TR	LT		
Upstream Signal		0			0	

Minor Street	Eastbound			Westbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)	54	0	37			
Peak-Hour Factor, PHF	0.84	0.84	0.84	1.00	1.00	1.00
Hourly Flow Rate, HFR (veh/h)	64	0	44	0	0	0
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	3			0		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	1	0	0	0	0
Configuration		LTR				

### Delay, Queue Length, and Level of Service

Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT					LTR	
v (veh/h)		81					108	
C (m) (veh/h)		1442					710	
v/c		0.06					0.15	
95% queue length		0.18					0.53	
Control Delay (s/veh)		7.6					11.0	
LOS		A					B	
Approach Delay (s/veh)	--	--					11.0	
Approach LOS	--	--					B	

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	Dingle/ US Route 6 & 202
Agency/Co.	TMA	Jurisdiction	Town of Southeast, NY
Date Performed	12/4/2006	Analysis Year	Existing Condition
Analysis Time Period	Saturday Peak Hour		

Project Description *Stateline*

East/West Street: *US Route 6 & 202 (Danbury Rd)*

North/South Street: *Dingle Ridge Road*

Intersection Orientation: *East-West*

Study Period (hrs): *0.25*

### Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	0	248	35	34	313	0
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.89	0.89	0.89
Hourly Flow Rate, HFR (veh/h)	0	295	41	38	351	0
Percent Heavy Vehicles	1	--	--	1	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	2	0	0	2	0
Configuration	LT		TR	LT		TR
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)	32	0	31	0	0	1
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.25	0.25	0.25
Hourly Flow Rate, HFR (veh/h)	36	0	35	0	0	4
Percent Heavy Vehicles	1	1	1	0	0	0
Percent Grade (%)	2			2		
Flared Approach		Y			N	
Storage		1			0	
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration		LTR			LTR	

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT	LT		LTR			LTR	
v (veh/h)	0	38		71			4	
C (m) (veh/h)	1212	1227		779			843	
v/c	0.00	0.03		0.09			0.00	
95% queue length	0.00	0.10		0.30			0.01	
Control Delay (s/veh)	8.0	8.0		12.3			9.3	
LOS	A	A		B			A	
Approach Delay (s/veh)	--	--		12.3			9.3	
Approach LOS	--	--		B			A	

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	Joe's Hill Road / US 6
Agency/Co.	TMA	Jurisdiction	Town of Southeast, NY
Date Performed	12/4/2006	Analysis Year	Existing Condition
Analysis Time Period	Saturday Peak Hour		

Project Description <i>Stateline</i>	
East/West Street: <i>US Route 6 &amp; 202</i>	North/South Street: <i>Joe's Hill Road</i>
Intersection Orientation: <i>East-West</i>	Study Period (hrs): <i>0.25</i>

Vehicle Volumes and Adjustments						
Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	12	272			326	16
Peak-Hour Factor, PHF	0.83	0.83	1.00	1.00	0.87	0.87
Hourly Flow Rate, HFR (veh/h)	14	327	0	0	374	18
Percent Heavy Vehicles	1	--	--	0	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	2	0	0	2	0
Configuration	<i>LT</i>	<i>T</i>			<i>T</i>	<i>TR</i>
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)				9		4
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.65	1.00	0.65
Hourly Flow Rate, HFR (veh/h)	0	0	0	13	0	6
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			2		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					<i>LR</i>	

Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>						<i>LR</i>	
v (veh/h)	14						19	
C (m) (veh/h)	1170						524	
v/c	0.01						0.04	
95% queue length	0.04						0.11	
Control Delay (s/veh)	8.1						12.1	
LOS	<i>A</i>						<i>B</i>	
Approach Delay (s/veh)	--	--					12.1	
Approach LOS	--	--					<i>B</i>	



## TWO-WAY STOP CONTROL SUMMARY

General Information				Site Information			
Analyst	JAG			Intersection	US Rte 6/NYS 121		
Agency/Co.	TMA			Jurisdiction	Town of Southeast, NY		
Date Performed	12/4/2006			Analysis Year	Existing Condition		
Analysis Time Period	Saturday Peak Hour						
Project Description <i>Stateline</i>							
East/West Street: <i>US Route 6 &amp; 202</i>				North/South Street: <i>NYS Rte 121 Peach Lake</i>			
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>			
Vehicle Volumes and Adjustments							
Major Street	Eastbound			Westbound			
Movement	1	2	3	4	5	6	
	L	T	R	L	T	R	
Volume (veh/h)		273	182	51	281		
Peak-Hour Factor, PHF	1.00	0.80	0.80	0.90	0.90	1.00	
Hourly Flow Rate, HFR (veh/h)	0	341	227	56	312	0	
Percent Heavy Vehicles	0	--	--	1	--	--	
Median Type	<i>Undivided</i>						
RT Channelized			1			0	
Lanes	0	2	1	0	2	0	
Configuration		T	R	LT	T		
Upstream Signal		0			0		
Minor Street	Northbound			Southbound			
Movement	7	8	9	10	11	12	
	L	T	R	L	T	R	
Volume (veh/h)	171		28				
Peak-Hour Factor, PHF	0.94	1.00	0.94	1.00	1.00	1.00	
Hourly Flow Rate, HFR (veh/h)	181	0	29	0	0	0	
Percent Heavy Vehicles	2	0	2	0	0	0	
Percent Grade (%)		-4			0		
Flared Approach		N			N		
Storage		0			0		
RT Channelized			0			0	
Lanes	1	0	1	0	0	0	
Configuration	L		R				
Delay, Queue Length, and Level of Service							
Approach	Eastbound	Westbound	Northbound			Southbound	
Movement	1	4	7	8	9	10	11
Lane Configuration		LT	L		R		
v (veh/h)		56	181		29		
C (m) (veh/h)		1222	407		844		
v/c		0.05	0.44		0.03		
95% queue length		0.14	2.23		0.11		
Control Delay (s/veh)		8.1	20.7		9.4		
LOS		A	C		A		
Approach Delay (s/veh)	--	--	19.2				
Approach LOS	--	--	C				

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	Route 121/ I-84 off-ramp
Agency/Co.	TMA	Jurisdiction	Town of Southeast
Date Performed	12/6/2006	Analysis Year	Existing Condition
Analysis Time Period	Saturday Peak Hour		

Project Description: <i>Stateline</i>	
East/West Street: <i>I-84 off-ramp</i>	North/South Street: <i>Peach Lake Rd (NYS Rt 121)</i>
Intersection Orientation: <i>North-South</i>	Study Period (hrs): <i>0.25</i>

Vehicle Volumes and Adjustments						
Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)		130			212	
Peak-Hour Factor, PHF	1.00	0.79	1.00	1.00	0.93	1.00
Hourly Flow Rate, HFR (veh/h)	0	164	0	0	227	0
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration		T			T	
Upstream Signal		0			0	

Minor Street	Eastbound			Westbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)				95		68
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.89	1.00	0.89
Hourly Flow Rate, HFR (veh/h)	0	0	0	106	0	76
Percent Heavy Vehicles	0	0	0	2	0	2
Percent Grade (%)	0			-3		
Flared Approach		N			Y	
Storage		0			3	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					LR	

Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration				LR				
v (veh/h)				182				
C (m) (veh/h)				1053				
v/c				0.17				
95% queue length				0.62				
Control Delay (s/veh)				11.0				
LOS				B				
Approach Delay (s/veh)	--	--	11.0					
Approach LOS	--	--	B					

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	NYS Route 121/ I-84 on-ramp
Agency/Co.	TMA	Jurisdiction	Town of Southeast
Date Performed	12/6/2006	Analysis Year	Existing Condition
Analysis Time Period	Saturday Peak Hour		
Project Description <i>Stateline</i>			
East/West Street: <i>I-84 on-ramp</i>		North/South Street: <i>NYS Rt. 121 Peach Lake Rd.</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

Vehicle Volumes and Adjustments						
Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)		127	122	73	223	
Peak-Hour Factor, PHF	1.00	0.83	0.83	0.94	0.94	1.00
Hourly Flow Rate, HFR (veh/h)	0	153	146	77	237	0
Percent Heavy Vehicles	0	--	--	1	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration			<i>TR</i>	<i>LT</i>		
Upstream Signal		0			0	

Minor Street	Eastbound			Westbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)						
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration						

Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		<i>LT</i>						
v (veh/h)		77						
C (m) (veh/h)		1268						
v/c		0.06						
95% queue length		0.19						
Control Delay (s/veh)		8.0						
LOS		<i>A</i>						
Approach Delay (s/veh)	--	--						
Approach LOS	--	--						

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	WFB + JAG		Intersection	Route 6 & Sodom				
Agency/Co.	TMA		Jurisdiction	Town of Southeast				
Date Performed	10/30/2007		Analysis Year	Existing Condition				
Analysis Time Period	Saturday Peak Hour							
Project Description <i>Stateline Retail Center</i>								
East/West Street: <i>US Route 6/202/22</i>			North/South Street: <i>Sodom Road/Driveway</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	0	349	28	8	520	52		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.89	0.89	0.89		
Hourly Flow Rate, HFR (veh/h)	0	379	30	8	584	58		
Percent Heavy Vehicles	2	--	--	1	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	2	0	0	1	1		
Configuration	<i>LT</i>		<i>TR</i>	<i>LT</i>		<i>R</i>		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	7	0	25	50	0	0		
Peak-Hour Factor, PHF	0.80	0.80	0.80	0.78	0.78	0.78		
Hourly Flow Rate, HFR (veh/h)	8	0	31	64	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	0			0				
Flared Approach		<i>N</i>			<i>N</i>			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	1	1	0	1	0		
Configuration	<i>LT</i>		<i>R</i>		<i>LTR</i>			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>	<i>LT</i>	<i>LT</i>		<i>R</i>		<i>LTR</i>	
v (veh/h)	0	8	8		31		64	
C (m) (veh/h)	939	1153	191		809		272	
v/c	0.00	0.01	0.04		0.04		0.24	
95% queue length	0.00	0.02	0.13		0.12		0.89	
Control Delay (s/veh)	8.8	8.1	24.7		9.6		22.3	
LOS	A	A	C		A		C	
Approach Delay (s/veh)	--	--	12.7			22.3		
Approach LOS	--	--	B			C		

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	WFB + JAG		Intersection	US Rt 6 and CR 50				
Agency/Co.	TMA		Jurisdiction	Town of Southeast				
Date Performed	10/30/2007		Analysis Year	Existing Condition				
Analysis Time Period	Saturday Peak Hour							
Project Description <i>Stateline Retail Center</i>								
East/West Street: <i>US Route 6/202/22</i>			North/South Street: <i>County Road 50</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	68	392			464	6		
Peak-Hour Factor, PHF	0.85	0.85	1.00	1.00	0.86	0.86		
Hourly Flow Rate, HFR (veh/h)	79	461	0	0	539	6		
Percent Heavy Vehicles	1	--	--	0	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	1	1	0	0	1	0		
Configuration	L	T				TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				10		112		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.85	1.00	0.85		
Hourly Flow Rate, HFR (veh/h)	0	0	0	11	0	131		
Percent Heavy Vehicles	0	0	0	2	0	2		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	1	0	1		
Configuration				L		R		
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L					L		R
v (veh/h)	79					11		131
C (m) (veh/h)	1029					199		540
v/c	0.08					0.06		0.24
95% queue length	0.25					0.17		0.94
Control Delay (s/veh)	8.8					24.1		13.8
LOS	A					C		B
Approach Delay (s/veh)	--	--				14.6		
Approach LOS	--	--				B		

### HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>I-84 WB ramps/Saw Mill</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>10/28/2007</i>	Jurisdiction <i>Danbury, CT</i>
Time Period <i>A.M. Peak Hour</i>	Analysis Year <i>No Build Condition</i>
	Project ID <i>Stateline</i>

#### Volume and Timing Input

	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N <sub>1</sub>				1	1	0	1	1			2	0	
Lane Group				L	LTR		L	T			TR		
Volume, V (vph)				460	5	406	259	180			163	56	
% Heavy Vehicles, %HV				3	3	3	1	1			1	1	
Peak-Hour Factor, PHF				0.92	0.92	0.92	0.86	0.86			0.92	0.92	
Pretimed (P) or Actuated (A)				A	A	A	A	A			A	A	
Start-up Lost Time, I <sub>1</sub>				2.0	2.0		2.0	2.0			2.0		
Extension of Effective Green, e				2.0	2.0		2.0	2.0			2.0		
Arrival Type, AT				3	3		5	5			5		
Unit Extension, UE				3.0	3.0		3.0	3.0			3.0		
Filtering/Metering, I				1.000	1.000		1.000	1.000			1.000		
Initial Unmet Demand, Q <sub>b</sub>				0.0	0.0		0.0	0.0			0.0		
Ped / Bike / RTOR Volumes				0	0	200	0	0		0	0	0	
Lane Width				12.0	12.0		11.0	11.0			12.0		
Parking / Grade / Parking				N	-1	N	N	1	N	N	-1	N	
Parking Maneuvers, N <sub>m</sub>													
Buses Stopping, N <sub>b</sub>				0	0		0	0			0		
Min. Time for Pedestrians, G <sub>p</sub>				3.2			3.2			3.2			
Phasing	WB Only	02	03	04	NB Only	SB Only	07	08					
Timing	G = 19.0	G =	G =	G =	G = 18.0	G = 23.0	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =					
Duration of Analysis, T = 0.25						Cycle Length, C = 75.0							

#### Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v				400	329		301	209			238	
Lane Group Capacity, c				446	415		413	434			1061	
v/c Ratio, X				0.90	0.79		0.73	0.48			0.22	
Total Green Ratio, g/C				0.25	0.25		0.24	0.24			0.31	
Uniform Delay, d <sub>1</sub>				27.1	26.2		26.3	24.5			19.4	
Progression Factor, PF				1.000	1.000		0.789	0.789			0.705	
Delay Calibration, k				0.42	0.34		0.29	0.11			0.11	
Incremental Delay, d <sub>2</sub>				20.4	10.1		6.4	0.8			0.1	
Initial Queue Delay, d <sub>3</sub>				0.0	0.0		0.0	0.0			0.0	
Control Delay				47.5	36.3		27.1	20.2			13.8	
Lane Group LOS				D	D		C	C			B	
Approach Delay				42.4			24.3			13.8		
Approach LOS				D			C			B		
Intersection Delay	31.6			X <sub>c</sub> = 0.59			Intersection LOS			C		

<b>HCS+™ DETAILED REPORT</b>													
<b>General Information</b>						<b>Site Information</b>							
Analyst JAG						Intersection US 6 and Saw Mill Rd							
Agency or Co. TMA						Area Type All other areas							
Date Performed 10/28/2007						Jurisdiction Danbury, CT							
Time Period A.M. Peak Hour						Analysis Year No Build Condition							
						Project ID Stateline							
<b>Volume and Timing Input</b>													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N <sub>1</sub>		2	0	1	1		2		0				
Lane Group		TR		L	T		L	LR					
Volume, V (vph)		96	103	117	573		535		66				
% Heavy Vehicles, %HV		1	1	0	0		1		1				
Peak-Hour Factor, PHF		0.89	0.89	0.80	0.80		0.87		0.87				
Pretimed (P) or Actuated (A)		A	A	A	A		A		A				
Start-up Lost Time, I <sub>1</sub>		2.0		2.0	2.0		2.0	2.0					
Extension of Effective Green, e		2.0		2.0	2.0		2.0	2.0					
Arrival Type, AT		3		3	3		5	3					
Unit Extension, UE		3.0		3.0	3.0		3.0	3.0					
Filtering/Metering, I		1.000		1.000	1.000		1.000	1.000					
Initial Unmet Demand, Q <sub>b</sub>		0.0		0.0	0.0		0.0	0.0					
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0				
Lane Width		13.0		13.0	14.0		11.0	11.0					
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N				
Parking Maneuvers, N <sub>m</sub>													
Buses Stopping, N <sub>b</sub>		0		0	0		0	0					
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2			3.2					
Phasing	EW Perm	WB Only	03			04			NB Only	06		07	08
Timing	G = 13.0	G = 10.0	G =			G =			G = 37.0	G =		G =	G =
	Y = 5	Y = 5	Y =			Y =			Y = 5	Y =		Y =	Y =
Duration of Analysis, T = 0.25							Cycle Length, C = 75.0						
<b>Lane Group Capacity, Control Delay, and LOS Determination</b>													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		224		146	716		338	353					
Lane Group Capacity, c		592		656	757		1655	835					
v/c Ratio, X		0.38		0.22	0.95		0.20	0.42					
Total Green Ratio, g/C		0.17		0.41	0.37		0.49	0.49					
Uniform Delay, d <sub>1</sub>		27.4		14.7	22.8		10.7	12.2					
Progression Factor, PF		1.000		1.000	1.000		0.351	1.000					
Delay Calibration, k		0.11		0.11	0.46		0.11	0.11					
Incremental Delay, d <sub>2</sub>		0.4		0.2	20.6		0.1	0.3					
Initial Queue Delay, d <sub>3</sub>		0.0		0.0	0.0		0.0	0.0					
Control Delay		27.8		14.9	43.4		3.8	12.5					
Lane Group LOS		C		B	D		A	B					
Approach Delay	27.8			38.5			8.3						
Approach LOS	C			D			A						
Intersection Delay	25.4			X <sub>c</sub> = 0.65			Intersection LOS			C			

## SHORT REPORT

General Information	Site Information
Analyst <b>JAG</b> Agency or Co. <b>TMA</b> Date Performed <b>12/28/2006</b> Time Period <b>A.M. Peak Hour</b>	Intersection <b>US 6, Starr NYS 22 ramp</b> Area Type <b>All other areas</b> Jurisdiction <b>Town of Southeast</b> Analysis Year <b>No Build Condition</b>

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes	0	2	0	0	2	0	0	1	0	1	1	0
Lane Group	DefL	TR			LTR			LTR		L	TR	
Volume (vph)	130	342	113	42	1174	90	95	28	7	125	98	116
% Heavy Vehicles	5	5	5	5	5	5	5	5	5	5	5	5
PHF	0.74	0.74	0.74	0.91	0.91	0.91	0.80	0.80	0.80	0.91	0.91	0.91
Pretimed/Actuated (P/A)	A	A	A	A	A	A	A	A	A	A	A	A
Startup Lost Time	2.0	2.0			2.0			2.0		2.0	2.0	
Extension of Effective Green	2.0	2.0			2.0			2.0		2.0	2.0	
Arrival Type	3	3			3			3		3	3	
Unit Extension	3.0	3.0			3.0			3.0		3.0	3.0	
Ped/Bike/RTOR Volume	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	12.0	12.0			12.0			15.0		10.0	9.0	
Parking/Grade/Parking	N	5	N	N	-2	N	N	-6	N	N	-5	N
Parking/Hour												
Bus Stops/Hour	0	0			0			0		0	0	
Minimum Pedestrian Time		3.2			3.2			3.2			3.2	
Phasing	EW Perm	EB Only	03	04	NS Perm	06	07	08				
Timing	G = 30.0	G = 4.0	G =	G =	G = 16.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis (hrs) = 0.25						Cycle Length C = 65.0						

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	Adjusted Flow Rate	176	615			1435			163		137	235
Lane Group Capacity	420	1019			1457			201		294	378	
v/c Ratio	0.42	0.60			0.98			0.81		0.47	0.62	
Green Ratio	0.65	0.60			0.46			0.25		0.25	0.25	
Uniform Delay $d_1$	18.7	8.2			17.3			23.1		20.9	21.8	
Delay Factor k	0.11	0.19			0.49			0.35		0.11	0.20	
Incremental Delay $d_2$	0.7	1.0			20.0			21.6		1.2	3.1	
PF Factor	1.000	1.000			1.000			1.000		1.000	1.000	
Control Delay	19.4	9.2			37.2			44.7		22.0	25.0	
Lane Group LOS	B	A			D			D		C	C	
Approach Delay	11.5			37.2			44.7			23.9		
Approach LOS	B			D			D			C		
Intersection Delay	28.5			Intersection LOS						C		



## SHORT REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>Argonne/US 6</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>12/28/2006</i>	Jurisdiction <i>Town of Southeast</i>
Time Period <i>A.M. Peak Hour</i>	Analysis Year <i>No Build Condition</i>

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes	0	1	0	1	1	0	0	2	0	0	2	0
Lane Group		<i>LTR</i>		<i>L</i>	<i>TR</i>			<i>LTR</i>		<i>DefL</i>	<i>TR</i>	
Volume (vph)	2	82	104	212	6	120	18	787	583	176	279	3
% Heavy Vehicles	5	5	5	5	5	5	5	5	5	5	5	5
PHF	0.74	0.74	0.74	0.85	0.85	0.85	0.97	0.97	0.97	0.88	0.88	0.88
Pretimed/Actuated (P/A)	A	A	A	A	A	A	A	A	A	A	A	A
Startup Lost Time		2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green		2.0		2.0	2.0			2.0		2.0	2.0	
Arrival Type		3		3	3			3		3	3	
Unit Extension		3.0		3.0	3.0			3.0		3.0	3.0	
Ped/Bike/RTOR Volume	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.0		9.0	9.0			12.0		12.0	12.0	
Parking/Grade/Parking	<i>N</i>	0	<i>N</i>	<i>N</i>	-6	<i>N</i>	<i>N</i>	-4	<i>N</i>	<i>N</i>	4	<i>N</i>
Parking/Hour												
Bus Stops/Hour		0		0	0			0		0	0	
Minimum Pedestrian Time		3.2			3.2			3.2			3.2	
Phasing	EW Perm	WB Only	03	04	NS Perm	SB Only	07	08				
Timing	G = 13.0	G = 2.0	G =	G =	G = 33.0	G = 2.0	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis (hrs) = 0.25							Cycle Length C = 70.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	Adjusted Flow Rate		255		249	148			1431		200	320
Lane Group Capacity		341		342	411			1473		344	1012	
v/c Ratio		0.75		0.73	0.36			0.97		0.58	0.32	
Green Ratio		0.19		0.33	0.29			0.47		0.61	0.57	
Uniform Delay d <sub>1</sub>		26.9		25.7	19.9			18.0		23.4	7.8	
Delay Factor k		0.30		0.29	0.11			0.48		0.17	0.11	
Incremental Delay d <sub>2</sub>		8.8		7.6	0.5			17.1		2.5	0.2	
PF Factor		1.000		1.000	1.000			1.000		1.000	1.000	
Control Delay		35.8		33.3	20.4			35.1		25.9	8.0	
Lane Group LOS		<i>D</i>		<i>C</i>	<i>C</i>			<i>D</i>		<i>C</i>	<i>A</i>	
Approach Delay		35.8			28.5			35.1			14.9	
Approach LOS		<i>D</i>			<i>C</i>			<i>D</i>			<i>B</i>	
Intersection Delay		30.1			Intersection LOS						<i>C</i>	

### HCS+™ DETAILED REPORT

General Information	Site Information
Analyst           WFB + JAG	Intersection     Rt 6 (East Main) & 202/22
Agency or Co.   TMA	Area Type        All other areas
Date Performed  10/30/2007	Jurisdiction     Town of Southeast
Time Period     AM Peak Hour	Analysis Year    No Build Condition
	Project ID        Stateline Retail Center

#### Volume and Timing Input

	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N <sub>1</sub>	0	1			1	1				1		1	
Lane Group		LT			T	R				L		R	
Volume, V (vph)	15	167			512	427				511		115	
% Heavy Vehicles, %HV	22	22			5	5				7		7	
Peak-Hour Factor, PHF	0.75	0.75			0.93	0.93				0.77		0.77	
Pretimed (P) or Actuated (A)	A	A			A	A				A		A	
Start-up Lost Time, I <sub>1</sub>		2.0			2.0	2.0				2.0		2.0	
Extension of Effective Green, e		2.0			2.0	2.0				2.0		2.0	
Arrival Type, AT		3			3	3				3		3	
Unit Extension, UE		3.0			3.0	3.0				3.0		3.0	
Filtering/Metering, I		1.000			1.000	1.000				1.000		1.000	
Initial Unmet Demand, Q <sub>b</sub>		0.0			0.0	0.0				0.0		0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0				0	0	0	
Lane Width		13.0			11.0	11.0				12.0		11.0	
Parking / Grade / Parking	N	0	N	N	0	N				N	0	N	
Parking Maneuvers, N <sub>m</sub>													
Buses Stopping, N <sub>b</sub>		0			0	0				0		0	
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2						3.2		
Phasing	EW Perm	02	03	04	SB Only	06	07	08					
Timing	G = 39.0	G =	G =	G =	G = 36.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 85.0						

#### Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		243			551	459				664		149
Lane Group Capacity, c		697			802	1487				714		618
v/c Ratio, X		0.35			0.69	0.31				0.93		0.24
Total Green Ratio, g/C		0.46			0.46	1.00				0.42		0.42
Uniform Delay, d <sub>1</sub>		14.8			18.2	0.0				23.3		15.7
Progression Factor, PF		1.000			1.000	0.950				1.000		1.000
Delay Calibration, k		0.11			0.26	0.11				0.45		0.11
Incremental Delay, d <sub>2</sub>		0.3			2.5	0.1				18.7		0.2
Initial Queue Delay, d <sub>3</sub>		0.0			0.0	0.0				0.0		0.0
Control Delay		15.1			20.7	0.1				42.0		15.9
Lane Group LOS		B			C	A				D		B
Approach Delay	15.1			11.3						37.2		
Approach LOS	B			B						D		
Intersection Delay	22.0			X <sub>c</sub> = 0.80			Intersection LOS			C		

<b>HCS+™ DETAILED REPORT</b>												
<b>General Information</b>						<b>Site Information</b>						
Analyst JAG						Intersection Saw Mill Rd/I-84 EB ramps						
Agency or Co. TMA						Area Type All other areas						
Date Performed 10/28/2007						Jurisdiction Danbury, CT						
Time Period A.M. Peak Hour						Analysis Year No Build Condition						
						Project ID Stateline						
<b>Volume and Timing Input</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>	0	1	1					1	1	0	2	
Lane Group		LT	R					T	R		LT	
Volume, V (vph)	72	0	596					363	282	77	544	
% Heavy Vehicles, %HV	1	1	1					0	0	2	2	
Peak-Hour Factor, PHF	0.89	0.89	0.89					0.89	0.89	0.96	0.96	
Pretimed (P) or Actuated (A)	A	A	A					A	A	A	A	
Start-up Lost Time, I <sub>1</sub>		2.0	2.0					2.0	2.0		2.0	
Extension of Effective Green, e		2.0	2.0					2.0	2.0		2.0	
Arrival Type, AT		3	3					3	3		5	
Unit Extension, UE		3.0	3.0					3.0	3.0		3.0	
Filtering/Metering, I		1.000	1.000					1.000	1.000		1.000	
Initial Unmet Demand, Q <sub>b</sub>		0.0	0.0					0.0	0.0		0.0	
Ped / Bike / RTOR Volumes	0	0	0				0	0	50	0	0	
Lane Width		12.0	12.0					11.0	11.0		11.0	
Parking / Grade / Parking	N	3	N				N	8	N	N	-5	N
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0	0					0	0		0	
Min. Time for Pedestrians, G <sub>p</sub>		3.2						3.2			3.2	
Phasing	EB Only	02	03	04	SB Only	NB Only	07	08				
Timing	G = 10.0	G =	G =	G =	G = 30.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 75.0						
<b>Lane Group Capacity, Control Delay, and LOS Determination</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		81	670					408	261		647	
Lane Group Capacity, c		235	735					470	400		1397	
v/c Ratio, X		0.34	0.91					0.87	0.65		0.46	
Total Green Ratio, g/C		0.13	0.47					0.27	0.27		0.40	
Uniform Delay, d <sub>1</sub>		29.5	18.6					26.2	24.4		16.6	
Progression Factor, PF		1.000	1.000					1.000	1.000		0.556	
Delay Calibration, k		0.11	0.43					0.40	0.23		0.11	
Incremental Delay, d <sub>2</sub>		0.9	15.6					15.8	3.8		0.2	
Initial Queue Delay, d <sub>3</sub>		0.0	0.0					0.0	0.0		0.0	
Control Delay		30.4	34.2					42.0	28.2		9.4	
Lane Group LOS		C	C					D	C		A	
Approach Delay	33.8						36.6			9.4		
Approach LOS	C						D			A		
Intersection Delay	27.1			X <sub>c</sub> = 0.65			Intersection LOS			C		

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	US 6/Dingle Ridge Road
Agency/Co.	TMA	Jurisdiction	Town of Southeast, NY
Date Performed	12/28/2006	Analysis Year	No Build Condition
Analysis Time Period	A.M. Peak Hour		

Project Description *Stateline*

East/West Street: *US Route 6 & 202 (Danbury Rd)*

North/South Street: *Dingle Ridge Road*

Intersection Orientation: *East-West*

Study Period (hrs): *0.25*

### Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	0	181	20	14	1109	2
Peak-Hour Factor, PHF	0.75	0.75	0.75	0.92	0.92	0.92
Hourly Flow Rate, HFR (veh/h)	0	241	26	15	1205	2
Percent Heavy Vehicles	5	--	--	1	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	2	0	0	2	0
Configuration	<i>LT</i>		<i>TR</i>	<i>LT</i>		<i>TR</i>
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)	38	2	19	0	2	1
Peak-Hour Factor, PHF	0.78	0.78	0.78	0.38	0.38	0.38
Hourly Flow Rate, HFR (veh/h)	48	2	24	0	5	2
Percent Heavy Vehicles	3	3	3	3	3	3
Percent Grade (%)	2			2		
Flared Approach		Y			N	
Storage		1			0	
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration		<i>LTR</i>			<i>LTR</i>	

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>	<i>LT</i>		<i>LTR</i>			<i>LTR</i>	
v (veh/h)	0	15		74			7	
C (m) (veh/h)	557	1301		320			148	
v/c	0.00	0.01		0.23			0.05	
95% queue length	0.00	0.03		0.88			0.15	
Control Delay (s/veh)	11.5	7.8		21.0			30.5	
LOS	<i>B</i>	<i>A</i>		<i>C</i>			<i>D</i>	
Approach Delay (s/veh)	--	--	21.0			30.5		
Approach LOS	--	--	<i>C</i>			<i>D</i>		

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst		Intersection	<i>Joe's Hill Road / US 6</i>
Agency/Co.	<i>TMA</i>	Jurisdiction	<i>Town of Southeast, NY</i>
Date Performed	<i>12/28/2006</i>	Analysis Year	<i>No Build Condition</i>
Analysis Time Period	<i>A.M. Peak Hour</i>		

Project Description *Stateline*

East/West Street: *US Route 6 & 202*

North/South Street: *Joe's Hill Road*

Intersection Orientation: *East-West*

Study Period (hrs): *0.25*

### Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	14	190			1119	7
Peak-Hour Factor, PHF	0.73	0.73	1.00	1.00	0.91	0.91
Hourly Flow Rate, HFR (veh/h)	19	260	0	0	1229	7
Percent Heavy Vehicles	5	--	--	0	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	2	0	0	2	0
Configuration	<i>LT</i>	<i>T</i>			<i>T</i>	<i>TR</i>
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)				21		54
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.86	1.00	0.86
Hourly Flow Rate, HFR (veh/h)	0	0	0	24	0	62
Percent Heavy Vehicles	0	0	0	3	0	3
Percent Grade (%)	0			2		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					<i>LR</i>	

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>						<i>LR</i>	
v (veh/h)	19						86	
C (m) (veh/h)	543						256	
v/c	0.03						0.34	
95% queue length	0.11						1.42	
Control Delay (s/veh)	11.9						26.0	
LOS	<i>B</i>						<i>D</i>	
Approach Delay (s/veh)	--	--					26.0	
Approach LOS	--	--					<i>D</i>	

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	US Rte 6/NYS 121
Agency/Co.	TMA	Jurisdiction	Town of Southeast, NY
Date Performed	12/28/2006	Analysis Year	No Build Condition
Analysis Time Period	A.M. Peak Hour		

Project Description <i>Stateline</i>	
East/West Street: <i>US Route 6 &amp; 202</i>	North/South Street: <i>NYS Rte 121 Peach Lake</i>
Intersection Orientation: <i>East-West</i>	Study Period (hrs): <i>0.25</i>

Vehicle Volumes and Adjustments						
Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)		203	283	244	953	
Peak-Hour Factor, PHF	1.00	0.75	0.75	0.92	0.92	1.00
Hourly Flow Rate, HFR (veh/h)	0	270	377	265	1035	0
Percent Heavy Vehicles	0	--	--	5	--	--
Median Type	<i>Undivided</i>					
RT Channelized			1			0
Lanes	0	2	1	0	2	0
Configuration		T	R	LT	T	
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)	327		14			
Peak-Hour Factor, PHF	0.86	1.00	0.86	1.00	1.00	1.00
Hourly Flow Rate, HFR (veh/h)	380	0	16	0	0	0
Percent Heavy Vehicles	4	0	4	0	0	0
Percent Grade (%)	-4			0		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	1	0	1	0	0	0
Configuration	L		R			

Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT	L		R			
v (veh/h)		265	380		16			
C (m) (veh/h)		1269	116		883			
v/c		0.21	3.28		0.02			
95% queue length		0.79	36.87		0.06			
Control Delay (s/veh)		8.6	1103		9.2			
LOS		A	F		A			
Approach Delay (s/veh)	--	--	1059					
Approach LOS	--	--	F					

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	Route 121/ I-84 off-ramp
Agency/Co.	TMA	Jurisdiction	Town of Southeast
Date Performed	12/28/06	Analysis Year	No Build Condition
Analysis Time Period	A.M. Peak Hour		

Project Description: <i>Stateline</i>	
East/West Street: <i>I-84 off-ramp</i>	North/South Street: <i>Peach Lake Rd (NYS Rt 121)</i>
Intersection Orientation: <i>North-South</i>	Study Period (hrs): <i>0.25</i>

Vehicle Volumes and Adjustments						
Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)		115			563	
Peak-Hour Factor, PHF	1.00	0.88	1.00	1.00	0.72	1.00
Hourly Flow Rate, HFR (veh/h)	0	130	0	0	781	0
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration		<i>T</i>			<i>T</i>	
Upstream Signal		0			0	

Minor Street	Eastbound			Westbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)				443		203
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.87	1.00	0.87
Hourly Flow Rate, HFR (veh/h)	0	0	0	509	0	233
Percent Heavy Vehicles	0	0	0	5	0	5
Percent Grade (%)	0			-3		
Flared Approach		<i>N</i>			<i>Y</i>	
Storage		0			3	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					<i>LR</i>	

Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration				<i>LR</i>				
v (veh/h)				742				
C (m) (veh/h)				384				
v/c				1.93				
95% queue length				50.28				
Control Delay (s/veh)				452.5				
LOS				<i>F</i>				
Approach Delay (s/veh)	--	--	452.5					
Approach LOS	--	--	<i>F</i>					

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	NYS Route 121/ I-84 on-ramp
Agency/Co.	TMA	Jurisdiction	Town of Southeast
Date Performed	12/28/2006	Analysis Year	No Build Condition
Analysis Time Period	A.M. Peak Hour		
Project Description <i>Stateline</i>			
East/West Street: <i>I-84 on-ramp</i>		North/South Street: <i>NYS Rt. 121 Peach Lake Rd.</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

### Vehicle Volumes and Adjustments

Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)		115	71	89	913	
Peak-Hour Factor, PHF	1.00	0.91	0.91	0.89	0.89	1.00
Hourly Flow Rate, HFR (veh/h)	0	126	78	100	1025	0
Percent Heavy Vehicles	0	--	--	3	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration			<i>TR</i>	<i>LT</i>		
Upstream Signal		0			0	

Minor Street	Eastbound			Westbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)						
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration						

### Delay, Queue Length, and Level of Service

Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		<i>LT</i>						
v (veh/h)		100						
C (m) (veh/h)		1362						
v/c		0.07						
95% queue length		0.24						
Control Delay (s/veh)		7.9						
LOS		<i>A</i>						
Approach Delay (s/veh)	--	--						
Approach LOS	--	--						



## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information						
Analyst	WFB + JAG	Intersection	Route 6 & Sodom					
Agency/Co.	TMA	Jurisdiction	Town of Southeast					
Date Performed	10/30/2007	Analysis Year	No Build Condition					
Analysis Time Period	A.M. Peak Hour							
Project Description <i>Stateline</i>								
East/West Street: <i>US Route 6/202/22</i>				North/South Street: <i>Sodom Road/Driveway</i>				
Intersection Orientation: <i>East-West</i>				Study Period (hrs): <i>0.25</i>				
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	0	285	4	1	857	19		
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.95	0.95	0.95		
Hourly Flow Rate, HFR (veh/h)	0	335	4	1	902	20		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	2	0	0	1	1		
Configuration	<i>LT</i>		<i>TR</i>	<i>LT</i>		<i>R</i>		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	7	3	13	31	0	5		
Peak-Hour Factor, PHF	0.52	0.52	0.52	0.67	0.67	0.67		
Hourly Flow Rate, HFR (veh/h)	13	5	25	46	0	7		
Percent Heavy Vehicles	9	9	9	14	14	14		
Percent Grade (%)	0			0				
Flared Approach		<i>N</i>			<i>N</i>			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	1	1	0	1	0		
Configuration	<i>LT</i>		<i>R</i>		<i>LTR</i>			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>	<i>LT</i>	<i>LT</i>		<i>R</i>		<i>LTR</i>	
v (veh/h)	0	1	18		25		53	
C (m) (veh/h)	749	1231	127		823		159	
v/c	0.00	0.00	0.14		0.03		0.33	
95% queue length	0.00	0.00	0.48		0.09		1.36	
Control Delay (s/veh)	9.8	7.9	38.0		9.5		38.6	
LOS	<i>A</i>	<i>A</i>	<i>E</i>		<i>A</i>		<i>E</i>	
Approach Delay (s/veh)	--	--	21.4			38.6		
Approach LOS	--	--	<i>C</i>			<i>E</i>		

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	WFB + JAG		Intersection	US Route 6 and CR 50				
Agency/Co.	TMA		Jurisdiction	Town of Southeast				
Date Performed	10/30/2007		Analysis Year	No Build Condition				
Analysis Time Period	A.M. Peak Hour							
Project Description <i>Stateline Retail Center</i>								
East/West Street: <i>US Route 6/202/22</i>			North/South Street: <i>County Road 50</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	67	289			869	0		
Peak-Hour Factor, PHF	0.86	0.86	1.00	1.00	0.97	0.97		
Hourly Flow Rate, HFR (veh/h)	77	336	0	0	895	0		
Percent Heavy Vehicles	11	--	--	0	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	1	1	0	0	1	0		
Configuration	L	T				TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				2		148		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.87	1.00	0.87		
Hourly Flow Rate, HFR (veh/h)	0	0	0	2	0	170		
Percent Heavy Vehicles	0	0	0	10	0	10		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	1	0	1		
Configuration				L		R		
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L					L		R
v (veh/h)	77					2		170
C (m) (veh/h)	722					136		328
v/c	0.11					0.01		0.52
95% queue length	0.36					0.04		2.82
Control Delay (s/veh)	10.6					31.9		27.2
LOS	B					D		D
Approach Delay (s/veh)	--	--				27.3		
Approach LOS	--	--				D		

## HCS+™ DETAILED REPORT

General Information				Site Information			
Analyst	JAG			Intersection	I-84 WB ramps/Saw Mill		
Agency or Co.	TMA			Area Type	All other areas		
Date Performed	10/29/2007			Jurisdiction	Danbury, CT		
Time Period	P.M. Peak Hour			Analysis Year	No Build Condition		
				Project ID	Stateline		

## Volume and Timing Input

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>				1	1	0	1	1			2	0
Lane Group				L	LTR		L	T			TR	
Volume, V (vph)				334	3	68	345	178			326	73
% Heavy Vehicles, %HV				1	1	1	1	1			1	1
Peak-Hour Factor, PHF				0.89	0.89	0.89	0.88	0.88			0.92	0.92
Pretimed (P) or Actuated (A)				A	A	A	A	A			A	A
Start-up Lost Time, I <sub>1</sub>				2.0	2.0		2.0	2.0			2.0	
Extension of Effective Green, e				2.0	2.0		2.0	2.0			2.0	
Arrival Type, AT				3	3		5	5			5	
Unit Extension, UE				3.0	3.0		3.0	3.0			3.0	
Filtering/Metering, I				1.000	1.000		1.000	1.000			1.000	
Initial Unmet Demand, Q <sub>b</sub>				0.0	0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes				0	0	15	0	0		0	0	0
Lane Width				12.0	12.0		11.0	11.0			12.0	
Parking / Grade / Parking				N	-1	N	N	1	N	N	-1	N
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>				0	0		0	0			0	
Min. Time for Pedestrians, G <sub>p</sub>				3.2			3.2			3.2		
Phasing	WB Only	02	03	04	NB Only	SB Only	07	08				
Timing	G = 13.0	G =	G =	G =	G = 23.0	G = 24.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 75.0						

## Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v				263	175		392	202			433	
Lane Group Capacity, c				311	301		527	555			1120	
v/c Ratio, X				0.85	0.58		0.74	0.36			0.39	
Total Green Ratio, g/C				0.17	0.17		0.31	0.31			0.32	
Uniform Delay, d <sub>1</sub>				30.0	28.5		23.4	20.3			19.8	
Progression Factor, PF				1.000	1.000		0.705	0.705			0.686	
Delay Calibration, k				0.38	0.17		0.30	0.11			0.11	
Incremental Delay, d <sub>2</sub>				18.9	2.8		5.7	0.4			0.2	
Initial Queue Delay, d <sub>3</sub>				0.0	0.0		0.0	0.0			0.0	
Control Delay				49.0	31.3		22.1	14.7			13.8	
Lane Group LOS				D	C		C	B			B	
Approach Delay				41.9			19.6			13.8		
Approach LOS				D			B			B		
Intersection Delay	24.6			X <sub>c</sub> = 0.62			Intersection LOS			C		

<b>HCS+™ DETAILED REPORT</b>													
<b>General Information</b>						<b>Site Information</b>							
Analyst JAG						Intersection US 6 and Saw Mill Rd							
Agency or Co. TMA						Area Type All other areas							
Date Performed 10/29/2007						Jurisdiction Danbury, CT							
Time Period P.M. Peak Hour						Analysis Year No Build Condition							
						Project ID Stateline							
<b>Volume and Timing Input</b>													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N <sub>1</sub>		2	0	1	1		2		0				
Lane Group		TR		L	T		L	LR					
Volume, V (vph)		349	217	176	222		124		131				
% Heavy Vehicles, %HV		1	1	1	1		1		1				
Peak-Hour Factor, PHF		0.95	0.95	0.87	0.87		0.98		0.98				
Pretimed (P) or Actuated (A)		A	A	A	A		A		A				
Start-up Lost Time, I <sub>1</sub>		2.0		2.0	2.0		2.0	2.0					
Extension of Effective Green, e		2.0		2.0	2.0		2.0	2.0					
Arrival Type, AT		3		3	3		5	3					
Unit Extension, UE		3.0		3.0	3.0		3.0	3.0					
Filtering/Metering, I		1.000		1.000	1.000		1.000	1.000					
Initial Unmet Demand, Q <sub>b</sub>		0.0		0.0	0.0		0.0	0.0					
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0				
Lane Width		13.0		13.0	14.0		11.0	11.0					
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N				
Parking Maneuvers, N <sub>m</sub>													
Buses Stopping, N <sub>b</sub>		0		0	0		0	0					
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2			3.2					
Phasing	EW Perm	WB Only	03			04			NB Only	06		07	08
Timing	G = 20.0	G = 4.0	G =			G =			G = 36.0	G =		G =	
	Y = 5	Y = 5	Y =			Y =			Y = 5	Y =		Y =	
Duration of Analysis, T = 0.25							Cycle Length, C = 75.0						
<b>Lane Group Capacity, Control Delay, and LOS Determination</b>													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		595		202	255		64	197					
Lane Group Capacity, c		930		445	776		1610	771					
v/c Ratio, X		0.64		0.45	0.33		0.04	0.26					
Total Green Ratio, g/C		0.27		0.43	0.39		0.48	0.48					
Uniform Delay, d <sub>1</sub>		24.3		21.5	16.2		10.3	11.6					
Progression Factor, PF		1.000		1.000	1.000		0.385	1.000					
Delay Calibration, k		0.22		0.11	0.11		0.11	0.11					
Incremental Delay, d <sub>2</sub>		1.5		0.7	0.2		0.0	0.2					
Initial Queue Delay, d <sub>3</sub>		0.0		0.0	0.0		0.0	0.0					
Control Delay		25.8		22.3	16.4		4.0	11.7					
Lane Group LOS		C		C	B		A	B					
Approach Delay	25.8			19.0			9.8						
Approach LOS	C			B			A						
Intersection Delay	20.3			X <sub>c</sub> = 0.48			Intersection LOS			C			

### HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>US 6, Starr/NYS 22 ramp</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>12/29/2006</i>	Jurisdiction <i>Town of Southeast</i>
Time Period <i>P.M. Peak Hour</i>	Analysis Year <i>No Build Condition</i>
	Project ID <i>Stateline</i>

#### Volume and Timing Input

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>	0	2	0	0	2	0	0	1	0	1	1	0
Lane Group		<i>LTR</i>			<i>LTR</i>			<i>LTR</i>		<i>L</i>	<i>TR</i>	
Volume, V (vph)	156	522	87	19	485	90	147	50	19	238	39	321
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.95	0.95	0.95	0.92	0.92	0.92	0.90	0.90	0.90	0.90	0.90	0.90
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A
Start-up Lost Time, I <sub>1</sub>		2.0			2.0			2.0		2.0	2.0	
Extension of Effective Green, e		2.0			2.0			2.0		2.0	2.0	
Arrival Type, AT		3			3			3		3	3	
Unit Extension, UE		3.0			3.0			3.0		3.0	3.0	
Filtering/Metering, I		1.000			1.000			1.000		1.000	1.000	
Initial Unmet Demand, Q <sub>b</sub>		0.0			0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0			12.0			15.0		10.0	9.0	
Parking / Grade / Parking	N	5	N	N	-2	N	N	-6	N	N	-5	N
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0			0			0		0	0	
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2			3.2		3.2	3.2	
Phasing	EW Perm	EB Only	03	04	NS Perm	06	07	08				
Timing	G = 20.0	G = 5.0	G =	G =	G = 25.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 65.0					

#### Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		805			646			240		264	400	
Lane Group Capacity, c		1187			993			312		438	578	
v/c Ratio, X		0.68			0.65			0.77		0.60	0.69	
Total Green Ratio, g/C		0.46			0.31			0.38		0.38	0.38	
Uniform Delay, d <sub>1</sub>		13.7			19.5			17.5		16.0	16.8	
Progression Factor, PF		1.000			1.000			1.000		1.000	1.000	
Delay Calibration, k		0.25			0.23			0.32		0.19	0.26	
Incremental Delay, d <sub>2</sub>		1.6			1.5			11.1		2.3	3.5	
Initial Queue Delay, d <sub>3</sub>		0.0			0.0			0.0		0.0	0.0	
Control Delay		15.3			21.0			28.6		18.4	20.3	
Lane Group LOS		B			C			C		B	C	
Approach Delay	15.3			21.0			28.6			19.5		
Approach LOS	B			C			C			B		
Intersection Delay	19.4			X <sub>c</sub> = 0.90			Intersection LOS			B		

### HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>Argonne /Rt 22 ramps/US 6</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>11/9/07</i>	Jurisdiction <i>Town of Southeast, NY</i>
Time Period <i>P.M. Peak Hour</i>	Analysis Year <i>No Build Condition</i>
	Project ID <i>Stateline Retail Center</i>

#### Volume and Timing Input

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>	0	1	0	1	1	0	0	2	0	0	2	0
Lane Group		<i>LTR</i>		<i>L</i>	<i>TR</i>			<i>LTR</i>		<i>DefL</i>	<i>TR</i>	
Volume, V (vph)	7	71	268	129	10	168	28	743	222	144	345	2
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.93	0.93	0.93	0.89	0.89	0.89	0.94	0.94	0.94	0.97	0.97	0.97
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A
Start-up Lost Time, I <sub>1</sub>		2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e		2.0		2.0	2.0			2.0		2.0	2.0	
Arrival Type, AT		3		3	3			3		3	3	
Unit Extension, UE		3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I		1.000		1.000	1.000			1.000		1.000	1.000	
Initial Unmet Demand, Q <sub>b</sub>		0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.0		9.0	9.0			12.0		12.0	12.0	
Parking / Grade / Parking	N	0	N	N	-6	N	N	-4	N	N	4	N
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0		0	0			0		0	0	
Min. Time for Pedestrians, G <sub>p</sub>		3.2		3.2	3.2			3.2		3.2	3.2	
Phasing	EW Perm	02	03	04	NS Perm	SB Only	07	08				
Timing	G = 20.0	G =	G =	G =	G = 23.0	G = 2.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					

#### Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		372		145	200			1056		148	358	
Lane Group Capacity, c		614		250	499			1268		415	921	
v/c Ratio, X		0.61		0.58	0.40			0.83		0.36	0.39	
Total Green Ratio, g/C		0.33		0.33	0.33			0.38		0.55	0.50	
Uniform Delay, d <sub>1</sub>		16.7		16.5	15.4			16.8		16.0	9.3	
Progression Factor, PF		1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k		0.19		0.17	0.11			0.37		0.11	0.11	
Incremental Delay, d <sub>2</sub>		1.7		3.4	0.5			4.9		0.5	0.3	
Initial Queue Delay, d <sub>3</sub>		0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay		18.4		19.9	15.9			21.7		16.6	9.6	
Lane Group LOS		B		B	B			C		B	A	
Approach Delay		18.4		17.6				21.7		11.6		
Approach LOS		B		B				C		B		
Intersection Delay		18.3		X <sub>c</sub> = 1.04				Intersection LOS		B		

<b>HCS+™ DETAILED REPORT</b>												
<b>General Information</b>						<b>Site Information</b>						
Analyst <i>WFB + JAG</i>						Intersection <i>Rt 6 (East Main) &amp; 202/22</i>						
Agency or Co. <i>TMA</i>						Area Type <i>All other areas</i>						
Date Performed <i>10/30/2007</i>						Jurisdiction <i>Town of Southeast</i>						
Time Period <i>PM Peak Hour</i>						Analysis Year <i>No Build Condition</i>						
						Project ID <i>Stateline Retail Center</i>						
<b>Volume and Timing Input</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>	0	1			1	1				1		1
Lane Group		LT			T	R				L		R
Volume, V (vph)	69	359			181	691				520		46
% Heavy Vehicles, %HV	4	4			2	2				3		3
Peak-Hour Factor, PHF	0.93	0.93			0.96	0.96				0.92		0.92
Pretimed (P) or Actuated (A)	A	A			A	A				A		A
Start-up Lost Time, I <sub>1</sub>		2.0			2.0	2.0				2.0		2.0
Extension of Effective Green, e		2.0			2.0	2.0				2.0		2.0
Arrival Type, AT		3			3	3				3		3
Unit Extension, UE		3.0			3.0	3.0				3.0		3.0
Filtering/Metering, I		1.000			1.000	1.000				1.000		1.000
Initial Unmet Demand, Q <sub>b</sub>		0.0			0.0	0.0				0.0		0.0
Ped / Bike / RTOR Volumes	0	0		0	0	0				0	0	0
Lane Width		13.0			11.0	11.0				12.0		11.0
Parking / Grade / Parking	N	0	N	N	0	N				N	0	N
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0			0	0				0		0
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2					3.2		
Phasing	EW Perm	02	03	04	SB Only	06	07	08				
Timing	G = 39.0	G =	G =	G =	G = 36.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 85.0						
<b>Lane Group Capacity, Control Delay, and LOS Determination</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		460			189	720				565		50
Lane Group Capacity, c		793			826	1531				742		642
v/c Ratio, X		0.58			0.23	0.47				0.76		0.08
Total Green Ratio, g/C		0.46			0.46	1.00				0.42		0.42
Uniform Delay, d <sub>1</sub>		17.0			13.9	0.0				20.8		14.6
Progression Factor, PF		1.000			1.000	0.950				1.000		1.000
Delay Calibration, k		0.17			0.11	0.11				0.31		0.11
Incremental Delay, d <sub>2</sub>		1.1			0.1	0.2				4.7		0.1
Initial Queue Delay, d <sub>3</sub>		0.0			0.0	0.0				0.0		0.0
Control Delay		18.0			14.0	0.2				25.5		14.7
Lane Group LOS		B			B	A				C		B
Approach Delay	18.0			3.1						24.6		
Approach LOS	B			A						C		
Intersection Delay	13.2			X <sub>c</sub> = 0.67			Intersection LOS			B		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst JAG						Intersection Saw Mill Rd/I-84 EB ramps						
Agency or Co. TMA						Area Type All other areas						
Date Performed 10/28/2007						Jurisdiction Danbury, CT						
Time Period P.M. Peak Hour						Analysis Year No Build Condition						
						Project ID Stateline						
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>	0	1	1					1	1	0	2	
Lane Group		LT	R					T	R	DefL	T	
Volume, V (vph)	50	0	330					486	539	166	482	
% Heavy Vehicles, %HV	1	1	1					0	0	2	2	
Peak-Hour Factor, PHF	0.73	0.73	0.73					0.83	0.83	0.87	0.87	
Pretimed (P) or Actuated (A)	A	A	A					A	A	A	A	
Start-up Lost Time, I <sub>1</sub>		2.0	2.0					2.0	2.0	2.0	2.0	
Extension of Effective Green, e		2.0	2.0					2.0	2.0	2.0	2.0	
Arrival Type, AT		3	3					3	3	5	5	
Unit Extension, UE		3.0	3.0					3.0	3.0	3.0	3.0	
Filtering/Metering, I		1.000	1.000					1.000	1.000	1.000	1.000	
Initial Unmet Demand, Q <sub>b</sub>		0.0	0.0					0.0	0.0	0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0				0	0	150	0	0	
Lane Width		12.0	12.0					11.0	11.0	11.0	11.0	
Parking / Grade / Parking	N	3	N				N	8	N	N	-5	N
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0	0					0	0	0	0	
Min. Time for Pedestrians, G <sub>p</sub>		3.2						3.2			3.2	
Phasing	EB Only	02	03	04	SB Only	NS Perm	07	08				
Timing	G = 23.0	G =	G =	G =	G = 10.0	G = 27.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 75.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		68	452					586	469	191	554	
Lane Group Capacity, c		541	483					635	540	333	1034	
v/c Ratio, X		0.13	0.94					0.92	0.87	0.57	0.54	
Total Green Ratio, g/C		0.31	0.31					0.36	0.36	0.56	0.56	
Uniform Delay, d <sub>1</sub>		18.7	25.3					23.0	22.3	12.8	10.4	
Progression Factor, PF		1.000	1.000					1.000	1.000	0.897	0.152	
Delay Calibration, k		0.11	0.45					0.44	0.40	0.17	0.14	
Incremental Delay, d <sub>2</sub>		0.1	25.8					19.2	14.1	2.4	0.6	
Initial Queue Delay, d <sub>3</sub>		0.0	0.0					0.0	0.0	0.0	0.0	
Control Delay		18.9	51.1					42.2	36.5	13.9	2.1	
Lane Group LOS		B	D					D	D	B	A	
Approach Delay	46.9						39.7			5.1		
Approach LOS	D						D			A		
Intersection Delay	30.2			X <sub>c</sub> = 1.15			Intersection LOS			C		



## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	US 6/Dingle Ridge Road
Agency/Co.	TMA	Jurisdiction	Town of Southeast, NY
Date Performed	12/28/2006	Analysis Year	No Build Condition
Analysis Time Period	P.M. Peak Hour		

Project Description *Stateline*

East/West Street: *US Route 6 & 202 (Danbury Rd)*

North/South Street: *Dingle Ridge Road*

Intersection Orientation: *East-West*

Study Period (hrs): *0.25*

### Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	1	518	51	31	386	1
Peak-Hour Factor, PHF	0.95	0.95	0.95	0.93	0.93	0.93
Hourly Flow Rate, HFR (veh/h)	1	545	53	33	415	1
Percent Heavy Vehicles	1	--	--	1	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	2	0	0	2	0
Configuration	<i>LT</i>		<i>TR</i>	<i>LT</i>		<i>TR</i>
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)	58	0	26	3	0	0
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.38	0.38	0.38
Hourly Flow Rate, HFR (veh/h)	64	0	28	7	0	0
Percent Heavy Vehicles	1	1	1	1	1	1
Percent Grade (%)	2			2		
Flared Approach		Y			N	
Storage		1			0	
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration		<i>LTR</i>			<i>LTR</i>	

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>	<i>LT</i>		<i>LTR</i>			<i>LTR</i>	
v (veh/h)	1	33		92			7	
C (m) (veh/h)	1147	982		358			278	
v/c	0.00	0.03		0.26			0.03	
95% queue length	0.00	0.10		1.01			0.08	
Control Delay (s/veh)	8.1	8.8		20.1			18.3	
LOS	A	A		C			C	
Approach Delay (s/veh)	--	--	20.1			18.3		
Approach LOS	--	--	C			C		

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	Joe's Hill Rd/US Rt 6 & 202
Agency/Co.	TMA	Jurisdiction	Town of Southeast, NY
Date Performed	12/28/2006	Analysis Year	No Build Condition
Analysis Time Period	P.M. Peak Hour		

Project Description *Stateline*

East/West Street: *US Route 6 & 202*

North/South Street: *Joe's Hill Road*

Intersection Orientation: *East-West*

Study Period (hrs): *0.25*

### Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	51	571			362	34
Peak-Hour Factor, PHF	0.95	0.95	1.00	1.00	0.89	0.89
Hourly Flow Rate, HFR (veh/h)	53	601	0	0	406	38
Percent Heavy Vehicles	1	--	--	0	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	2	0	0	2	0
Configuration	<i>LT</i>	<i>T</i>			<i>T</i>	<i>TR</i>
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)				9		23
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.68	1.00	0.68
Hourly Flow Rate, HFR (veh/h)	0	0	0	13	0	33
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			2		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					<i>LR</i>	

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>						<i>LR</i>	
v (veh/h)	53						46	
C (m) (veh/h)	1120						537	
v/c	0.05						0.09	
95% queue length	0.15						0.28	
Control Delay (s/veh)	8.4						12.3	
LOS	<i>A</i>						<i>B</i>	
Approach Delay (s/veh)	--	--					12.3	
Approach LOS	--	--					<i>B</i>	

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	US Rte 6/NYS 121
Agency/Co.	TMA	Jurisdiction	Town of Southeast, NY
Date Performed	12/28/2006	Analysis Year	No Build Condition
Analysis Time Period	P.M. Peak Hour		

Project Description *Stateline*

East/West Street: *US Route 6 & 202*

North/South Street: *NYS Rte 121 Peach Lake*

Intersection Orientation: *East-West*

Study Period (hrs): *0.25*

### Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)		508	197	59	352	
Peak-Hour Factor, PHF	1.00	0.85	0.85	0.97	0.97	1.00
Hourly Flow Rate, HFR (veh/h)	0	597	231	60	362	0
Percent Heavy Vehicles	0	--	--	1	--	--
Median Type	<i>Undivided</i>					
RT Channelized			1			0
Lanes	0	2	1	0	2	0
Configuration		T	R	LT	T	
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)	256		54			
Peak-Hour Factor, PHF	0.95	1.00	0.95	1.00	1.00	1.00
Hourly Flow Rate, HFR (veh/h)	269	0	56	0	0	0
Percent Heavy Vehicles	1	0	1	0	0	0
Percent Grade (%)		-4			0	
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	1	0	1	0	0	0
Configuration	L		R			

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT	L		R			
v (veh/h)		60	269		56			
C (m) (veh/h)		983	264		701			
v/c		0.06	1.02		0.08			
95% queue length		0.19	10.36		0.26			
Control Delay (s/veh)		8.9	102.1		10.6			
LOS		A	F		B			
Approach Delay (s/veh)	--	--	86.3					
Approach LOS	--	--	F					

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	Route 121/ I-84 off-ramp
Agency/Co.	TMA	Jurisdiction	Town of Southeast
Date Performed	12/28/2006	Analysis Year	No Build Condition
Analysis Time Period	P.M. Peak Hour		

Project Description: <i>Stateline</i>	
East/West Street: <i>I-84 off-ramp</i>	North/South Street: <i>Peach Lake Rd (NYS Rt 121)</i>
Intersection Orientation: <i>North-South</i>	Study Period (hrs): <i>0.25</i>

Vehicle Volumes and Adjustments						
Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)		254			259	
Peak-Hour Factor, PHF	1.00	0.92	1.00	1.00	0.88	1.00
Hourly Flow Rate, HFR (veh/h)	0	276	0	0	294	0
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration		T			T	
Upstream Signal		0			0	

Minor Street	Eastbound			Westbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)				91		56
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.83	1.00	0.83
Hourly Flow Rate, HFR (veh/h)	0	0	0	109	0	67
Percent Heavy Vehicles	0	0	0	1	0	1
Percent Grade (%)	0			-3		
Flared Approach		N			Y	
Storage		0			3	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					LR	

Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration				LR				
v (veh/h)				176				
C (m) (veh/h)				783				
v/c				0.22				
95% queue length				0.86				
Control Delay (s/veh)				12.9				
LOS				B				
Approach Delay (s/veh)	--	--	12.9					
Approach LOS	--	--	B					

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	NYS Route 121/ I-84 on-ramp
Agency/Co.	TMA	Jurisdiction	Town of Southeast
Date Performed	12/28/2006	Analysis Year	No Build Condition
Analysis Time Period	P.M. Peak Hour		
Project Description <i>Stateline</i>			
East/West Street: <i>I-84 on-ramp</i>		North/South Street: <i>NYS Rt. 121 Peach Lake Rd.</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

Vehicle Volumes and Adjustments						
Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)		252	210	92	258	
Peak-Hour Factor, PHF	1.00	0.97	0.97	0.82	0.82	1.00
Hourly Flow Rate, HFR (veh/h)	0	259	216	112	314	0
Percent Heavy Vehicles	0	--	--	2	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration			<i>TR</i>	<i>LT</i>		
Upstream Signal		0			0	

Minor Street	Eastbound			Westbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)						
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration						

Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		<i>LT</i>						
v (veh/h)		112						
C (m) (veh/h)		1087						
v/c		0.10						
95% queue length		0.34						
Control Delay (s/veh)		8.7						
LOS		<i>A</i>						
Approach Delay (s/veh)	--	--						
Approach LOS	--	--						

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	WFB + JAG		Intersection	Route 6 & Sodom				
Agency/Co.	TMA		Jurisdiction	Town of Southeast				
Date Performed	10/30/2007		Analysis Year	No Build Condition				
Analysis Time Period	P.M. Peak Hour							
Project Description <i>Stateline</i>								
East/West Street: <i>US Route 6/202/22</i>			North/South Street: <i>Sodom Road/Driveway</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	0	390	2	22	917	107		
Peak-Hour Factor, PHF	0.94	0.94	0.94	0.98	0.98	0.98		
Hourly Flow Rate, HFR (veh/h)	0	414	2	22	935	109		
Percent Heavy Vehicles	2	--	--	3	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	2	0	0	1	1		
Configuration	<i>LT</i>		<i>TR</i>	<i>LT</i>		<i>R</i>		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	22	9	24	54	2	0		
Peak-Hour Factor, PHF	0.66	0.66	0.66	0.79	0.79	0.79		
Hourly Flow Rate, HFR (veh/h)	33	13	36	68	2	0		
Percent Heavy Vehicles	8	8	8	2	2	2		
Percent Grade (%)	0			0				
Flared Approach		<i>N</i>			<i>N</i>			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	1	1	0	1	0		
Configuration	<i>LT</i>		<i>R</i>		<i>LTR</i>			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound		Southbound			
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>	<i>LT</i>	<i>LT</i>		<i>R</i>		<i>LTR</i>	
v (veh/h)	0	22	46		36		70	
C (m) (veh/h)	662	1132	89		780		119	
v/c	0.00	0.02	0.52		0.05		0.59	
95% queue length	0.00	0.06	2.26		0.14		2.91	
Control Delay (s/veh)	10.4	8.2	82.4		9.8		71.4	
LOS	<i>B</i>	<i>A</i>	<i>F</i>		<i>A</i>		<i>F</i>	
Approach Delay (s/veh)	--	--	50.6			71.4		
Approach LOS	--	--	<i>F</i>			<i>F</i>		

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	WFB + JAG		Intersection	US Route 6 and CR 50				
Agency/Co.	TMA		Jurisdiction	Town of Southeast				
Date Performed	10/30/2007		Analysis Year	No Build Condition				
Analysis Time Period	P.M. Peak Hour							
Project Description <i>Stateline Retail Center</i>								
East/West Street: <i>US Route 6/202/22</i>			North/South Street: <i>County Road 50</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	128	381			939	0		
Peak-Hour Factor, PHF	0.98	0.98	1.00	1.00	0.98	0.98		
Hourly Flow Rate, HFR (veh/h)	130	388	0	0	958	0		
Percent Heavy Vehicles	3	--	--	0	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	1	1	0	0	1	0		
Configuration	L	T				TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				7		134		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.90	1.00	0.90		
Hourly Flow Rate, HFR (veh/h)	0	0	0	7	0	148		
Percent Heavy Vehicles	0	0	0	1	0	1		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	1	0	1		
Configuration				L		R		
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L					L		R
v (veh/h)	130					7		148
C (m) (veh/h)	714					95		313
v/c	0.18					0.07		0.47
95% queue length	0.66					0.23		2.41
Control Delay (s/veh)	11.2					45.9		26.4
LOS	B					E		D
Approach Delay (s/veh)	--	--				27.3		
Approach LOS	--	--				D		

### HCS+™ DETAILED REPORT

General Information	Site Information
Analyst            JAG	Intersection     I-84 WB ramps/Saw Mill
Agency or Co.   TMA	Area Type        All other areas
Date Performed  10/29/2007	Jurisdiction     Danbury, CT
Time Period      Saturday Peak Hour	Analysis Year    No Build Condition
	Project ID        Stateline

#### Volume and Timing Input

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>				1	1	0	1	1			2	0
Lane Group				L	LTR		L	T			TR	
Volume, V (vph)				86	7	76	198	133			153	36
% Heavy Vehicles, %HV				0	0	0	1	1			0	0
Peak-Hour Factor, PHF				0.70	0.70	0.70	0.86	0.86			0.81	0.81
Pretimed (P) or Actuated (A)				A	A	A	A	A			A	A
Start-up Lost Time, I <sub>1</sub>				2.0	2.0		2.0	2.0			2.0	
Extension of Effective Green, e				2.0	2.0		2.0	2.0			2.0	
Arrival Type, AT				3	3		5	5			5	
Unit Extension, UE				3.0	3.0		3.0	3.0			3.0	
Filtering/Metering, I				1.000	1.000		1.000	1.000			1.000	
Initial Unmet Demand, Q <sub>b</sub>				0.0	0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes				0	0	40	0	0		0	0	0
Lane Width				12.0	12.0		11.0	11.0			12.0	
Parking / Grade / Parking				N	-1	N	N	1	N	N	-1	N
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>				0	0		0	0			0	
Min. Time for Pedestrians, G <sub>p</sub>				3.2			3.2			3.2		
Phasing	WB Only	02	03	04	NB Only	SB Only	07	08				
Timing	G = 11.0	G =	G =	G =	G = 26.0	G = 23.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 75.0						

#### Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v				98	86		230	155			233	
Lane Group Capacity, c				266	252		596	627			1083	
v/c Ratio, X				0.37	0.34		0.39	0.25			0.22	
Total Green Ratio, g/C				0.15	0.15		0.35	0.35			0.31	
Uniform Delay, d <sub>1</sub>				28.9	28.7		18.5	17.5			19.3	
Progression Factor, PF				1.000	1.000		0.646	0.646			0.705	
Delay Calibration, k				0.11	0.11		0.11	0.11			0.11	
Incremental Delay, d <sub>2</sub>				0.9	0.8		0.4	0.2			0.1	
Initial Queue Delay, d <sub>3</sub>				0.0	0.0		0.0	0.0			0.0	
Control Delay				29.7	29.6		12.4	11.5			13.7	
Lane Group LOS				C	C		B	B			B	
Approach Delay				29.7			12.0			13.7		
Approach LOS				C			B			B		
Intersection Delay	16.6			X <sub>c</sub> = 0.32			Intersection LOS			B		



### HCS+™ DETAILED REPORT

General Information	Site Information
Analyst            JAG	Intersection      US 6 and Saw Mill Rd
Agency or Co.   TMA	Area Type        All other areas
Date Performed  10/29/2007	Jurisdiction     Danbury, CT
Time Period      Saturday Peak Hour	Analysis Year    No Build Condition
	Project ID        Stateline

#### Volume and Timing Input

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>		2	0	1	1		2		0			
Lane Group		TR		L	T		L	LR				
Volume, V (vph)		219	109	89	274		110		89			
% Heavy Vehicles, %HV		0	0	0	0		0		0			
Peak-Hour Factor, PHF		0.92	0.92	0.94	0.94		0.83		0.83			
Pretimed (P) or Actuated (A)		A	A	A	A		A		A			
Start-up Lost Time, I <sub>1</sub>		2.0		2.0	2.0		2.0	2.0				
Extension of Effective Green, e		2.0		2.0	2.0		2.0	2.0				
Arrival Type, AT		3		3	3		5	3				
Unit Extension, UE		3.0		3.0	3.0		3.0	3.0				
Filtering/Metering, I		1.000		1.000	1.000		1.000	1.000				
Initial Unmet Demand, Q <sub>b</sub>		0.0		0.0	0.0		0.0	0.0				
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0			
Lane Width		13.0		13.0	14.0		11.0	11.0				
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N			
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0		0	0		0	0				
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2			3.2				
Phasing	EW Perm	WB Only	03	04	NB Only	06	07	08				
Timing	G = 15.0	G = 8.0	G =	G =	G = 37.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 75.0					

#### Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		356		95	291		73	167				
Lane Group Capacity, c		710		582	757		1671	805				
v/c Ratio, X		0.50		0.16	0.38		0.04	0.21				
Total Green Ratio, g/C		0.20		0.41	0.37		0.49	0.49				
Uniform Delay, d <sub>1</sub>		26.7		15.5	17.2		9.8	10.7				
Progression Factor, PF		1.000		1.000	1.000		0.351	1.000				
Delay Calibration, k		0.11		0.11	0.11		0.11	0.11				
Incremental Delay, d <sub>2</sub>		0.6		0.1	0.3		0.0	0.1				
Initial Queue Delay, d <sub>3</sub>		0.0		0.0	0.0		0.0	0.0				
Control Delay		27.2		15.6	17.5		3.5	10.9				
Lane Group LOS		C		B	B		A	B				
Approach Delay		27.2			17.0			8.6				
Approach LOS		C			B			A				
Intersection Delay		18.7			X <sub>c</sub> = 0.28			Intersection LOS			B	

## HCS+™ DETAILED REPORT

General Information				Site Information			
Analyst	JAG			Intersection	US 6, Starr/ NYS 22 ramp		
Agency or Co.	TMA			Area Type	All other areas		
Date Performed	12/29/2006			Jurisdiction	Town of Southeast		
Time Period	Saturday Peak Hour			Analysis Year	No Build Condition		
				Project ID	Stateline		

## Volume and Timing Input

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>	0	2	0	0	2	0	0	1	0	1	1	0
Lane Group		LTR			LTR			LTR		L	TR	
Volume, V (vph)	228	387	92	31	364	76	115	34	9	69	31	166
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.95	0.95	0.95	0.93	0.93	0.93	0.95	0.95	0.95	0.76	0.76	0.76
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A
Start-up Lost Time, I <sub>1</sub>		2.0			2.0			2.0		2.0	2.0	
Extension of Effective Green, e		2.0			2.0			2.0		2.0	2.0	
Arrival Type, AT		3			3			3		3	3	
Unit Extension, UE		3.0			3.0			3.0		3.0	3.0	
Filtering/Metering, I		1.000			1.000			1.000		1.000	1.000	
Initial Unmet Demand, Q <sub>b</sub>		0.0			0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		12.0			12.0			15.0		10.0	9.0	
Parking / Grade / Parking	N	5	N	N	-2	N	N	-6	N	N	-5	N
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0			0			0		0	0	
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2			3.2		3.2	3.2	
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 35.0	G =	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 65.0					

## Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		744			506			166		91	259	
Lane Group Capacity, c		1244			1678			290		378	466	
v/c Ratio, X		0.60			0.30			0.57		0.24	0.56	
Total Green Ratio, g/C		0.54			0.54			0.31		0.31	0.31	
Uniform Delay, d <sub>1</sub>		10.2			8.3			18.9		16.8	18.8	
Progression Factor, PF		1.000			1.000			1.000		1.000	1.000	
Delay Calibration, k		0.19			0.11			0.17		0.11	0.15	
Incremental Delay, d <sub>2</sub>		0.8			0.1			2.7		0.3	1.5	
Initial Queue Delay, d <sub>3</sub>		0.0			0.0			0.0		0.0	0.0	
Control Delay		11.0			8.4			21.6		17.2	20.3	
Lane Group LOS		B			A			C		B	C	
Approach Delay	11.0			8.4			21.6			19.5		
Approach LOS	B			A			C			B		
Intersection Delay	12.9			X <sub>c</sub> = 0.59			Intersection LOS			B		

<b>HCS+™ DETAILED REPORT</b>												
<b>General Information</b>						<b>Site Information</b>						
Analyst	JAG					Intersection	Argonne /Rt 22 ramps/US 6					
Agency or Co.	TMA					Area Type	All other areas					
Date Performed	11/12/07					Jurisdiction	Town of Southeast, NY					
Time Period	Saturday Peak Hour					Analysis Year	No Build Condition					
						Project ID	Stateline retail Center					
<b>Volume and Timing Input</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>i</sub>	0	1	0	1	1	0	0	2	0	0	2	0
Lane Group	LTR			L			TR			LTR		
Volume, V (vph)	5	59	221	120	5	161	13	491	172	165	384	20
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.84	0.84	0.84	0.86	0.86	0.86	0.91	0.91	0.91
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A
Start-up Lost Time, I <sub>1</sub>		2.0		2.0	2.0			2.0			2.0	
Extension of Effective Green, e		2.0		2.0	2.0			2.0			2.0	
Arrival Type, AT		3		3	3			3			3	
Unit Extension, UE		3.0		3.0	3.0			3.0			3.0	
Filtering/Metering, I		1.000		1.000	1.000			1.000			1.000	
Initial Unmet Demand, Q <sub>b</sub>		0.0		0.0	0.0			0.0			0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.0		9.0	9.0			12.0			12.0	
Parking / Grade / Parking	N	0	N	N	-6	N	N	-4	N	N	4	N
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0		0	0			0			0	
Min. Time for Pedestrians, G <sub>p</sub>	3.2			3.2			3.2			3.2		
Phasing	EW Perm	02	03	04	NS Perm	SB Only	07	08				
Timing	G = 20.0	G =	G =	G =	G = 23.0	G = 2.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
<b>Lane Group Capacity, Control Delay, and LOS Determination</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		318		143	198			786			625	
Lane Group Capacity, c		614		283	497			1265			1169	
v/c Ratio, X		0.52		0.51	0.40			0.62			0.53	
Total Green Ratio, g/C		0.33		0.33	0.33			0.38			0.50	
Uniform Delay, d <sub>1</sub>		16.1		16.0	15.4			15.0			10.2	
Progression Factor, PF		1.000		1.000	1.000			1.000			1.000	
Delay Calibration, k		0.12		0.11	0.11			0.20			0.14	
Incremental Delay, d <sub>2</sub>		0.8		1.5	0.5			1.0			0.5	
Initial Queue Delay, d <sub>3</sub>		0.0		0.0	0.0			0.0			0.0	
Control Delay		16.9		17.5	15.9			15.9			10.7	
Lane Group LOS		B		B	B			B			B	
Approach Delay	16.9			16.6			15.9			10.7		
Approach LOS	B			B			B			B		
Intersection Delay	14.6			X <sub>c</sub> = 0.72			Intersection LOS			B		

<b>HCS+™ DETAILED REPORT</b>												
<b>General Information</b>						<b>Site Information</b>						
Analyst <i>WFB + JAG</i>						Intersection <i>Rt 6 (East Main) &amp; 202/22</i>						
Agency or Co. <i>TMA</i>						Area Type <i>All other areas</i>						
Date Performed <i>10/30/2007</i>						Jurisdiction <i>Town of Southeast</i>						
Time Period <i>Saturday Peak Hour</i>						Analysis Year <i>No Build Condition</i>						
						Project ID <i>Stateline Retail Center</i>						
<b>Volume and Timing Input</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>	0	1			1	1				1		1
Lane Group		LT			T	R				L		R
Volume, V (vph)	49	181			138	434				453		38
% Heavy Vehicles, %HV	5	5			1	1				2		2
Peak-Hour Factor, PHF	0.91	0.91			0.98	0.98				0.95		0.95
Pretimed (P) or Actuated (A)	A	A			A	A				A		A
Start-up Lost Time, I <sub>1</sub>		2.0			2.0	2.0				2.0		2.0
Extension of Effective Green, e		2.0			2.0	2.0				2.0		2.0
Arrival Type, AT		3			3	3				3		3
Unit Extension, UE		3.0			3.0	3.0				3.0		3.0
Filtering/Metering, I		1.000			1.000	1.000				1.000		1.000
Initial Unmet Demand, Q <sub>b</sub>		0.0			0.0	0.0				0.0		0.0
Ped / Bike / RTOR Volumes	0	0		0	0	0				0	0	0
Lane Width		13.0			11.0	11.0				12.0		11.0
Parking / Grade / Parking	N	0	N	N	0	N				N	0	N
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0			0	0				0		0
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2					3.2		
Phasing	EW Perm	02	03	04	SB Only	06	07	08				
Timing	G = 39.0	G =	G =	G =	G = 36.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 85.0					
<b>Lane Group Capacity, Control Delay, and LOS Determination</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		253			141	443				477		40
Lane Group Capacity, c		782			834	1546				750		648
v/c Ratio, X		0.32			0.17	0.29				0.64		0.06
Total Green Ratio, g/C		0.46			0.46	1.00				0.42		0.42
Uniform Delay, d <sub>1</sub>		14.6			13.5	0.0				19.3		14.5
Progression Factor, PF		1.000			1.000	0.950				1.000		1.000
Delay Calibration, k		0.11			0.11	0.11				0.22		0.11
Incremental Delay, d <sub>2</sub>		0.2			0.1	0.1				1.8		0.0
Initial Queue Delay, d <sub>3</sub>		0.0			0.0	0.0				0.0		0.0
Control Delay		14.9			13.6	0.1				21.1		14.5
Lane Group LOS		B			B	A				C		B
Approach Delay	14.9			3.4						20.6		
Approach LOS	B			A						C		
Intersection Delay	12.1			X <sub>c</sub> = 0.47			Intersection LOS			B		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst JAG Agency or Co. TMA Date Performed 10/28/2007 Time Period Saturday Peak Hour						Intersection Saw Mill Rd / I-84 EB ramps Area Type All other areas Jurisdiction Danbury, CT Analysis Year No Build Condition Project ID Stateline						
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>	0	1	1					1	1	0	2	
Lane Group		LT	R					T	R		LT	
Volume, V (vph)	60	0	240					261	225	78	282	
% Heavy Vehicles, %HV	0	0	0					2	2	0	0	
Peak-Hour Factor, PHF	0.84	0.84	0.84					0.86	0.86	0.92	0.92	
Pretimed (P) or Actuated (A)	A	A	A					A	A	A	A	
Start-up Lost Time, I <sub>1</sub>		2.0	2.0					2.0	2.0		2.0	
Extension of Effective Green, e		2.0	2.0					2.0	2.0		2.0	
Arrival Type, AT		3	3					3	3		5	
Unit Extension, UE		3.0	3.0					3.0	3.0		3.0	
Filtering/Metering, I		1.000	1.000					1.000	1.000		1.000	
Initial Unmet Demand, Q <sub>b</sub>		0.0	0.0					0.0	0.0		0.0	
Ped / Bike / RTOR Volumes	0	0	0				0	0	0	0	0	
Lane Width		12.0	12.0					11.0	11.0		11.0	
Parking / Grade / Parking	N	3	N				N	8	N	N	-5	N
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0	0					0	0		0	
Min. Time for Pedestrians, G <sub>p</sub>		3.2						3.2			3.2	
Phasing	EB Only	02	03	04	SB Only	NB Only	07	08				
Timing	G = 10.0	G =	G =	G =	G = 34.0	G = 16.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 75.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		71	286					303	262		392	
Lane Group Capacity, c		238	658					369	313		1608	
v/c Ratio, X		0.30	0.43					0.82	0.84		0.24	
Total Green Ratio, g/C		0.13	0.41					0.21	0.21		0.45	
Uniform Delay, d <sub>1</sub>		29.3	15.7					28.1	28.3		12.6	
Progression Factor, PF		1.000	1.000					1.000	1.000		0.447	
Delay Calibration, k		0.11	0.11					0.36	0.37		0.11	
Incremental Delay, d <sub>2</sub>		0.7	0.5					13.8	17.7		0.1	
Initial Queue Delay, d <sub>3</sub>		0.0	0.0					0.0	0.0		0.0	
Control Delay		30.0	16.2					41.9	46.0		5.7	
Lane Group LOS		C	B					D	D		A	
Approach Delay	18.9						43.8			5.7		
Approach LOS	B						D			A		
Intersection Delay	25.7			X <sub>c</sub> = 0.41			Intersection LOS			C		

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	US 6/Dingle Ridge Road
Agency/Co.	TMA	Jurisdiction	Town of Southeast, NY
Date Performed	12/29/2006	Analysis Year	No Build Condition
Analysis Time Period	Saturday Peak Hour		

Project Description *Stateline*

East/West Street: *US Route 6 & 202 (Danbury Rd)*

North/South Street: *Dingle Ridge Road*

Intersection Orientation: *East-West*

Study Period (hrs): *0.25*

### Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	0	278	37	38	344	0
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.89	0.89	0.89
Hourly Flow Rate, HFR (veh/h)	0	330	44	42	386	0
Percent Heavy Vehicles	1	--	--	1	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	2	0	0	2	0
Configuration	LT		TR	LT		TR
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)	33	0	34	0	0	1
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.25	0.25	0.25
Hourly Flow Rate, HFR (veh/h)	37	0	38	0	0	4
Percent Heavy Vehicles	1	1	1	0	0	0
Percent Grade (%)	2			2		
Flared Approach		Y			N	
Storage		1			0	
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration		LTR			LTR	

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT	LT		LTR			LTR	
v (veh/h)	0	42		75			4	
C (m) (veh/h)	1176	1188		722			822	
v/c	0.00	0.04		0.10			0.00	
95% queue length	0.00	0.11		0.35			0.01	
Control Delay (s/veh)	8.1	8.1		12.9			9.4	
LOS	A	A		B			A	
Approach Delay (s/veh)	--	--		12.9			9.4	
Approach LOS	--	--		B			A	

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	Joe's Hill Road / US 6
Agency/Co.	TMA	Jurisdiction	Town of Southeast, NY
Date Performed	12/29/2006	Analysis Year	No Build Condition
Analysis Time Period	Saturday Peak Hour		

Project Description *Stateline*

East/West Street: *US Route 6 & 202*

North/South Street: *Joe's Hill Road*

Intersection Orientation: *East-West*

Study Period (hrs): *0.25*

### Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	13	301			356	19
Peak-Hour Factor, PHF	0.83	0.83	1.00	1.00	0.87	0.87
Hourly Flow Rate, HFR (veh/h)	15	362	0	0	409	21
Percent Heavy Vehicles	1	--	--	0	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	2	0	0	2	0
Configuration	<i>LT</i>	<i>T</i>			<i>T</i>	<i>TR</i>
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)				11		4
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.65	1.00	0.65
Hourly Flow Rate, HFR (veh/h)	0	0	0	16	0	6
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			2		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					<i>LR</i>	

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>						<i>LR</i>	
v (veh/h)	15						22	
C (m) (veh/h)	1133						474	
v/c	0.01						0.05	
95% queue length	0.04						0.15	
Control Delay (s/veh)	8.2						13.0	
LOS	<i>A</i>						<i>B</i>	
Approach Delay (s/veh)	--	--					13.0	
Approach LOS	--	--					<i>B</i>	

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	US Rte 6/NYS 121
Agency/Co.	TMA	Jurisdiction	Town of Southeast, NY
Date Performed	12/29/2006	Analysis Year	No Build Condition
Analysis Time Period	Saturday Peak Hour		

Project Description <i>Stateline</i>	
East/West Street: <i>US Route 6 &amp; 202</i>	North/South Street: <i>NYS Rte 121 Peach Lake</i>
Intersection Orientation: <i>East-West</i>	Study Period (hrs): <i>0.25</i>

Vehicle Volumes and Adjustments						
Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)		302	194	53	309	
Peak-Hour Factor, PHF	1.00	0.80	0.80	0.90	0.90	1.00
Hourly Flow Rate, HFR (veh/h)	0	377	242	58	343	0
Percent Heavy Vehicles	0	--	--	1	--	--
Median Type	<i>Undivided</i>					
RT Channelized			1			0
Lanes	0	2	1	0	2	0
Configuration		T	R	LT	T	
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)	179		29			
Peak-Hour Factor, PHF	0.94	1.00	0.94	1.00	1.00	1.00
Hourly Flow Rate, HFR (veh/h)	190	0	30	0	0	0
Percent Heavy Vehicles	2	0	2	0	0	0
Percent Grade (%)		-4		0		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	1	0	1	0	0	0
Configuration	L		R			

Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT	L		R			
v (veh/h)		58	190		30			
C (m) (veh/h)		1185	375		822			
v/c		0.05	0.51		0.04			
95% queue length		0.15	2.75		0.11			
Control Delay (s/veh)		8.2	24.1		9.5			
LOS		A	C		A			
Approach Delay (s/veh)	--	--	22.1					
Approach LOS	--	--	C					



## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	Route 121/ I-84 off-ramp
Agency/Co.	TMA	Jurisdiction	Town of Southeast
Date Performed	12/29/2006	Analysis Year	No Build Condition
Analysis Time Period	Saturday Peak Hour		

Project Description: <i>Stateline</i>	
East/West Street: <i>I-84 off-ramp</i>	North/South Street: <i>Peach Lake Rd (NYS Rt 121)</i>
Intersection Orientation: <i>North-South</i>	Study Period (hrs): <i>0.25</i>

Vehicle Volumes and Adjustments						
Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)		136			222	
Peak-Hour Factor, PHF	1.00	0.79	1.00	1.00	0.93	1.00
Hourly Flow Rate, HFR (veh/h)	0	172	0	0	238	0
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration		T			T	
Upstream Signal		0			0	

Minor Street	Eastbound			Westbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)				111		71
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.89	1.00	0.89
Hourly Flow Rate, HFR (veh/h)	0	0	0	124	0	79
Percent Heavy Vehicles	0	0	0	2	0	2
Percent Grade (%)	0			-3		
Flared Approach		N			Y	
Storage		0			3	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					LR	

Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration				LR				
v (veh/h)				203				
C (m) (veh/h)				979				
v/c				0.21				
95% queue length				0.78				
Control Delay (s/veh)				11.4				
LOS				B				
Approach Delay (s/veh)	--	--	11.4					
Approach LOS	--	--	B					

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	NYS Route 121/ I-84 on-ramp
Agency/Co.	TMA	Jurisdiction	Town of Southeast
Date Performed	12/29/2006	Analysis Year	No Build Condition
Analysis Time Period	Saturday Peak Hour		
Project Description <i>Stateline</i>			
East/West Street: <i>I-84 on-ramp</i>		North/South Street: <i>NYS Rt. 121 Peach Lake Rd.</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

Vehicle Volumes and Adjustments						
Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)		133	138	76	245	
Peak-Hour Factor, PHF	1.00	0.83	0.83	0.94	0.94	1.00
Hourly Flow Rate, HFR (veh/h)	0	160	166	80	260	0
Percent Heavy Vehicles	0	--	--	1	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration			<i>TR</i>	<i>LT</i>		
Upstream Signal		0			0	

Minor Street	Eastbound			Westbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)						
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration						

Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		<i>LT</i>						
v (veh/h)		80						
C (m) (veh/h)		1239						
v/c		0.06						
95% queue length		0.21						
Control Delay (s/veh)		8.1						
LOS		<i>A</i>						
Approach Delay (s/veh)	--	--						
Approach LOS	--	--						

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	WFB + JAG		Intersection	Route 6 & Sodom				
Agency/Co.	TMA		Jurisdiction	Town of Southeast				
Date Performed	10/30/2007		Analysis Year	No Build Condition				
Analysis Time Period	Saturday Peak Hour							
Project Description <i>Stateline Retail Center</i>								
East/West Street: <i>US Route 6/202/22</i>			North/South Street: <i>Sodom Road/Driveway</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	0	379	29	8	553	54		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.89	0.89	0.89		
Hourly Flow Rate, HFR (veh/h)	0	411	31	8	621	60		
Percent Heavy Vehicles	2	--	--	1	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	2	0	0	1	1		
Configuration	<i>LT</i>		<i>TR</i>	<i>LT</i>		<i>R</i>		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	7	0	26	52	0	0		
Peak-Hour Factor, PHF	0.80	0.80	0.80	0.78	0.78	0.78		
Hourly Flow Rate, HFR (veh/h)	8	0	32	66	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	0			0				
Flared Approach		<i>N</i>			<i>N</i>			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	1	1	0	1	0		
Configuration	<i>LT</i>		<i>R</i>		<i>LTR</i>			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound		Southbound			
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>	<i>LT</i>	<i>LT</i>		<i>R</i>		<i>LTR</i>	
v (veh/h)	0	8	8		32		66	
C (m) (veh/h)	907	1122	170		789		248	
v/c	0.00	0.01	0.05		0.04		0.27	
95% queue length	0.00	0.02	0.15		0.13		1.04	
Control Delay (s/veh)	9.0	8.2	27.2		9.8		24.7	
LOS	<i>A</i>	<i>A</i>	<i>D</i>		<i>A</i>		<i>C</i>	
Approach Delay (s/veh)	--	--	13.2			24.7		
Approach LOS	--	--	<i>B</i>			<i>C</i>		

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	WFB + JAG		Intersection	US Route 6 and CR 50				
Agency/Co.	TMA		Jurisdiction	Town of Southeast				
Date Performed	10/30/2007		Analysis Year	No Build Condition				
Analysis Time Period	Saturday Peak Hour							
Project Description <i>Stateline Retail Center</i>								
East/West Street: <i>US Route 6/202/22</i>			North/South Street: <i>County Road 50</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	70	423			495	6		
Peak-Hour Factor, PHF	0.85	0.85	1.00	1.00	0.86	0.86		
Hourly Flow Rate, HFR (veh/h)	82	497	0	0	575	6		
Percent Heavy Vehicles	1	--	--	0	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	1	1	0	0	1	0		
Configuration	L	T				TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				10		115		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.85	1.00	0.85		
Hourly Flow Rate, HFR (veh/h)	0	0	0	11	0	135		
Percent Heavy Vehicles	0	0	0	2	0	2		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	1	0	1		
Configuration				L		R		
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L					L		R
v (veh/h)	82					11		135
C (m) (veh/h)	998					178		516
v/c	0.08					0.06		0.26
95% queue length	0.27					0.20		1.04
Control Delay (s/veh)	8.9					26.6		14.4
LOS	A					D		B
Approach Delay (s/veh)	--	--				15.3		
Approach LOS	--	--				C		

<b>HCS+™ DETAILED REPORT</b>												
<b>General Information</b>						<b>Site Information</b>						
Analyst JAG						Intersection I-84 WB ramps/Saw Mill						
Agency or Co. TMA						Area Type All other areas						
Date Performed 10/28/2007						Jurisdiction Danbury, CT						
Time Period A.M. Peak Hour						Analysis Year Build Condition						
						Project ID Stateline						
<b>Volume and Timing Input</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>l</sub>				1	1	0	1	1			2	0
Lane Group				L	LTR		L	T			TR	
Volume, V (vph)				460	5	430	259	191			177	80
% Heavy Vehicles, %HV				3	3	3	1	1			1	1
Peak-Hour Factor, PHF				0.92	0.92	0.92	0.86	0.86			0.92	0.92
Pretimed (P) or Actuated (A)				A	A	A	A	A			A	A
Start-up Lost Time, I <sub>1</sub>				2.0	2.0		2.0	2.0			2.0	
Extension of Effective Green, e				2.0	2.0		2.0	2.0			2.0	
Arrival Type, AT				3	3		5	5			5	
Unit Extension, UE				3.0	3.0		3.0	3.0			3.0	
Filtering/Metering, I				1.000	1.000		1.000	1.000			1.000	
Initial Unmet Demand, Q <sub>b</sub>				0.0	0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes				0	0	215	0	0		0	0	0
Lane Width				12.0	12.0		11.0	11.0			12.0	
Parking / Grade / Parking				N	-1	N	N	1	N	N	-1	N
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>				0	0		0	0			0	
Min. Time for Pedestrians, G <sub>p</sub>				3.2			3.2			3.2		
Phasing	WB Only	02	03	04	NB Only	SB Only	07	08				
Timing	G = 19.0	G =	G =	G =	G = 18.0	G = 23.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 75.0						
<b>Lane Group Capacity, Control Delay, and LOS Determination</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v				400	339		301	222			279	
Lane Group Capacity, c				446	415		413	434			1052	
v/c Ratio, X				0.90	0.82		0.73	0.51			0.27	
Total Green Ratio, g/C				0.25	0.25		0.24	0.24			0.31	
Uniform Delay, d <sub>1</sub>				27.1	26.4		26.3	24.7			19.6	
Progression Factor, PF				1.000	1.000		0.789	0.789			0.705	
Delay Calibration, k				0.42	0.36		0.29	0.12			0.11	
Incremental Delay, d <sub>2</sub>				20.4	12.1		6.4	1.0			0.1	
Initial Queue Delay, d <sub>3</sub>				0.0	0.0		0.0	0.0			0.0	
Control Delay				47.5	38.4		27.1	20.5			14.0	
Lane Group LOS				D	D		C	C			B	
Approach Delay				43.3			24.3			14.0		
Approach LOS				D			C			B		
Intersection Delay	31.6			X <sub>c</sub> = 0.60			Intersection LOS			C		

<b>HCS+™ DETAILED REPORT</b>												
<b>General Information</b>						<b>Site Information</b>						
Analyst JAG						Intersection US 6 and Saw Mill Rd						
Agency or Co. TMA						Area Type All other areas						
Date Performed 10/28/2007						Jurisdiction Danbury, CT						
Time Period A.M. Peak Hour						Analysis Year Build Condition						
						Project ID Stateline						
<b>Volume and Timing Input</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>		2	0	1	1		2		0			
Lane Group		TR		L	T		L	LR				
Volume, V (vph)		98	141	117	577		572		66			
% Heavy Vehicles, %HV		1	1	0	0		1		1			
Peak-Hour Factor, PHF		0.89	0.89	0.80	0.80		0.87		0.87			
Pretimed (P) or Actuated (A)		A	A	A	A		A		A			
Start-up Lost Time, I <sub>1</sub>		2.0		2.0	2.0		2.0	2.0				
Extension of Effective Green, e		2.0		2.0	2.0		2.0	2.0				
Arrival Type, AT		3		3	3		5	3				
Unit Extension, UE		3.0		3.0	3.0		3.0	3.0				
Filtering/Metering, I		1.000		1.000	1.000		1.000	1.000				
Initial Unmet Demand, Q <sub>b</sub>		0.0		0.0	0.0		0.0	0.0				
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0			
Lane Width		13.0		13.0	14.0		11.0	11.0				
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N			
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0		0	0		0	0				
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2			3.2				
Phasing	EW Perm	WB Only	03		04		NB Only	06		07		08
Timing	G = 13.0	G = 10.0	G =		G =		G = 37.0	G =		G =		G =
	Y = 5	Y = 5	Y =		Y =		Y = 5	Y =		Y =		Y =
Duration of Analysis, T = 0.25							Cycle Length, C = 75.0					
<b>Lane Group Capacity, Control Delay, and LOS Determination</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		268		146	721		361	372				
Lane Group Capacity, c		585		644	757		1655	836				
v/c Ratio, X		0.46		0.23	0.95		0.22	0.44				
Total Green Ratio, g/C		0.17		0.41	0.37		0.49	0.49				
Uniform Delay, d <sub>1</sub>		27.8		15.0	22.9		10.8	12.3				
Progression Factor, PF		1.000		1.000	1.000		0.351	1.000				
Delay Calibration, k		0.11		0.11	0.46		0.11	0.11				
Incremental Delay, d <sub>2</sub>		0.6		0.2	21.8		0.1	0.4				
Initial Queue Delay, d <sub>3</sub>		0.0		0.0	0.0		0.0	0.0				
Control Delay		28.4		15.1	44.7		3.9	12.7				
Lane Group LOS		C		B	D		A	B				
Approach Delay	28.4			39.7			8.3					
Approach LOS	C			D			A					
Intersection Delay	25.8			X <sub>c</sub> = 0.66			Intersection LOS			C		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst	JAG					Intersection	US 6, Starr, and NYS 22 ramp					
Agency or Co.	TMA					Area Type	All other areas					
Date Performed	10/28/2007					Jurisdiction	Town of Southeast					
Time Period	A.M. Peak Hour					Analysis Year	Build Condition					
						Project ID	Stateline					
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>	0	2	0	0	2	0	0	1	0	1	1	0
Lane Group	DefL	TR			LTR			LTR		L	TR	
Volume, V (vph)	130	390	113	43	1205	99	95	28	10	180	98	116
% Heavy Vehicles, %HV	5	5	5	5	5	5	5	5	5	5	5	5
Peak-Hour Factor, PHF	0.74	0.74	0.74	0.91	0.91	0.91	0.80	0.80	0.80	0.91	0.91	0.91
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A
Start-up Lost Time, I <sub>1</sub>	2.0	2.0			2.0			2.0		2.0	2.0	
Extension of Effective Green, e	2.0	2.0			2.0			2.0		2.0	2.0	
Arrival Type, AT	3	3			3			3		3	3	
Unit Extension, UE	3.0	3.0			3.0			3.0		3.0	3.0	
Filtering/Metering, I	1.000	1.000			1.000			1.000		1.000	1.000	
Initial Unmet Demand, Q <sub>b</sub>	0.0	0.0			0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width	12.0	12.0			12.0			15.0		10.0	9.0	
Parking / Grade / Parking	N	5	N	N	-2	N	N	-6	N	N	-5	N
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>	0	0			0			0		0	0	
Min. Time for Pedestrians, G <sub>p</sub>	3.2			3.2			3.2			3.2		
Phasing	EW Perm	EB Only	03	04	NS Perm	06	07	08				
Timing	G = 30.0	G = 4.0	G =	G =	G = 16.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 65.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v	176	680			1480			166		198	235	
Lane Group Capacity, c	420	1023			1450			202		292	378	
v/c Ratio, X	0.42	0.66			1.02			0.82		0.68	0.62	
Total Green Ratio, g/C	0.65	0.60			0.46			0.25		0.25	0.25	
Uniform Delay, d <sub>1</sub>	19.2	8.6			17.5			23.2		22.2	21.8	
Progression Factor, PF	1.000	1.000			1.000			1.000		1.000	1.000	
Delay Calibration, k	0.11	0.24			0.50			0.36		0.25	0.20	
Incremental Delay, d <sub>2</sub>	0.7	1.6			29.0			23.0		6.2	3.1	
Initial Queue Delay, d <sub>3</sub>	0.0	0.0			0.0			0.0		0.0	0.0	
Control Delay	19.8	10.3			46.5			46.2		28.4	25.0	
Lane Group LOS	B	B			D			D		C	C	
Approach Delay	12.3			46.5			46.2			26.5		
Approach LOS	B			D			D			C		
Intersection Delay	33.5			X <sub>C</sub> = 1.40			Intersection LOS			C		

### HCS+™ DETAILED REPORT

General Information				Site Information			
Analyst	JAG			Intersection	Argonne /Rt 22 ramps/US 6		
Agency or Co.	TMA			Area Type	All other areas		
Date Performed	10/28/2007			Jurisdiction	Town of Southeast, NY		
Time Period	A.M. Peak Hour			Analysis Year	Build Condition		
				Project ID	Stateline		

#### Volume and Timing Input

	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N <sub>1</sub>	0	1	0	1	1	0	0	2	0	0	2	0	
Lane Group		LTR		L	TR			LTR		DefL	TR		
Volume, V (vph)	2	82	113	232	6	120	18	800	601	176	290	3	
% Heavy Vehicles, %HV	5	5	5	5	5	5	5	5	5	5	5	5	
Peak-Hour Factor, PHF	0.74	0.74	0.74	0.85	0.85	0.85	0.97	0.97	0.97	0.88	0.88	0.88	
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A	
Start-up Lost Time, I <sub>1</sub>		2.0		2.0	2.0			2.0		2.0	2.0		
Extension of Effective Green, e		2.0		2.0	2.0			2.0		2.0	2.0		
Arrival Type, AT		3		3	3			3		3	3		
Unit Extension, UE		3.0		3.0	3.0			3.0		3.0	3.0		
Filtering/Metering, I		1.000		1.000	1.000			1.000		1.000	1.000		
Initial Unmet Demand, Q <sub>b</sub>		0.0		0.0	0.0			0.0		0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0	
Lane Width		15.0		9.0	9.0			12.0		12.0	12.0		
Parking / Grade / Parking	N	0	N	N	-6	N	N	-4	N	N	4	N	
Parking Maneuvers, N <sub>m</sub>													
Buses Stopping, N <sub>b</sub>		0		0	0			0		0	0		
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2			3.2			3.2		
Phasing	EW Perm	WB Only	03			04			NS Perm	SB Only	07		08
Timing	G = 13.0	G = 2.0	G =			G =			G = 33.0	G = 2.0	G =		G =
	Y = 5	Y = 5	Y =			Y =			Y = 5	Y = 5	Y =		Y =
Duration of Analysis, T = 0.25							Cycle Length, C = 70.0						

#### Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		267		273	148			1464		200	333		
Lane Group Capacity, c		340		335	411			1472		344	1012		
v/c Ratio, X		0.79		0.81	0.36			0.99		0.58	0.33		
Total Green Ratio, g/C		0.19		0.33	0.29			0.47		0.61	0.57		
Uniform Delay, d <sub>1</sub>		27.2		26.5	19.9			18.4		23.7	7.9		
Progression Factor, PF		1.000		1.000	1.000			1.000		1.000	1.000		
Delay Calibration, k		0.33		0.36	0.11			0.50		0.17	0.11		
Incremental Delay, d <sub>2</sub>		11.5		14.4	0.5			22.1		2.5	0.2		
Initial Queue Delay, d <sub>3</sub>		0.0		0.0	0.0			0.0		0.0	0.0		
Control Delay		38.7		40.9	20.4			40.5		26.1	8.1		
Lane Group LOS		D		D	C			D		C	A		
Approach Delay		38.7			33.7			40.5			14.9		
Approach LOS		D			C			D			B		
Intersection Delay		34.2			X <sub>c</sub> = 1.22			Intersection LOS			C		



<b>HCS+™ DETAILED REPORT</b>												
<b>General Information</b>						<b>Site Information</b>						
Analyst <i>WFB + JAG</i>						Intersection <i>Rt 6 (East Main) &amp; 202/22</i>						
Agency or Co. <i>TMA</i>						Area Type <i>All other areas</i>						
Date Performed <i>10/30/2007</i>						Jurisdiction <i>Town of Southeast</i>						
Time Period <i>AM Peak Hour</i>						Analysis Year <i>Build Condition</i>						
						Project ID <i>Stateline Retail Center</i>						
<b>Volume and Timing Input</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>	0	1			1	1				1		1
Lane Group		LT			T	R				L		R
Volume, V (vph)	15	176			516	436				530		115
% Heavy Vehicles, %HV	22	22			5	5				7		7
Peak-Hour Factor, PHF	0.75	0.75			0.93	0.93				0.77		0.77
Pretimed (P) or Actuated (A)	A	A			A	A				A		A
Start-up Lost Time, I <sub>1</sub>		2.0			2.0	2.0				2.0		2.0
Extension of Effective Green, e		2.0			2.0	2.0				2.0		2.0
Arrival Type, AT		3			3	3				3		3
Unit Extension, UE		3.0			3.0	3.0				3.0		3.0
Filtering/Metering, I		1.000			1.000	1.000				1.000		1.000
Initial Unmet Demand, Q <sub>b</sub>		0.0			0.0	0.0				0.0		0.0
Ped / Bike / RTOR Volumes	0	0		0	0	0				0	0	0
Lane Width		13.0			11.0	11.0				12.0		11.0
Parking / Grade / Parking	N	0	N	N	0	N				N	0	N
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0			0	0				0		0
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2					3.2		
Phasing	EW Perm	02	03	04	SB Only	06	07	08				
Timing	G = 39.0	G =	G =	G =	G = 36.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 85.0					
<b>Lane Group Capacity, Control Delay, and LOS Determination</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		255			555	469				688		149
Lane Group Capacity, c		699			802	1487				714		618
v/c Ratio, X		0.36			0.69	0.32				0.96		0.24
Total Green Ratio, g/C		0.46			0.46	1.00				0.42		0.42
Uniform Delay, d <sub>1</sub>		14.9			18.2	0.0				23.9		15.7
Progression Factor, PF		1.000			1.000	0.950				1.000		1.000
Delay Calibration, k		0.11			0.26	0.11				0.47		0.11
Incremental Delay, d <sub>2</sub>		0.3			2.6	0.1				24.9		0.2
Initial Queue Delay, d <sub>3</sub>		0.0			0.0	0.0				0.0		0.0
Control Delay		15.3			20.8	0.1				48.8		15.9
Lane Group LOS		B			C	A				D		B
Approach Delay	15.3			11.3						43.0		
Approach LOS	B			B						D		
Intersection Delay	24.3			X <sub>c</sub> = 0.82			Intersection LOS			C		

<b>HCS+™ DETAILED REPORT</b>												
<b>General Information</b>						<b>Site Information</b>						
Analyst JAG						Intersection Saw Mill Rd/I-84 EB ramps						
Agency or Co. TMA						Area Type All other areas						
Date Performed 10/28/2007						Jurisdiction Danbury, CT						
Time Period A.M. Peak Hour						Analysis Year Build Condition						
						Project ID Stateline						
<b>Volume and Timing Input</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>	0	1	1					1	1	0	2	
Lane Group		LT	R					T	R		LT	
Volume, V (vph)	77	0	596					371	282	88	545	
% Heavy Vehicles, %HV	1	1	1					0	0	2	2	
Peak-Hour Factor, PHF	0.89	0.89	0.89					0.89	0.89	0.96	0.96	
Pretimed (P) or Actuated (A)	A	A	A					A	A	A	A	
Start-up Lost Time, I <sub>1</sub>		2.0	2.0					2.0	2.0		2.0	
Extension of Effective Green, e		2.0	2.0					2.0	2.0		2.0	
Arrival Type, AT		3	3					3	3		5	
Unit Extension, UE		3.0	3.0					3.0	3.0		3.0	
Filtering/Metering, I		1.000	1.000					1.000	1.000		1.000	
Initial Unmet Demand, Q <sub>b</sub>		0.0	0.0					0.0	0.0		0.0	
Ped / Bike / RTOR Volumes	0	0	0				0	0	50	0	0	
Lane Width		12.0	12.0					11.0	11.0		11.0	
Parking / Grade / Parking	N	3	N				N	8	N	N	-5	N
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0	0					0	0		0	
Min. Time for Pedestrians, G <sub>p</sub>		3.2						3.2			3.2	
Phasing	EB Only	02	03	04	SB Only	NB Only	07	08				
Timing	G = 10.0	G =	G =	G =	G = 30.0	G = 20.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 75.0						
<b>Lane Group Capacity, Control Delay, and LOS Determination</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		87	670					417	261		660	
Lane Group Capacity, c		235	735					470	400		1396	
v/c Ratio, X		0.37	0.91					0.89	0.65		0.47	
Total Green Ratio, g/C		0.13	0.47					0.27	0.27		0.40	
Uniform Delay, d <sub>1</sub>		29.6	18.6					26.4	24.4		16.6	
Progression Factor, PF		1.000	1.000					1.000	1.000		0.556	
Delay Calibration, k		0.11	0.43					0.41	0.23		0.11	
Incremental Delay, d <sub>2</sub>		1.0	15.6					18.3	3.8		0.3	
Initial Queue Delay, d <sub>3</sub>		0.0	0.0					0.0	0.0		0.0	
Control Delay		30.6	34.2					44.7	28.2		9.5	
Lane Group LOS		C	C					D	C		A	
Approach Delay	33.8						38.3			9.5		
Approach LOS	C						D			A		
Intersection Delay	27.6			X <sub>c</sub> = 0.66			Intersection LOS			C		

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	JAG		Intersection	US 6/Dingle Ridge Road				
Agency/Co.	TMA		Jurisdiction	Town of Southeast, NY				
Date Performed	10/28/2007		Analysis Year	Build Condition				
Analysis Time Period	A.M. Peak Hour							
Project Description <i>Stateline</i>								
East/West Street: <i>US Route 6 &amp; 202 (Danbury Rd)</i>			North/South Street: <i>Dingle Ridge Road</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	0	221	20	14	1150	2		
Peak-Hour Factor, PHF	0.75	0.75	0.75	0.92	0.92	0.92		
Hourly Flow Rate, HFR (veh/h)	0	294	26	15	1249	2		
Percent Heavy Vehicles	5	--	--	1	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	2	0	0	2	0		
Configuration	LT		TR	LT		TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	39	2	19	0	2	1		
Peak-Hour Factor, PHF	0.78	0.78	0.78	0.38	0.38	0.38		
Hourly Flow Rate, HFR (veh/h)	50	2	24	0	5	2		
Percent Heavy Vehicles	3	3	3	3	3	3		
Percent Grade (%)	2			2				
Flared Approach		Y			N			
Storage		1			0			
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration		LTR			LTR			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT	LT		LTR			LTR	
v (veh/h)	0	15		76			7	
C (m) (veh/h)	536	1244		278			130	
v/c	0.00	0.01		0.27			0.05	
95% queue length	0.00	0.04		1.08			0.17	
Control Delay (s/veh)	11.7	7.9		24.1			34.3	
LOS	B	A		C			D	
Approach Delay (s/veh)	--	--		24.1			34.3	
Approach LOS	--	--		C			D	

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	JAG		Intersection	Joe's Hill Road				
Agency/Co.	TMA		Jurisdiction	Town of Southeast, NY				
Date Performed	10/28/2007		Analysis Year	Build Condition				
Analysis Time Period	A.M. Peak Hour							
Project Description <i>Stateline</i>								
East/West Street: <i>US Route 6 &amp; 202</i>			North/South Street: <i>Joe's Hill Road</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	15	230			1161	7		
Peak-Hour Factor, PHF	0.73	0.73	1.00	1.00	0.91	0.91		
Hourly Flow Rate, HFR (veh/h)	20	315	0	0	1275	7		
Percent Heavy Vehicles	5	--	--	0	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	2	0	0	2	0		
Configuration	<i>LT</i>	<i>T</i>			<i>T</i>	<i>TR</i>		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				21		55		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.86	1.00	0.86		
Hourly Flow Rate, HFR (veh/h)	0	0	0	24	0	63		
Percent Heavy Vehicles	0	0	0	3	0	3		
Percent Grade (%)	0			2				
Flared Approach		<i>N</i>			<i>N</i>			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					<i>LR</i>			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>						<i>LR</i>	
v (veh/h)	20						87	
C (m) (veh/h)	521						238	
v/c	0.04						0.37	
95% queue length	0.12						1.59	
Control Delay (s/veh)	12.2						28.6	
LOS	<i>B</i>						<i>D</i>	
Approach Delay (s/veh)	--	--					28.6	
Approach LOS	--	--					<i>D</i>	

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	JAG		Intersection	US Rte 6/NYS 121				
Agency/Co.	TMA		Jurisdiction	Town of Southeast, NY				
Date Performed	10/28/2007		Analysis Year	Build Condition				
Analysis Time Period	A.M. Peak Hour							
Project Description <i>Stateline</i>								
East/West Street: <i>US Route 6 &amp; 202</i>			North/South Street: <i>NYS Rte 121 Peach Lake</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		309	283	250	994			
Peak-Hour Factor, PHF	1.00	0.75	0.75	0.92	0.92	1.00		
Hourly Flow Rate, HFR (veh/h)	0	412	377	271	1080	0		
Percent Heavy Vehicles	0	--	--	5	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			1			0		
Lanes	0	2	1	0	2	0		
Configuration		T	R	LT	T			
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	327		34					
Peak-Hour Factor, PHF	0.86	1.00	0.86	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	380	0	39	0	0	0		
Percent Heavy Vehicles	4	0	4	0	0	0		
Percent Grade (%)	-4			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	1	0	1	0	0	0		
Configuration	L		R					
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT	L		R			
v (veh/h)		271	380		39			
C (m) (veh/h)		1122	85		794			
v/c		0.24	4.47		0.05			
95% queue length		0.95	40.40		0.15			
Control Delay (s/veh)		9.2	1662		9.8			
LOS		A	F		A			
Approach Delay (s/veh)	--	--	1508					
Approach LOS	--	--	F					

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	JAG		Intersection	Route 121/ I-84 off-ramp				
Agency/Co.	TMA		Jurisdiction	Town of Southeast				
Date Performed	10/28/07		Analysis Year	Build Condition				
Analysis Time Period	A.M. Peak Hour							
Project Description <i>Stateline</i>								
East/West Street: <i>I-84 off-ramp</i>			North/South Street: <i>Peach Lake Rd (NYS Rt 121)</i>					
Intersection Orientation: <i>North-South</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		118			569			
Peak-Hour Factor, PHF	1.00	0.88	1.00	1.00	0.72	1.00		
Hourly Flow Rate, HFR (veh/h)	0	134	0	0	790	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration		T			T			
Upstream Signal		0			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				443		220		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.87	1.00	0.87		
Hourly Flow Rate, HFR (veh/h)	0	0	0	509	0	252		
Percent Heavy Vehicles	0	0	0	5	0	5		
Percent Grade (%)	0			-3				
Flared Approach		N			Y			
Storage		0			3			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration				LR				
v (veh/h)				761				
C (m) (veh/h)				385				
v/c				1.98				
95% queue length				52.44				
Control Delay (s/veh)				472.0				
LOS				F				
Approach Delay (s/veh)	--	--	472.0					
Approach LOS	--	--	F					

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	JAG	Intersection	Rt 121/I-84 on-ramp
Agency/Co.	TMA	Jurisdiction	Town of Southeast
Date Performed	10/28/2007	Analysis Year	Build Condition
Analysis Time Period	A.M. Peak Hour		
Project Description <i>Stateline</i>			
East/West Street: <i>I-84 on-ramp</i>		North/South Street: <i>NYS Rt. 121 Peach Lake Rd.</i>	
Intersection Orientation: <i>North-South</i>		Study Period (hrs): <i>0.25</i>	

### Vehicle Volumes and Adjustments

Major Street	Northbound			Southbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)		118	71	94	914	
Peak-Hour Factor, PHF	1.00	0.91	0.91	0.89	0.89	1.00
Hourly Flow Rate, HFR (veh/h)	0	129	78	105	1026	0
Percent Heavy Vehicles	0	--	--	3	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration			TR	LT		
Upstream Signal		0			0	

Minor Street	Eastbound			Westbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)						
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0			0		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration						

### Delay, Queue Length, and Level of Service

Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT						
v (veh/h)		105						
C (m) (veh/h)		1358						
v/c		0.08						
95% queue length		0.25						
Control Delay (s/veh)		7.9						
LOS		A						
Approach Delay (s/veh)	--	--						
Approach LOS	--	--						

## TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	WFB + JAG	Intersection	US Rt 6 and Sodom
Agency/Co.	TMA	Jurisdiction	Town of Southeast
Date Performed	10/30/2007	Analysis Year	Build Condition
Analysis Time Period	A.M. Peak Hour		
Project Description <i>Stateline</i>			
East/West Street: <i>US Route 6/202/22</i>		North/South Street: <i>Sodom Road/Driveway</i>	
Intersection Orientation: <i>East-West</i>		Study Period (hrs): <i>0.25</i>	

### Vehicle Volumes and Adjustments

Major Street Movement	Eastbound			Westbound		
	1 L	2 T	3 R	4 L	5 T	6 R
Volume (veh/h)	0	304	4	1	870	19
Peak-Hour Factor, PHF	0.85	0.85	0.85	0.95	0.95	0.95
Hourly Flow Rate, HFR (veh/h)	0	357	4	1	915	20
Percent Heavy Vehicles	0	--	--	0	--	--
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	2	0	0	1	1
Configuration	LT		TR	LT		R
Upstream Signal		0			0	

Minor Street Movement	Northbound			Southbound		
	7 L	8 T	9 R	10 L	11 T	12 R
Volume (veh/h)	7	3	13	31	0	5
Peak-Hour Factor, PHF	0.52	0.52	0.52	0.67	0.67	0.67
Hourly Flow Rate, HFR (veh/h)	13	5	25	46	0	7
Percent Heavy Vehicles	9	9	9	14	14	14
Percent Grade (%)	0			0		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	1	1	0	1	0
Configuration	LT		R		LTR	

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
			7	8	9	10	11	12
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT	LT	LT		R		LTR	
v (veh/h)	0	1	18		25		53	
C (m) (veh/h)	741	1209	120		810		153	
v/c	0.00	0.00	0.15		0.03		0.35	
95% queue length	0.00	0.00	0.51		0.10		1.43	
Control Delay (s/veh)	9.9	8.0	40.2		9.6		40.5	
LOS	A	A	E		A		E	
Approach Delay (s/veh)	--	--	22.4			40.5		
Approach LOS	--	--	C			E		



## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	WFB + JAG		Intersection	US Route 6 and CR 50				
Agency/Co.	TMA		Jurisdiction	Town of Southeast				
Date Performed	10/30/2007		Analysis Year	Build Condition				
Analysis Time Period	A.M. Peak Hour							
Project Description <i>Stateline Retail Center</i>								
East/West Street: <i>US Route 6/202/22</i>			North/South Street: <i>County Road 50</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	67	302			882	0		
Peak-Hour Factor, PHF	0.86	0.86	1.00	1.00	0.97	0.97		
Hourly Flow Rate, HFR (veh/h)	77	351	0	0	909	0		
Percent Heavy Vehicles	11	--	--	0	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	1	1	0	0	1	0		
Configuration	L	T				TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				2		148		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.87	1.00	0.87		
Hourly Flow Rate, HFR (veh/h)	0	0	0	2	0	170		
Percent Heavy Vehicles	0	0	0	10	0	10		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	1	0	1		
Configuration				L		R		
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L					L		R
v (veh/h)	77					2		170
C (m) (veh/h)	713					130		322
v/c	0.11					0.02		0.53
95% queue length	0.36					0.05		2.91
Control Delay (s/veh)	10.7					33.1		28.0
LOS	B					D		D
Approach Delay (s/veh)	--	--				28.1		
Approach LOS	--	--				D		

**HCS+™ DETAILED REPORT**

<b>General Information</b>				<b>Site Information</b>			
Analyst	JAG			Intersection	I-84 WB ramps/Saw Mill		
Agency or Co.	TMA			Area Type	All other areas		
Date Performed	10/29/2007			Jurisdiction	Danbury, CT		
Time Period	P.M. Peak Hour			Analysis Year	Build Condition		
				Project ID	Stateline		

**Volume and Timing Input**

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>				1	1	0	1	1			2	0
Lane Group				L	LTR		L	T			TR	
Volume, V (vph)				334	3	122	345	242			411	161
% Heavy Vehicles, %HV				1	1	1	1	1			1	1
Peak-Hour Factor, PHF				0.89	0.89	0.89	0.88	0.88			0.92	0.92
Pretimed (P) or Actuated (A)				A	A	A	A	A			A	A
Start-up Lost Time, I <sub>1</sub>				2.0	2.0		2.0	2.0			2.0	
Extension of Effective Green, e				2.0	2.0		2.0	2.0			2.0	
Arrival Type, AT				3	3		5	5			5	
Unit Extension, UE				3.0	3.0		3.0	3.0			3.0	
Filtering/Metering, I				1.000	1.000		1.000	1.000			1.000	
Initial Unmet Demand, Q <sub>b</sub>				0.0	0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes				0	0	35	0	0		0	0	0
Lane Width				12.0	12.0		11.0	11.0			12.0	
Parking / Grade / Parking				N	-1	N	N	1	N	N	-1	N
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>				0	0		0	0			0	
Min. Time for Pedestrians, G <sub>p</sub>				3.2			3.2			3.2		
Phasing	WB Only	02	03	04	NB Only	SB Only	07	08				
Timing	G = 13.0	G =	G =	G =	G = 23.0	G = 24.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 75.0						

**Lane Group Capacity, Control Delay, and LOS Determination**

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v				263	213		392	275			622	
Lane Group Capacity, c				311	297		527	555			1103	
v/c Ratio, X				0.85	0.72		0.74	0.50			0.56	
Total Green Ratio, g/C				0.17	0.17		0.31	0.31			0.32	
Uniform Delay, d <sub>1</sub>				30.0	29.3		23.4	21.3			21.2	
Progression Factor, PF				1.000	1.000		0.705	0.705			0.686	
Delay Calibration, k				0.38	0.28		0.30	0.11			0.16	
Incremental Delay, d <sub>2</sub>				18.9	8.1		5.7	0.7			0.7	
Initial Queue Delay, d <sub>3</sub>				0.0	0.0		0.0	0.0			0.0	
Control Delay				49.0	37.3		22.1	15.7			15.2	
Lane Group LOS				D	D		C	B			B	
Approach Delay				43.8			19.5			15.2		
Approach LOS				D			B			B		
Intersection Delay	24.5			X <sub>c</sub> = 0.69			Intersection LOS			C		

<b>HCS+™ DETAILED REPORT</b>												
<b>General Information</b>						<b>Site Information</b>						
Analyst JAG						Intersection US 6 and Saw Mill Rd						
Agency or Co. TMA						Area Type All other areas						
Date Performed 10/29/2007						Jurisdiction Danbury, CT						
Time Period P.M. Peak Hour						Analysis Year Build Condition						
						Project ID Stateline						
<b>Volume and Timing Input</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>l</sub>		2	0	1	1		2		0			
Lane Group		TR		L	T		L	LR				
Volume, V (vph)		361	390	176	233		242		131			
% Heavy Vehicles, %HV		1	1	1	1		1		1			
Peak-Hour Factor, PHF		0.95	0.95	0.87	0.87		0.98		0.98			
Pretimed (P) or Actuated (A)		A	A	A	A		A		A			
Start-up Lost Time, I <sub>1</sub>		2.0		2.0	2.0		2.0	2.0				
Extension of Effective Green, e		2.0		2.0	2.0		2.0	2.0				
Arrival Type, AT		3		3	3		5	3				
Unit Extension, UE		3.0		3.0	3.0		3.0	3.0				
Filtering/Metering, I		1.000		1.000	1.000		1.000	1.000				
Initial Unmet Demand, Q <sub>b</sub>		0.0		0.0	0.0		0.0	0.0				
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0			
Lane Width		13.0		13.0	14.0		11.0	11.0				
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N			
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0		0	0		0	0				
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2			3.2				
Phasing	EW Perm	WB Only	03		04		NB Only	06		07		08
Timing	G = 20.0	G = 4.0	G =		G =		G = 36.0	G =		G =		G =
	Y = 5	Y = 5	Y =		Y =		Y = 5	Y =		Y =		Y =
Duration of Analysis, T = 0.25							Cycle Length, C = 75.0					
<b>Lane Group Capacity, Control Delay, and LOS Determination</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		791		202	268		124	257				
Lane Group Capacity, c		910		400	776		1610	786				
v/c Ratio, X		0.87		0.50	0.35		0.08	0.33				
Total Green Ratio, g/C		0.27		0.43	0.39		0.48	0.48				
Uniform Delay, d <sub>1</sub>		26.3		24.8	16.3		10.5	12.0				
Progression Factor, PF		1.000		1.000	1.000		0.385	1.000				
Delay Calibration, k		0.40		0.11	0.11		0.11	0.11				
Incremental Delay, d <sub>2</sub>		9.1		1.0	0.3		0.0	0.2				
Initial Queue Delay, d <sub>3</sub>		0.0		0.0	0.0		0.0	0.0				
Control Delay		35.3		25.8	16.5		4.1	12.3				
Lane Group LOS		D		C	B		A	B				
Approach Delay	35.3			20.5			9.6					
Approach LOS	D			C			A					
Intersection Delay	25.1			X <sub>c</sub> = 0.55			Intersection LOS			C		

**HCS+™ DETAILED REPORT**

General Information				Site Information			
Analyst	JAG			Intersection	US 6, Starr, and NYS 22 ramp		
Agency or Co.	TMA			Area Type	All other areas		
Date Performed	10/29/2007			Jurisdiction	Town of Southeast		
Time Period	P.M. Peak Hour			Analysis Year	Build Condition		
				Project ID	Stateline Retail Center		

**Volume and Timing Input**

	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N <sub>1</sub>	0	2	0	0	2	0	0	1	0	1	1	0	
Lane Group		LTR			LTR			LTR		L	TR		
Volume, V (vph)	156	630	87	28	675	143	147	50	26	368	39	321	
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1	
Peak-Hour Factor, PHF	0.95	0.95	0.95	0.92	0.92	0.92	0.90	0.90	0.90	0.90	0.90	0.90	
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A	
Start-up Lost Time, I <sub>1</sub>		2.0			2.0			2.0		2.0	2.0		
Extension of Effective Green, e		2.0			2.0			2.0		2.0	2.0		
Arrival Type, AT		3			3			3		3	3		
Unit Extension, UE		3.0			3.0			3.0		3.0	3.0		
Filtering/Metering, I		1.000			1.000			1.000		1.000	1.000		
Initial Unmet Demand, Q <sub>b</sub>		0.0			0.0			0.0		0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0	
Lane Width		12.0			12.0			15.0		10.0	9.0		
Parking / Grade / Parking	N	5	N	N	-2	N	N	-6	N	N	-5	N	
Parking Maneuvers, N <sub>m</sub>													
Buses Stopping, N <sub>b</sub>		0			0			0		0	0		
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2			3.2			3.2		
Phasing	EW Perm	EB Only	03	04	NS Perm	06	07	08					
Timing	G = 20.0	G = 5.0	G =	G =	G = 25.0	G =	G =	G =					
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 65.0						

**Lane Group Capacity, Control Delay, and LOS Determination**

	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		919			919			248		409	400		
Lane Group Capacity, c		1097			939			315		434	578		
v/c Ratio, X		0.84			0.98			0.79		0.94	0.69		
Total Green Ratio, g/C		0.46			0.31			0.38		0.38	0.38		
Uniform Delay, d <sub>1</sub>		15.4			22.3			17.7		19.3	16.8		
Progression Factor, PF		1.000			1.000			1.000		1.000	1.000		
Delay Calibration, k		0.37			0.48			0.33		0.46	0.26		
Incremental Delay, d <sub>2</sub>		5.9			24.2			12.5		29.1	3.5		
Initial Queue Delay, d <sub>3</sub>		0.0			0.0			0.0		0.0	0.0		
Control Delay		21.2			46.5			30.2		48.4	20.3		
Lane Group LOS		C			D			C		D	C		
Approach Delay		21.2			46.5			30.2			34.5		
Approach LOS		C			D			C			C		
Intersection Delay		33.7			X <sub>c</sub> = 0.97			Intersection LOS			C		

## HCS+™ DETAILED REPORT

General Information				Site Information			
Analyst	JAG			Intersection	Argonne /Rt 22 ramps/US 6		
Agency or Co.	TMA			Area Type	All other areas		
Date Performed	10/29/2007			Jurisdiction	Town of Southeast, NY		
Time Period	P.M. Peak Hour			Analysis Year	Build Condition		
				Project ID	Stateline		

## Volume and Timing Input

	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N <sub>l</sub>	0	1	0	1	1	0	0	2	0	0	2	0	
Lane Group		LTR		L	TR			LTR		DefL	TR		
Volume, V (vph)	7	71	289	173	10	168	28	819	336	144	388	2	
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1	
Peak-Hour Factor, PHF	0.93	0.93	0.93	0.89	0.89	0.89	0.94	0.94	0.94	0.97	0.97	0.97	
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A	
Start-up Lost Time, I <sub>1</sub>		2.0		2.0	2.0			2.0		2.0	2.0		
Extension of Effective Green, e		2.0		2.0	2.0			2.0		2.0	2.0		
Arrival Type, AT		3		3	3			3		3	3		
Unit Extension, UE		3.0		3.0	3.0			3.0		3.0	3.0		
Filtering/Metering, I		1.000		1.000	1.000			1.000		1.000	1.000		
Initial Unmet Demand, Q <sub>b</sub>		0.0		0.0	0.0			0.0		0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0	
Lane Width		15.0		9.0	9.0			12.0		12.0	12.0		
Parking / Grade / Parking	N	0	N	N	-6	N	N	-4	N	N	4	N	
Parking Maneuvers, N <sub>m</sub>													
Buses Stopping, N <sub>b</sub>		0		0	0			0		0	0		
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2			3.2			3.2		
Phasing	EW Perm	02		03		04		NS Perm	SB Only		07		08
Timing	G = 20.0	G =		G =		G =		G = 23.0	G = 2.0		G =		G =
	Y = 5	Y =		Y =		Y =		Y = 5	Y = 5		Y =		Y =
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0						

## Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		395		194	200			1258		148	402		
Lane Group Capacity, c		613		237	499			1258		415	921		
v/c Ratio, X		0.64		0.82	0.40			1.00		0.36	0.44		
Total Green Ratio, g/C		0.33		0.33	0.33			0.38		0.55	0.50		
Uniform Delay, d <sub>1</sub>		17.0		18.3	15.4			18.5		18.1	9.6		
Progression Factor, PF		1.000		1.000	1.000			1.000		1.000	1.000		
Delay Calibration, k		0.22		0.36	0.11			0.50		0.11	0.11		
Incremental Delay, d <sub>2</sub>		2.3		19.8	0.5			25.4		0.5	0.3		
Initial Queue Delay, d <sub>3</sub>		0.0		0.0	0.0			0.0		0.0	0.0		
Control Delay		19.3		38.1	15.9			43.9		18.6	9.9		
Lane Group LOS		B		D	B			D		B	A		
Approach Delay		19.3			26.8			43.9			12.3		
Approach LOS		B			C			D			B		
Intersection Delay		30.9			X <sub>c</sub> = 1.17			Intersection LOS			C		

### HCS+™ DETAILED REPORT

General Information	Site Information
Analyst           WFB + JAG	Intersection     Rt 6 (East Main) & 202/22
Agency or Co.   TMA	Area Type       All other areas
Date Performed  10/30/2007	Jurisdiction     Town of Southeast
Time Period     PM Peak Hour	Analysis Year   Build Condition
	Project ID       Stateline Retail Center

#### Volume and Timing Input

	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N <sub>1</sub>	0	1			1	1				1		1	
Lane Group		LT			T	R				L		R	
Volume, V (vph)	69	379			205	743				564		46	
% Heavy Vehicles, %HV	4	4			2	2				3		3	
Peak-Hour Factor, PHF	0.93	0.93			0.96	0.96				0.92		0.92	
Pretimed (P) or Actuated (A)	A	A			A	A				A		A	
Start-up Lost Time, I <sub>1</sub>		2.0			2.0	2.0				2.0		2.0	
Extension of Effective Green, e		2.0			2.0	2.0				2.0		2.0	
Arrival Type, AT		3			3	3				3		3	
Unit Extension, UE		3.0			3.0	3.0				3.0		3.0	
Filtering/Metering, I		1.000			1.000	1.000				1.000		1.000	
Initial Unmet Demand, Q <sub>b</sub>		0.0			0.0	0.0				0.0		0.0	
Ped / Bike / RTOR Volumes	0	0		0	0	0				0	0	0	
Lane Width		13.0			11.0	11.0				12.0		11.0	
Parking / Grade / Parking	N	0	N	N	0	N				N	0	N	
Parking Maneuvers, N <sub>m</sub>													
Buses Stopping, N <sub>b</sub>		0			0	0				0		0	
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2						3.2		
Phasing	EW Perm	02	03	04	SB Only	06	07	08					
Timing	G = 39.0	G =	G =	G =	G = 36.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 85.0						

#### Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		482			214	774				613		50
Lane Group Capacity, c		792			826	1531				742		642
v/c Ratio, X		0.61			0.26	0.51				0.83		0.08
Total Green Ratio, g/C		0.46			0.46	1.00				0.42		0.42
Uniform Delay, d <sub>1</sub>		17.3			14.1	0.0				21.7		14.6
Progression Factor, PF		1.000			1.000	0.950				1.000		1.000
Delay Calibration, k		0.19			0.11	0.11				0.36		0.11
Incremental Delay, d <sub>2</sub>		1.4			0.2	0.3				7.7		0.1
Initial Queue Delay, d <sub>3</sub>		0.0			0.0	0.0				0.0		0.0
Control Delay		18.6			14.3	0.3				29.4		14.7
Lane Group LOS		B			B	A				C		B
Approach Delay	18.6			3.3						28.3		
Approach LOS	B			A						C		
Intersection Delay	14.5			X <sub>c</sub> = 0.71			Intersection LOS			B		

### HCS+™ DETAILED REPORT

General Information	Site Information
Analyst            JAG Agency or Co.   TMA Date Performed 10/28/2007 Time Period      P.M. Peak Hour	Intersection      Saw Mill Rd / I-84 EB ramps Area Type        All other areas Jurisdiction      Danbury, CT Analysis Year     Build Condition Project ID        Stateline

#### Volume and Timing Input

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>	0	1	1					1	1	0	2	
Lane Group		LT	R					T	R	DefL	T	
Volume, V (vph)	97	0	330					503	539	230	503	
% Heavy Vehicles, %HV	1	1	1					0	0	2	2	
Peak-Hour Factor, PHF	0.73	0.73	0.73					0.83	0.83	0.87	0.87	
Pretimed (P) or Actuated (A)	A	A	A					A	A	A	A	
Start-up Lost Time, I <sub>1</sub>		2.0	2.0					2.0	2.0	2.0	2.0	
Extension of Effective Green, e		2.0	2.0					2.0	2.0	2.0	2.0	
Arrival Type, AT		3	3					3	3	5	5	
Unit Extension, UE		3.0	3.0					3.0	3.0	3.0	3.0	
Filtering/Metering, I		1.000	1.000					1.000	1.000	1.000	1.000	
Initial Unmet Demand, Q <sub>b</sub>		0.0	0.0					0.0	0.0	0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0				0	0	150	0	0	
Lane Width		12.0	12.0					11.0	11.0	11.0	11.0	
Parking / Grade / Parking	N	3	N				N	8	N	N	-5	N
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0	0					0	0	0	0	
Min. Time for Pedestrians, G <sub>p</sub>		3.2						3.2			3.2	
Phasing	EB Only	02	03	04	SB Only	NS Perm	07	08				
Timing	G = 23.0	G =	G =	G =	G = 10.0	G = 27.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 75.0					

#### Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		133	452					606	469	264	578	
Lane Group Capacity, c		541	483					635	540	333	1034	
v/c Ratio, X		0.25	0.94					0.95	0.87	0.79	0.56	
Total Green Ratio, g/C		0.31	0.31					0.36	0.36	0.56	0.56	
Uniform Delay, d <sub>1</sub>		19.5	25.3					23.4	22.3	16.0	10.6	
Progression Factor, PF		1.000	1.000					1.000	1.000	0.897	0.152	
Delay Calibration, k		0.11	0.45					0.46	0.40	0.34	0.16	
Incremental Delay, d <sub>2</sub>		0.2	25.8					24.9	14.1	12.4	0.7	
Initial Queue Delay, d <sub>3</sub>		0.0	0.0					0.0	0.0	0.0	0.0	
Control Delay		19.7	51.1					48.3	36.5	26.8	2.3	
Lane Group LOS		B	D					D	D	C	A	
Approach Delay	44.0						43.1			10.0		
Approach LOS	D						D			A		
Intersection Delay	32.2			X <sub>c</sub> = 1.18			Intersection LOS			C		

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	JAG		Intersection	US 6/Dingle Ridge Road				
Agency/Co.	TMA		Jurisdiction	Town of Southeast, NY				
Date Performed	10/28/2007		Analysis Year	Build Condition				
Analysis Time Period	P.M. Peak Hour							
Project Description <i>Stateline</i>								
East/West Street: <i>US Route 6 &amp; 202 (Danbury Rd)</i>			North/South Street: <i>Dingle Ridge Road</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	1	703	56	31	515	1		
Peak-Hour Factor, PHF	0.95	0.95	0.95	0.93	0.93	0.93		
Hourly Flow Rate, HFR (veh/h)	1	740	58	33	553	1		
Percent Heavy Vehicles	1	--	--	1	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	2	0	0	2	0		
Configuration	<i>LT</i>		<i>TR</i>	<i>LT</i>		<i>TR</i>		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	61	0	26	3	0	0		
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.38	0.38	0.38		
Hourly Flow Rate, HFR (veh/h)	67	0	28	7	0	0		
Percent Heavy Vehicles	1	1	1	1	1	1		
Percent Grade (%)	2			2				
Flared Approach		Y			N			
Storage		1			0			
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration		<i>LTR</i>			<i>LTR</i>			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>	<i>LT</i>		<i>LTR</i>			<i>LTR</i>	
v (veh/h)	1	33		95			7	
C (m) (veh/h)	1019	827		213			185	
v/c	0.00	0.04		0.45			0.04	
95% queue length	0.00	0.12		2.11			0.12	
Control Delay (s/veh)	8.5	9.5		34.8			25.2	
LOS	A	A		D			D	
Approach Delay (s/veh)	--	--		34.8			25.2	
Approach LOS	--	--		D			D	



## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	JAG		Intersection	Joe's Hill Rd/ US 6				
Agency/Co.	TMA		Jurisdiction	Town of Southeast, NY				
Date Performed	10/28/2007		Analysis Year	Build Condition				
Analysis Time Period	P.M. Peak Hour							
Project Description <i>Stateline</i>								
East/West Street: <i>US Route 6 &amp; 202</i>			North/South Street: <i>Joe's Hill Road</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	55	761			494	34		
Peak-Hour Factor, PHF	0.95	0.95	1.00	1.00	0.89	0.89		
Hourly Flow Rate, HFR (veh/h)	57	801	0	0	555	38		
Percent Heavy Vehicles	1	--	--	0	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	2	0	0	2	0		
Configuration	<i>LT</i>	<i>T</i>			<i>T</i>	<i>TR</i>		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				9		27		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.68	1.00	0.68		
Hourly Flow Rate, HFR (veh/h)	0	0	0	13	0	39		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	0			2				
Flared Approach		<i>N</i>			<i>N</i>			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					<i>LR</i>			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>						<i>LR</i>	
v (veh/h)	57						52	
C (m) (veh/h)	986						434	
v/c	0.06						0.12	
95% queue length	0.18						0.40	
Control Delay (s/veh)	8.9						14.4	
LOS	<i>A</i>						<i>B</i>	
Approach Delay (s/veh)	--	--					14.4	
Approach LOS	--	--					<i>B</i>	

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	JAG		Intersection	US Rte 6/NYS 121				
Agency/Co.	TMA		Jurisdiction	Town of Southeast, NY				
Date Performed	10/28/2007		Analysis Year	Build Condition				
Analysis Time Period	P.M. Peak Hour							
Project Description <i>Stateline</i>								
East/West Street: <i>US Route 6 &amp; 202</i>			North/South Street: <i>NYS Rte 121 Peach Lake</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		753	197	113	504			
Peak-Hour Factor, PHF	1.00	0.85	0.85	0.97	0.97	1.00		
Hourly Flow Rate, HFR (veh/h)	0	885	231	116	519	0		
Percent Heavy Vehicles	0	--	--	1	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			1			0		
Lanes	0	2	1	0	2	0		
Configuration		T	R	LT	T			
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	256		108					
Peak-Hour Factor, PHF	0.95	1.00	0.95	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	269	0	113	0	0	0		
Percent Heavy Vehicles	1	0	1	0	0	0		
Percent Grade (%)	-4			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	1	0	1	0	0	0		
Configuration	L		R					
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT	L		R			
v (veh/h)		116	269		113			
C (m) (veh/h)		767	117		566			
v/c		0.15	2.30		0.20			
95% queue length		0.53	23.32		0.74			
Control Delay (s/veh)		10.5	670.5		12.9			
LOS		B	F		B			
Approach Delay (s/veh)	--	--	476.0					
Approach LOS	--	--	F					

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	JAG		Intersection	Route 121/ I-84 off-ramp				
Agency/Co.	TMA		Jurisdiction	Town of Southeast				
Date Performed	10/28/07		Analysis Year	Build Condition				
Analysis Time Period	P.M. Peak Hour							
Project Description <i>Stateline</i>								
East/West Street: <i>I-84 off-ramp</i>			North/South Street: <i>Peach Lake Rd (NYS Rt 121)</i>					
Intersection Orientation: <i>North-South</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		259			313			
Peak-Hour Factor, PHF	1.00	0.92	1.00	1.00	0.88	1.00		
Hourly Flow Rate, HFR (veh/h)	0	281	0	0	355	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration		T			T			
Upstream Signal		0			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				91		103		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.83	1.00	0.83		
Hourly Flow Rate, HFR (veh/h)	0	0	0	109	0	124		
Percent Heavy Vehicles	0	0	0	1	0	1		
Percent Grade (%)	0			-3				
Flared Approach		N			Y			
Storage		0			3			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration				LR				
v (veh/h)				233				
C (m) (veh/h)				949				
v/c				0.25				
95% queue length				0.97				
Control Delay (s/veh)				13.0				
LOS				B				
Approach Delay (s/veh)	--	--	13.0					
Approach LOS	--	--	B					

TWO-WAY STOP CONTROL SUMMARY								
<b>General Information</b>				<b>Site Information</b>				
Analyst	JAG			Intersection	NYS Rte 121/I-84 on-ramp			
Agency/Co.	TMA			Jurisdiction	Town of Southeast			
Date Performed	10/28/2007			Analysis Year	Build Condition			
Analysis Time Period	P.M. Peak Hour							
Project Description <i>Stateline</i>								
East/West Street: <i>I-84 on-ramp</i>				North/South Street: <i>NYS Rt. 121 Peach Lake Rd.</i>				
Intersection Orientation: <i>North-South</i>				Study Period (hrs): <i>0.25</i>				
<b>Vehicle Volumes and Adjustments</b>								
<b>Major Street</b>	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		259	210	139	265			
Peak-Hour Factor, PHF	1.00	0.97	0.97	0.82	0.82	1.00		
Hourly Flow Rate, HFR (veh/h)	0	267	216	169	323	0		
Percent Heavy Vehicles	0	--	--	2	--	--		
Median Type	Undivided							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration			TR	LT				
Upstream Signal		0			0			
<b>Minor Street</b>	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)								
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration								
<b>Delay, Queue Length, and Level of Service</b>								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT						
v (veh/h)		169						
C (m) (veh/h)		1080						
v/c		0.16						
95% queue length		0.55						
Control Delay (s/veh)		9.0						
LOS		A						
Approach Delay (s/veh)	--	--						
Approach LOS	--	--						

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	WFB + JAG		Intersection	Route 6 & Sodom				
Agency/Co.	TMA		Jurisdiction	Town of Southeast				
Date Performed	10/30/2007		Analysis Year	Build Condition				
Analysis Time Period	P.M. Peak Hour							
Project Description <i>Stateline</i>								
East/West Street: <i>US Route 6/202/22</i>			North/South Street: <i>Sodom Road/Driveway</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	0	433	2	22	993	107		
Peak-Hour Factor, PHF	0.94	0.94	0.94	0.98	0.98	0.98		
Hourly Flow Rate, HFR (veh/h)	0	460	2	22	1013	109		
Percent Heavy Vehicles	2	--	--	3	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	2	0	0	1	1		
Configuration	<i>LT</i>		<i>TR</i>	<i>LT</i>		<i>R</i>		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	22	9	24	54	2	0		
Peak-Hour Factor, PHF	0.66	0.66	0.66	0.79	0.79	0.79		
Hourly Flow Rate, HFR (veh/h)	33	13	36	68	2	0		
Percent Heavy Vehicles	8	8	8	2	2	2		
Percent Grade (%)	0			0				
Flared Approach		<i>N</i>			<i>N</i>			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	1	1	0	1	0		
Configuration	<i>LT</i>		<i>R</i>		<i>LTR</i>			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound		Southbound			
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>	<i>LT</i>	<i>LT</i>		<i>R</i>		<i>LTR</i>	
v (veh/h)	0	22	46		36		70	
C (m) (veh/h)	618	1088	71		753		98	
v/c	0.00	0.02	0.65		0.05		0.71	
95% queue length	0.00	0.06	2.88		0.15		3.66	
Control Delay (s/veh)	10.8	8.4	121.6		10.0		103.7	
LOS	<i>B</i>	<i>A</i>	<i>F</i>		<i>B</i>		<i>F</i>	
Approach Delay (s/veh)	--	--	72.6			103.7		
Approach LOS	--	--	<i>F</i>			<i>F</i>		

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	WFB + JAG		Intersection	US Route 6 and CR 50				
Agency/Co.	TMA		Jurisdiction	Town of Southeast				
Date Performed	10/30/2007		Analysis Year	Build Condition				
Analysis Time Period	P.M. Peak Hour							
Project Description <i>Stateline Retail Center</i>								
East/West Street: <i>US Route 6/202/22</i>			North/South Street: <i>County Road 50</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	128	424			1015	0		
Peak-Hour Factor, PHF	0.98	0.98	1.00	1.00	0.98	0.98		
Hourly Flow Rate, HFR (veh/h)	130	432	0	0	1035	0		
Percent Heavy Vehicles	3	--	--	0	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	1	1	0	0	1	0		
Configuration	L	T				TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				7		134		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.90	1.00	0.90		
Hourly Flow Rate, HFR (veh/h)	0	0	0	7	0	148		
Percent Heavy Vehicles	0	0	0	1	0	1		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	1	0	1		
Configuration				L		R		
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L					L		R
v (veh/h)	130					7		148
C (m) (veh/h)	668					79		283
v/c	0.19					0.09		0.52
95% queue length	0.72					0.28		2.82
Control Delay (s/veh)	11.7					55.0		30.9
LOS	B					F		D
Approach Delay (s/veh)	--	--				32.0		
Approach LOS	--	--				D		

<b>HCS+™ DETAILED REPORT</b>												
<b>General Information</b>						<b>Site Information</b>						
Analyst JAG						Intersection I-84 WB ramps/Saw Mill						
Agency or Co. TMA						Area Type All other areas						
Date Performed 10/29/2007						Jurisdiction Danbury, CT						
Time Period Saturday Peak Hour						Analysis Year Build Condition						
						Project ID Stateline						
<b>Volume and Timing Input</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>l</sub>				1	1	0	1	1			2	0
Lane Group				L	LTR		L	T			TR	
Volume, V (vph)				186	7	159	198	222			250	146
% Heavy Vehicles, %HV				0	0	0	1	1			0	0
Peak-Hour Factor, PHF				0.70	0.70	0.70	0.86	0.86			0.81	0.81
Pretimed (P) or Actuated (A)				A	A	A	A	A			A	A
Start-up Lost Time, I <sub>1</sub>				2.0	2.0		2.0	2.0			2.0	
Extension of Effective Green, e				2.0	2.0		2.0	2.0			2.0	
Arrival Type, AT				3	3		5	5			5	
Unit Extension, UE				3.0	3.0		3.0	3.0			3.0	
Filtering/Metering, I				1.000	1.000		1.000	1.000			1.000	
Initial Unmet Demand, Q <sub>b</sub>				0.0	0.0		0.0	0.0			0.0	
Ped / Bike / RTOR Volumes				0	0	80	0	0		0	0	0
Lane Width				12.0	12.0		11.0	11.0			12.0	
Parking / Grade / Parking				N	-1	N	N	1	N	N	-1	N
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>				0	0		0	0			0	
Min. Time for Pedestrians, G <sub>p</sub>				3.2			3.2			3.2		
Phasing	WB Only		02	03	04	NB Only		SB Only		07	08	
Timing	G = 11.0		G =	G =	G =	G = 26.0		G = 23.0		G =	G =	
	Y = 5		Y =	Y =	Y =	Y = 5		Y = 5		Y =	Y =	
Duration of Analysis, T = 0.25						Cycle Length, C = 75.0						
<b>Lane Group Capacity, Control Delay, and LOS Determination</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v				213	176		230	258			489	
Lane Group Capacity, c				266	249		596	627			1053	
v/c Ratio, X				0.80	0.71		0.39	0.41			0.46	
Total Green Ratio, g/C				0.15	0.15		0.35	0.35			0.31	
Uniform Delay, d <sub>1</sub>				30.9	30.5		18.5	18.7			21.0	
Progression Factor, PF				1.000	1.000		0.646	0.646			0.705	
Delay Calibration, k				0.34	0.27		0.11	0.11			0.11	
Incremental Delay, d <sub>2</sub>				15.9	8.9		0.4	0.4			0.3	
Initial Queue Delay, d <sub>3</sub>				0.0	0.0		0.0	0.0			0.0	
Control Delay				46.9	39.3		12.4	12.5			15.1	
Lane Group LOS				D	D		B	B			B	
Approach Delay				43.5			12.4			15.1		
Approach LOS				D			B			B		
Intersection Delay	22.2			X <sub>c</sub> = 0.50			Intersection LOS			C		

<b>HCS+™ DETAILED REPORT</b>													
<b>General Information</b>						<b>Site Information</b>							
Analyst JAG						Intersection US 6 and Saw Mill Rd							
Agency or Co. TMA						Area Type All other areas							
Date Performed 10/29/2007						Jurisdiction Danbury, CT							
Time Period Saturday Peak Hour						Analysis Year Build Condition							
						Project ID Stateline							
<b>Volume and Timing Input</b>													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N <sub>1</sub>		2	0	1	1		2		0				
Lane Group		TR		L	T		L	LR					
Volume, V (vph)		233	316	89	290		287		89				
% Heavy Vehicles, %HV		0	0	0	0		0		0				
Peak-Hour Factor, PHF		0.92	0.92	0.94	0.94		0.83		0.83				
Pretimed (P) or Actuated (A)		A	A	A	A		A		A				
Start-up Lost Time, I <sub>1</sub>		2.0		2.0	2.0		2.0	2.0					
Extension of Effective Green, e		2.0		2.0	2.0		2.0	2.0					
Arrival Type, AT		3		3	3		5	3					
Unit Extension, UE		3.0		3.0	3.0		3.0	3.0					
Filtering/Metering, I		1.000		1.000	1.000		1.000	1.000					
Initial Unmet Demand, Q <sub>b</sub>		0.0		0.0	0.0		0.0	0.0					
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0				
Lane Width		13.0		13.0	14.0		11.0	11.0					
Parking / Grade / Parking	N	0	N	N	0	N	N	0	N				
Parking Maneuvers, N <sub>m</sub>													
Buses Stopping, N <sub>b</sub>		0		0	0		0	0					
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2			3.2					
Phasing	EW Perm	WB Only	03			04			NB Only	06		07	08
Timing	G = 15.0	G = 8.0	G =			G =			G = 37.0	G =		G =	
	Y = 5	Y = 5	Y =			Y =			Y = 5	Y =		Y =	
Duration of Analysis, T = 0.25							Cycle Length, C = 75.0						
<b>Lane Group Capacity, Control Delay, and LOS Determination</b>													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		596		95	309		190	263					
Lane Group Capacity, c		683		503	757		1671	826					
v/c Ratio, X		0.87		0.19	0.41		0.11	0.32					
Total Green Ratio, g/C		0.20		0.41	0.37		0.49	0.49					
Uniform Delay, d <sub>1</sub>		29.1		19.5	17.4		10.2	11.4					
Progression Factor, PF		1.000		1.000	1.000		0.351	1.000					
Delay Calibration, k		0.40		0.11	0.11		0.11	0.11					
Incremental Delay, d <sub>2</sub>		12.0		0.2	0.4		0.0	0.2					
Initial Queue Delay, d <sub>3</sub>		0.0		0.0	0.0		0.0	0.0					
Control Delay		41.0		19.7	17.7		3.6	11.6					
Lane Group LOS		D		B	B		A	B					
Approach Delay	41.0			18.2			8.3						
Approach LOS	D			B			A						
Intersection Delay	24.5			X <sub>c</sub> = 0.39			Intersection LOS			C			



HCS+™ DETAILED REPORT													
General Information						Site Information							
Analyst	JAG					Intersection	US 6, Starr, and NYS 22 ramp						
Agency or Co.	TMA					Area Type	All other areas						
Date Performed	10/29/2007					Jurisdiction	Town of Southeast						
Time Period	Saturday Peak Hour					Analysis Year	Build Condition						
						Project ID	Stateline						
Volume and Timing Input													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N <sub>1</sub>	0	2	0	0	2	0	0	1	0	1	1	0	
Lane Group		LTR			LTR			LTR		L	TR		
Volume, V (vph)	228	551	92	40	581	136	115	31	19	265	31	166	
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1	
Peak-Hour Factor, PHF	0.95	0.95	0.95	0.93	0.93	0.93	0.95	0.95	0.95	0.76	0.76	0.76	
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A	
Start-up Lost Time, I <sub>1</sub>		2.0			2.0			2.0		2.0	2.0		
Extension of Effective Green, e		2.0			2.0			2.0		2.0	2.0		
Arrival Type, AT		3			3			3		3	3		
Unit Extension, UE		3.0			3.0			3.0		3.0	3.0		
Filtering/Metering, I		1.000			1.000			1.000		1.000	1.000		
Initial Unmet Demand, Q <sub>b</sub>		0.0			0.0			0.0		0.0	0.0		
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0	
Lane Width		12.0			12.0			15.0		10.0	9.0		
Parking / Grade / Parking	N	5	N	N	-2	N	N	-6	N	N	-5	N	
Parking Maneuvers, N <sub>m</sub>													
Buses Stopping, N <sub>b</sub>		0			0			0		0	0		
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2			3.2			3.2		
Phasing	EW Perm	02	03	04	NS Perm	06	07	08					
Timing	G = 35.0	G =	G =	G =	G = 20.0	G =	G =	G =					
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =					
Duration of Analysis, T = 0.25							Cycle Length, C = 65.0						
Lane Group Capacity, Control Delay, and LOS Determination													
	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Adjusted Flow Rate, v		917			814			174		349	259		
Lane Group Capacity, c		1104			1647			293		374	466		
v/c Ratio, X		0.83			0.49			0.59		0.93	0.56		
Total Green Ratio, g/C		0.54			0.54			0.31		0.31	0.31		
Uniform Delay, d <sub>1</sub>		12.5			9.4			19.1		21.9	18.8		
Progression Factor, PF		1.000			1.000			1.000		1.000	1.000		
Delay Calibration, k		0.37			0.11			0.18		0.45	0.15		
Incremental Delay, d <sub>2</sub>		5.5			0.2			3.2		30.1	1.5		
Initial Queue Delay, d <sub>3</sub>		0.0			0.0			0.0		0.0	0.0		
Control Delay		18.0			9.7			22.3		51.9	20.3		
Lane Group LOS		B			A			C		D	C		
Approach Delay	18.0			9.7			22.3			38.4			
Approach LOS	B			A			C			D			
Intersection Delay	20.5			X <sub>c</sub> = 0.87			Intersection LOS			C			

<b>HCS+™ DETAILED REPORT</b>												
<b>General Information</b>						<b>Site Information</b>						
Analyst	JAG					Intersection	Argonne /Rt 22 ramps/US 6					
Agency or Co.	TMA					Area Type	All other areas					
Date Performed	10/29/07					Jurisdiction	Town of Southeast, NY					
Time Period	Saturday Peak Hour					Analysis Year	Build Condition					
						Project ID	Stateline					
<b>Volume and Timing Input</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>l</sub>	0	1	0	1	1	0	0	2	0	0	2	0
Lane Group	LTR			L			LTR			DefL		
Volume, V (vph)	5	59	252	186	5	161	13	579	301	165	451	20
% Heavy Vehicles, %HV	1	1	1	1	1	1	1	1	1	1	1	1
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.84	0.84	0.84	0.86	0.86	0.86	0.91	0.91	0.91
Pretimed (P) or Actuated (A)	A	A	A	A	A	A	A	A	A	A	A	A
Start-up Lost Time, I <sub>1</sub>		2.0		2.0	2.0			2.0		2.0	2.0	
Extension of Effective Green, e		2.0		2.0	2.0			2.0		2.0	2.0	
Arrival Type, AT		3		3	3			3		3	3	
Unit Extension, UE		3.0		3.0	3.0			3.0		3.0	3.0	
Filtering/Metering, I		1.000		1.000	1.000			1.000		1.000	1.000	
Initial Unmet Demand, Q <sub>b</sub>		0.0		0.0	0.0			0.0		0.0	0.0	
Ped / Bike / RTOR Volumes	0	0	0	0	0	0	0	0	0	0	0	0
Lane Width		15.0		9.0	9.0			12.0		12.0	12.0	
Parking / Grade / Parking	N	0	N	N	-6	N	N	-4	N	N	4	N
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0		0	0			0		0	0	
Min. Time for Pedestrians, G <sub>p</sub>	3.2			3.2			3.2			3.2		
Phasing	EW Perm	02	03	04	NS Perm	SB Only	07	08				
Timing	G = 20.0	G =	G =	G =	G = 23.0	G = 2.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 60.0						
<b>Lane Group Capacity, Control Delay, and LOS Determination</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		352		221	198			1038		181	518	
Lane Group Capacity, c		613		261	497			1257		415	916	
v/c Ratio, X		0.57		0.85	0.40			0.83		0.44	0.57	
Total Green Ratio, g/C		0.33		0.33	0.33			0.38		0.55	0.50	
Uniform Delay, d <sub>1</sub>		16.5		18.6	15.4			16.7		16.9	10.5	
Progression Factor, PF		1.000		1.000	1.000			1.000		1.000	1.000	
Delay Calibration, k		0.17		0.38	0.11			0.36		0.11	0.16	
Incremental Delay, d <sub>2</sub>		1.3		22.0	0.5			4.7		0.7	0.8	
Initial Queue Delay, d <sub>3</sub>		0.0		0.0	0.0			0.0		0.0	0.0	
Control Delay		17.8		40.6	15.9			21.4		17.6	11.3	
Lane Group LOS		B		D	B			C		B	B	
Approach Delay	17.8			28.9			21.4			12.9		
Approach LOS	B			C			C			B		
Intersection Delay	19.8			X <sub>c</sub> = 1.26			Intersection LOS			B		

<b>HCS+™ DETAILED REPORT</b>												
<b>General Information</b>						<b>Site Information</b>						
Analyst <i>WFB + JAG</i>						Intersection <i>Rt 6 (East Main) &amp; 202/22</i>						
Agency or Co. <i>TMA</i>						Area Type <i>All other areas</i>						
Date Performed <i>10/30/2007</i>						Jurisdiction <i>Town of Southeast</i>						
Time Period <i>Saturday Peak Hour</i>						Analysis Year <i>Build Condition</i>						
						Project ID <i>Stateline Retail Center</i>						
<b>Volume and Timing Input</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>	0	1			1	1				1		1
Lane Group		LT			T	R				L		R
Volume, V (vph)	49	212			165	495				520		38
% Heavy Vehicles, %HV	5	5			1	1				2		2
Peak-Hour Factor, PHF	0.91	0.91			0.98	0.98				0.95		0.95
Pretimed (P) or Actuated (A)	A	A			A	A				A		A
Start-up Lost Time, I <sub>1</sub>		2.0			2.0	2.0				2.0		2.0
Extension of Effective Green, e		2.0			2.0	2.0				2.0		2.0
Arrival Type, AT		3			3	3				3		3
Unit Extension, UE		3.0			3.0	3.0				3.0		3.0
Filtering/Metering, I		1.000			1.000	1.000				1.000		1.000
Initial Unmet Demand, Q <sub>b</sub>		0.0			0.0	0.0				0.0		0.0
Ped / Bike / RTOR Volumes	0	0		0	0	0				0	0	0
Lane Width		13.0			11.0	11.0				12.0		11.0
Parking / Grade / Parking	N	0	N	N	0	N				N	0	N
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0			0	0				0		0
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2					3.2		
Phasing	EW Perm	02	03	04	SB Only	06	07	08				
Timing	G = 39.0	G =	G =	G =	G = 36.0	G =	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 85.0					
<b>Lane Group Capacity, Control Delay, and LOS Determination</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		287			168	505				547		40
Lane Group Capacity, c		786			834	1546				750		648
v/c Ratio, X		0.37			0.20	0.33				0.73		0.06
Total Green Ratio, g/C		0.46			0.46	1.00				0.42		0.42
Uniform Delay, d <sub>1</sub>		15.0			13.7	0.0				20.4		14.5
Progression Factor, PF		1.000			1.000	0.950				1.000		1.000
Delay Calibration, k		0.11			0.11	0.11				0.29		0.11
Incremental Delay, d <sub>2</sub>		0.3			0.1	0.1				3.6		0.0
Initial Queue Delay, d <sub>3</sub>		0.0			0.0	0.0				0.0		0.0
Control Delay		15.2			13.8	0.1				24.1		14.5
Lane Group LOS		B			B	A				C		B
Approach Delay	15.2			3.5						23.4		
Approach LOS	B			A						C		
Intersection Delay	13.3			X <sub>c</sub> = 0.54			Intersection LOS			B		

HCS+™ DETAILED REPORT												
General Information						Site Information						
Analyst JAG Agency or Co. TMA Date Performed 10/28/2007 Time Period Saturday Peak Hour						Intersection Saw Mill Rd / I-84 EB ramps Area Type All other areas Jurisdiction Danbury, CT Analysis Year Build Condition Project ID Stateline						
Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>	0	1	1					1	1	0	2	
Lane Group		LT	R					T	R		LT	
Volume, V (vph)	124	0	240					286	225	153	305	
% Heavy Vehicles, %HV	0	0	0					2	2	0	0	
Peak-Hour Factor, PHF	0.84	0.84	0.84					0.86	0.86	0.92	0.92	
Pretimed (P) or Actuated (A)	A	A	A					A	A	A	A	
Start-up Lost Time, I <sub>1</sub>		2.0	2.0					2.0	2.0		2.0	
Extension of Effective Green, e		2.0	2.0					2.0	2.0		2.0	
Arrival Type, AT		3	3					3	3		5	
Unit Extension, UE		3.0	3.0					3.0	3.0		3.0	
Filtering/Metering, I		1.000	1.000					1.000	1.000		1.000	
Initial Unmet Demand, Q <sub>b</sub>		0.0	0.0					0.0	0.0		0.0	
Ped / Bike / RTOR Volumes	0	0	0				0	0	0	0	0	
Lane Width		12.0	12.0					11.0	11.0		11.0	
Parking / Grade / Parking	N	3	N				N	8	N	N	-5	N
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0	0					0	0		0	
Min. Time for Pedestrians, G <sub>p</sub>		3.2						3.2			3.2	
Phasing	EB Only	02	03	04	SB Only	NB Only	07	08				
Timing	G = 10.0	G =	G =	G =	G = 34.0	G = 16.0	G =	G =				
	Y = 5	Y =	Y =	Y =	Y = 5	Y = 5	Y =	Y =				
Duration of Analysis, T = 0.25						Cycle Length, C = 75.0						
Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		148	286					333	262		498	
Lane Group Capacity, c		238	658					369	313		1598	
v/c Ratio, X		0.62	0.43					0.90	0.84		0.31	
Total Green Ratio, g/C		0.13	0.41					0.21	0.21		0.45	
Uniform Delay, d <sub>1</sub>		30.7	15.7					28.7	28.3		13.1	
Progression Factor, PF		1.000	1.000					1.000	1.000		0.447	
Delay Calibration, k		0.21	0.11					0.42	0.37		0.11	
Incremental Delay, d <sub>2</sub>		5.0	0.5					24.5	17.7		0.1	
Initial Queue Delay, d <sub>3</sub>		0.0	0.0					0.0	0.0		0.0	
Control Delay		35.7	16.2					53.3	46.0		5.9	
Lane Group LOS		D	B					D	D		A	
Approach Delay	22.8						50.1			5.9		
Approach LOS	C						D			A		
Intersection Delay	27.9			X <sub>c</sub> = 0.52			Intersection LOS			C		

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	JAG		Intersection	US 6/Dingle Ridge Road				
Agency/Co.	TMA		Jurisdiction	Town of Southeast, NY				
Date Performed	10/29/2007		Analysis Year	Build Condition				
Analysis Time Period	Saturday Peak Hour							
Project Description <i>Stateline</i>								
East/West Street: <i>US Route 6 &amp; 202 (Danbury Rd)</i>			North/South Street: <i>Dingle Ridge Road</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	0	499	41	38	532	0		
Peak-Hour Factor, PHF	0.84	0.84	0.84	0.89	0.89	0.89		
Hourly Flow Rate, HFR (veh/h)	0	594	48	42	597	0		
Percent Heavy Vehicles	1	--	--	1	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	2	0	0	2	0		
Configuration	LT		TR	LT		TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	38	0	34	0	0	1		
Peak-Hour Factor, PHF	0.88	0.88	0.88	0.25	0.25	0.25		
Hourly Flow Rate, HFR (veh/h)	43	0	38	0	0	4		
Percent Heavy Vehicles	1	1	1	0	0	0		
Percent Grade (%)	2			2				
Flared Approach		Y			N			
Storage		1			0			
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration		LTR			LTR			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT	LT		LTR			LTR	
v (veh/h)	0	42		81			4	
C (m) (veh/h)	983	945		358			704	
v/c	0.00	0.04		0.23			0.01	
95% queue length	0.00	0.14		0.86			0.02	
Control Delay (s/veh)	8.7	9.0		20.6			10.1	
LOS	A	A		C			B	
Approach Delay (s/veh)	--	--		20.6			10.1	
Approach LOS	--	--		C			B	

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	JAG		Intersection	Joe's Hill Road				
Agency/Co.	TMA		Jurisdiction	Town of Southeast, NY				
Date Performed	10/29/2007		Analysis Year	Build Condition				
Analysis Time Period	Saturday Peak Hour							
Project Description <i>Stateline</i>								
East/West Street: <i>US Route 6 &amp; 202</i>			North/South Street: <i>Joe's Hill Road</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	18	526			549	19		
Peak-Hour Factor, PHF	0.83	0.83	1.00	1.00	0.87	0.87		
Hourly Flow Rate, HFR (veh/h)	21	633	0	0	631	21		
Percent Heavy Vehicles	1	--	--	0	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	2	0	0	2	0		
Configuration	<i>LT</i>	<i>T</i>			<i>T</i>	<i>TR</i>		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				11		8		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.65	1.00	0.65		
Hourly Flow Rate, HFR (veh/h)	0	0	0	16	0	12		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	0			2				
Flared Approach		<i>N</i>			<i>N</i>			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					<i>LR</i>			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>						<i>LR</i>	
v (veh/h)	21						28	
C (m) (veh/h)	937						329	
v/c	0.02						0.09	
95% queue length	0.07						0.28	
Control Delay (s/veh)	8.9						17.0	
LOS	<i>A</i>						<i>C</i>	
Approach Delay (s/veh)	--	--					17.0	
Approach LOS	--	--					<i>C</i>	

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	JAG		Intersection	US Rte 6/NYS 121				
Agency/Co.	TMA		Jurisdiction	Town of Southeast, NY				
Date Performed	10/29/2007		Analysis Year	Build Condition				
Analysis Time Period	Saturday Peak Hour							
Project Description <i>Stateline</i>								
East/West Street: <i>US Route 6 &amp; 202</i>			North/South Street: <i>NYS Rte 121 Peach Lake</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		672	194	127	596			
Peak-Hour Factor, PHF	1.00	0.80	0.80	0.90	0.90	1.00		
Hourly Flow Rate, HFR (veh/h)	0	839	242	141	662	0		
Percent Heavy Vehicles	0	--	--	1	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			1			0		
Lanes	0	2	1	0	2	0		
Configuration		T	R	LT	T			
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	179		104					
Peak-Hour Factor, PHF	0.94	1.00	0.94	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	190	0	110	0	0	0		
Percent Heavy Vehicles	2	0	2	0	0	0		
Percent Grade (%)	-4			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	1	0	1	0	0	0		
Configuration	L		R					
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT	L		R			
v (veh/h)		141	190		110			
C (m) (veh/h)		798	100		582			
v/c		0.18	1.90		0.19			
95% queue length		0.64	15.77		0.69			
Control Delay (s/veh)		10.5	511.4		12.6			
LOS		B	F		B			
Approach Delay (s/veh)	--	--	328.5					
Approach LOS	--	--	F					

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	JAG		Intersection	Route 121/ I-84 off-ramp				
Agency/Co.	TMA		Jurisdiction	Town of Southeast				
Date Performed	10/29/07		Analysis Year	Build Condition				
Analysis Time Period	Saturday Peak Hour							
Project Description <i>Stateline</i>								
East/West Street: <i>I-84 off-ramp</i>			North/South Street: <i>Peach Lake Rd (NYS Rt 121)</i>					
Intersection Orientation: <i>North-South</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		143			296			
Peak-Hour Factor, PHF	1.00	0.79	1.00	1.00	0.93	1.00		
Hourly Flow Rate, HFR (veh/h)	0	181	0	0	318	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration		T			T			
Upstream Signal		0			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				111		136		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.89	1.00	0.89		
Hourly Flow Rate, HFR (veh/h)	0	0	0	124	0	152		
Percent Heavy Vehicles	0	0	0	2	0	2		
Percent Grade (%)		0			-3			
Flared Approach		N			Y			
Storage		0			3			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration					LR			
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration				LR				
v (veh/h)				276				
C (m) (veh/h)				1182				
v/c				0.23				
95% queue length				0.91				
Control Delay (s/veh)				11.8				
LOS				B				
Approach Delay (s/veh)	--	--		11.8				
Approach LOS	--	--		B				



## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	JAG		Intersection	Rt 121/I-84 on-ramp				
Agency/Co.	TMA		Jurisdiction	Town of Southeast				
Date Performed	10/29/07		Analysis Year	Build Condition				
Analysis Time Period	Saturday Peak Hour							
Project Description <i>Stateline</i>								
East/West Street: <i>I-84 on-ramp</i>			North/South Street: <i>Rt. 121 Peach Lake</i>					
Intersection Orientation: <i>North-South</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Northbound			Southbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		143	138	141	254			
Peak-Hour Factor, PHF	1.00	0.83	0.83	0.94	0.94	1.00		
Hourly Flow Rate, HFR (veh/h)	0	172	166	150	270	0		
Percent Heavy Vehicles	0	--	--	1	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	1	0	0	1	0		
Configuration			TR	LT				
Upstream Signal		0			0			
Minor Street	Eastbound			Westbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)								
Peak-Hour Factor, PHF	1.00	1.00	1.00	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration								
Delay, Queue Length, and Level of Service								
Approach	Northbound	Southbound	Westbound			Eastbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT						
v (veh/h)		150						
C (m) (veh/h)		1227						
v/c		0.12						
95% queue length		0.42						
Control Delay (s/veh)		8.3						
LOS		A						
Approach Delay (s/veh)	--	--						
Approach LOS	--	--						

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	WFB + JAG		Intersection	Route 6 & Sodom				
Agency/Co.	TMA		Jurisdiction	Town of Southeast				
Date Performed	10/30/2007		Analysis Year	Build Condition				
Analysis Time Period	Saturday Peak Hour							
Project Description <i>Stateline Retail Center</i>								
East/West Street: <i>US Route 6/202/22</i>			North/South Street: <i>Sodom Road/Driveway</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	0	446	29	8	641	54		
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.89	0.89	0.89		
Hourly Flow Rate, HFR (veh/h)	0	484	31	8	720	60		
Percent Heavy Vehicles	2	--	--	1	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	2	0	0	1	1		
Configuration	<i>LT</i>		<i>TR</i>	<i>LT</i>		<i>R</i>		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	7	0	26	52	0	0		
Peak-Hour Factor, PHF	0.80	0.80	0.80	0.78	0.78	0.78		
Hourly Flow Rate, HFR (veh/h)	8	0	32	66	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	0			0				
Flared Approach		<i>N</i>			<i>N</i>			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	1	1	0	1	0		
Configuration	<i>LT</i>		<i>R</i>		<i>LTR</i>			
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound		Southbound			
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>	<i>LT</i>	<i>LT</i>		<i>R</i>		<i>LTR</i>	
v (veh/h)	0	8	8		32		66	
C (m) (veh/h)	833	1054	127		747		197	
v/c	0.00	0.01	0.06		0.04		0.34	
95% queue length	0.00	0.02	0.20		0.13		1.39	
Control Delay (s/veh)	9.3	8.4	35.2		10.0		32.2	
LOS	<i>A</i>	<i>A</i>	<i>E</i>		<i>B</i>		<i>D</i>	
Approach Delay (s/veh)	--	--	15.1			32.2		
Approach LOS	--	--	<i>C</i>			<i>D</i>		

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	WFB + JAG		Intersection	US Route 6 and CR 50				
Agency/Co.	TMA		Jurisdiction	Town of Southeast				
Date Performed	10/30/2007		Analysis Year	Build Condition				
Analysis Time Period	Saturday Peak Hour							
Project Description <i>Stateline Retail Center</i>								
East/West Street: <i>US Route 6/202/22</i>			North/South Street: <i>County Road 50</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)	70	490			583	6		
Peak-Hour Factor, PHF	0.85	0.85	1.00	1.00	0.86	0.86		
Hourly Flow Rate, HFR (veh/h)	82	576	0	0	677	6		
Percent Heavy Vehicles	1	--	--	0	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	1	1	0	0	1	0		
Configuration	L	T				TR		
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)				10		115		
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.85	1.00	0.85		
Hourly Flow Rate, HFR (veh/h)	0	0	0	11	0	135		
Percent Heavy Vehicles	0	0	0	2	0	2		
Percent Grade (%)	0			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	1	0	1		
Configuration				L		R		
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	L					L		R
v (veh/h)	82					11		135
C (m) (veh/h)	915					137		451
v/c	0.09					0.08		0.30
95% queue length	0.29					0.26		1.24
Control Delay (s/veh)	9.3					33.6		16.4
LOS	A					D		C
Approach Delay (s/veh)	--	--				17.7		
Approach LOS	--	--				C		

### HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>US 6 and East site Access</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>11/27/07</i>	Jurisdiction <i>Town of Southeast</i>
Time Period <i>A.M. Peak Hour</i>	Analysis Year <i>Build w/ 5 lane Rt 6</i>
	Project ID <i>Stateline</i>

#### Volume and Timing Input

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>		2	0	1	2		1		1			
Lane Group		TR		L	T		L		R			
Volume, V (vph)		211	19	38	1178		48		36			
% Heavy Vehicles, %HV		5	5	3	3		2		2			
Peak-Hour Factor, PHF		0.73	0.73	0.91	0.91		0.95		0.95			
Pretimed (P) or Actuated (A)		A	A	A	A		A		A			
Start-up Lost Time, I <sub>1</sub>		2.0		2.0	2.0		2.0		2.0			
Extension of Effective Green, e		2.0		2.0	2.0		2.0		2.0			
Arrival Type, AT		3		3	3		3		3			
Unit Extension, UE		3.0		3.0	3.0		3.0		3.0			
Filtering/Metering, I		1.000		1.000	1.000		1.000		1.000			
Initial Unmet Demand, Q <sub>b</sub>		0.0		0.0	0.0		0.0		0.0			
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0			
Lane Width		11.0		11.0	12.0		12.0		12.0			
Parking / Grade / Parking	N	0	N	N	0	N	N	-3	N			
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0		0	0		0		0			
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2			3.2				
Phasing	EW Perm	WB Only	03		04		NB Only	06		07		08
Timing	G = 30.0	G = 10.0	G =	G =	G = 15.0	G =	G =	G =	G =			
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =	Y =			
Duration of Analysis, T = 0.25							Cycle Length, C = 70.0					

#### Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		315		42	1295		51		38			
Lane Group Capacity, c		1410		865	2258		385		689			
v/c Ratio, X		0.22		0.05	0.57		0.13		0.06			
Total Green Ratio, g/C		0.43		0.69	0.64		0.21		0.43			
Uniform Delay, d <sub>1</sub>		12.6		3.6	7.1		22.2		11.7			
Progression Factor, PF		1.000		1.000	1.000		1.000		1.000			
Delay Calibration, k		0.11		0.11	0.17		0.11		0.11			
Incremental Delay, d <sub>2</sub>		0.1		0.0	0.4		0.2		0.0			
Initial Queue Delay, d <sub>3</sub>		0.0		0.0	0.0		0.0		0.0			
Control Delay		12.7		3.6	7.4		22.4		11.7			
Lane Group LOS		B		A	A		C		B			
Approach Delay	12.7			7.3			17.8					
Approach LOS	B			A			B					
Intersection Delay	8.8			X <sub>c</sub> = 0.46			Intersection LOS			A		

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	JAG		Intersection	US 6 & 202/ western access				
Agency/Co.	TMA		Jurisdiction	Town of Southeast				
Date Performed	10/18/07		Analysis Year	Build Condition				
Analysis Time Period	A.M. Peak Hour							
Project Description <i>Stateline</i>								
East/West Street: <i>US Route 6 &amp; 202</i>			North/South Street: <i>Western Access right in/out</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		303	27	9	1217			
Peak-Hour Factor, PHF	1.00	0.73	0.73	0.92	0.92	1.00		
Hourly Flow Rate, HFR (veh/h)	0	415	36	9	1322	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	2	0	0	2	0		
Configuration		T	TR	LT	T			
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	3		2					
Peak-Hour Factor, PHF	0.95	1.00	0.95	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	3	0	2	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	-3			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration		LR						
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT		LR				
v (veh/h)		9		5				
C (m) (veh/h)		1120		290				
v/c		0.01		0.02				
95% queue length		0.02		0.05				
Control Delay (s/veh)		8.2		17.6				
LOS		A		C				
Approach Delay (s/veh)	--	--	17.6					
Approach LOS	--	--	C					

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	JAG		Intersection	US 6 & 202/ center access				
Agency/Co.	TMA		Jurisdiction	Town of Southeast				
Date Performed	10/28/07		Analysis Year	Build Condition				
Analysis Time Period	A.M. Peak Hour							
Project Description <i>Stateline</i>								
East/West Street: <i>US Route 6 &amp; 202</i>			North/South Street: <i>Center Access right in/out</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		224	81		1226			
Peak-Hour Factor, PHF	1.00	0.73	0.73	1.00	0.92	1.00		
Hourly Flow Rate, HFR (veh/h)	0	306	110	0	1332	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	2	0	0	2	0		
Configuration		T	TR		T			
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)			6					
Peak-Hour Factor, PHF	1.00	1.00	0.95	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	0	0	6	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	-3			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	1	0	0	0		
Configuration			R					
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration					R			
v (veh/h)					6			
C (m) (veh/h)					804			
v/c					0.01			
95% queue length					0.02			
Control Delay (s/veh)					9.5			
LOS					A			
Approach Delay (s/veh)	--	--	9.5					
Approach LOS	--	--	A					

### HCS+™ DETAILED REPORT

General Information	Site Information
Analyst           JAG	Intersection     US 6 and East site Access
Agency or Co.   TMA	Area Type       All other areas
Date Performed  11/28/07	Jurisdiction     Town of Southeast
Time Period     P.M. Peak Hour	Analysis Year   Build w/ 5 Lane Rt 6
	Project ID       Stateline

#### Volume and Timing Input

	EB			WB			NB			SB			
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT	
Number of Lanes, N <sub>1</sub>		2	0	1	2		1		1				
Lane Group		TR		L	T		L		R				
Volume, V (vph)		649	58	143	378		293		167				
% Heavy Vehicles, %HV		1	1	1	1		1		1				
Peak-Hour Factor, PHF		0.95	0.95	0.93	0.93		0.95		0.95				
Pretimed (P) or Actuated (A)		A	A	A	A		A		A				
Start-up Lost Time, I <sub>1</sub>		2.0		2.0	2.0		2.0		2.0				
Extension of Effective Green, e		2.0		2.0	2.0		2.0		2.0				
Arrival Type, AT		3		3	3		3		3				
Unit Extension, UE		3.0		3.0	3.0		3.0		3.0				
Filtering/Metering, I		1.000		1.000	1.000		1.000		1.000				
Initial Unmet Demand, Q <sub>b</sub>		0.0		0.0	0.0		0.0		0.0				
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0				
Lane Width		11.0		11.0	12.0		12.0		12.0				
Parking / Grade / Parking	N	0	N	N	0	N	N	-3	N				
Parking Maneuvers, N <sub>m</sub>													
Buses Stopping, N <sub>b</sub>		0		0	0		0		0				
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2			3.2					
Phasing	EW Perm	WB Only	03			04			NB Only	06		07	08
Timing	G = 30.0	G = 10.0	G =	G =			G = 20.0			G =	G =	G =	
	Y = 5	Y = 5	Y =	Y =			Y = 5			Y =	Y =	Y =	
Duration of Analysis, T = 0.25							Cycle Length, C = 75.0						

#### Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		744		154	406		308		176			
Lane Group Capacity, c		1368		617	2149		484		757			
v/c Ratio, X		0.54		0.25	0.19		0.64		0.23			
Total Green Ratio, g/C		0.40		0.64	0.60		0.27		0.47			
Uniform Delay, d <sub>1</sub>		17.3		10.3	6.8		24.3		12.0			
Progression Factor, PF		1.000		1.000	1.000		1.000		1.000			
Delay Calibration, k		0.14		0.11	0.11		0.22		0.11			
Incremental Delay, d <sub>2</sub>		0.5		0.2	0.0		2.8		0.2			
Initial Queue Delay, d <sub>3</sub>		0.0		0.0	0.0		0.0		0.0			
Control Delay		17.7		10.5	6.8		27.1		12.1			
Lane Group LOS		B		B	A		C		B			
Approach Delay	17.7			7.8			21.6					
Approach LOS	B			A			C					
Intersection Delay	15.7			X <sub>c</sub> = 0.55			Intersection LOS			B		

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	JAG		Intersection	US 6 & 202/ western access				
Agency/Co.	TMA		Jurisdiction	Town of Southeast				
Date Performed	10/28/07		Analysis Year	Build Condition				
Analysis Time Period	P.M. Peak Hour							
Project Description <i>Stateline</i>								
East/West Street: <i>US Route 6 &amp; 202</i>			North/South Street: <i>Western Access right in/out</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		916	5	2	669			
Peak-Hour Factor, PHF	1.00	0.95	0.95	0.93	0.93	1.00		
Hourly Flow Rate, HFR (veh/h)	0	964	5	2	719	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	2	0	0	2	0		
Configuration		T	TR	LT	T			
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	22		13					
Peak-Hour Factor, PHF	0.95	1.00	0.95	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	23	0	13	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	-3			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration		LR						
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT		LR				
v (veh/h)		2		36				
C (m) (veh/h)		719		201				
v/c		0.00		0.18				
95% queue length		0.01		0.63				
Control Delay (s/veh)		10.0		26.8				
LOS		B		D				
Approach Delay (s/veh)	--	--	26.8					
Approach LOS	--	--	D					



## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	JAG		Intersection	US 6 & 202/ Center access				
Agency/Co.	TMA		Jurisdiction	Town of Southeast				
Date Performed	10/18/07		Analysis Year	Build Condition				
Analysis Time Period	P.M. Peak Hour							
Project Description <i>Stateline</i>								
East/West Street: <i>US Route 6 &amp; 202</i>			North/South Street: <i>Center Access right in/out</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		670	259		671			
Peak-Hour Factor, PHF	1.00	0.95	0.95	1.00	0.93	1.00		
Hourly Flow Rate, HFR (veh/h)	0	705	272	0	721	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	2	0	0	2	0		
Configuration		T	TR		T			
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)			37					
Peak-Hour Factor, PHF	1.00	1.00	0.95	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	0	0	38	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	-3			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	1	0	0	0		
Configuration			R					
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration					R			
v (veh/h)					38			
C (m) (veh/h)					531			
v/c					0.07			
95% queue length					0.23			
Control Delay (s/veh)					12.3			
LOS					B			
Approach Delay (s/veh)	--	--	12.3					
Approach LOS	--	--	B					

### HCS+™ DETAILED REPORT

General Information	Site Information
Analyst <i>JAG</i>	Intersection <i>US 6 and East site Access</i>
Agency or Co. <i>TMA</i>	Area Type <i>All other areas</i>
Date Performed <i>11/28/07</i>	Jurisdiction <i>Town of Southeast</i>
Time Period <i>Saturday Peak Hour</i>	Analysis Year <i>Build w/ 5 lane Rt 6</i>
	Project ID <i>Stateline</i>

#### Volume and Timing Input

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>		2	0	1	2		1		1			
Lane Group		TR		L	T		L		R			
Volume, V (vph)		345	88	213	345		374		199			
% Heavy Vehicles, %HV		1	1	1	1		1		1			
Peak-Hour Factor, PHF		0.84	0.84	0.89	0.89		0.95		0.95			
Pretimed (P) or Actuated (A)		A	A	A	A		A		A			
Start-up Lost Time, I <sub>1</sub>		2.0		2.0	2.0		2.0		2.0			
Extension of Effective Green, e		2.0		2.0	2.0		2.0		2.0			
Arrival Type, AT		3		3	3		3		3			
Unit Extension, UE		3.0		3.0	3.0		3.0		3.0			
Filtering/Metering, I		1.000		1.000	1.000		1.000		1.000			
Initial Unmet Demand, Q <sub>b</sub>		0.0		0.0	0.0		0.0		0.0			
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0			
Lane Width		11.0		11.0	12.0		12.0		12.0			
Parking / Grade / Parking	N	0	N	N	0	N	N	-3	N			
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0		0	0		0		0			
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2			3.2				
Phasing	EW Perm	WB Only	03	04	NB Only	06	07	08				
Timing	G = 22.0	G = 13.0	G =	G =	G = 20.0	G =	G =	G =				
	Y = 5	Y = 5	Y =	Y =	Y = 5	Y =	Y =	Y =				
Duration of Analysis, T = 0.25							Cycle Length, C = 70.0					

#### Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		516		239	388		394		209			
Lane Group Capacity, c		1055		740	2047		518		881			
v/c Ratio, X		0.49		0.32	0.19		0.76		0.24			
Total Green Ratio, g/C		0.31		0.61	0.57		0.29		0.54			
Uniform Delay, d <sub>1</sub>		19.4		9.3	7.2		22.8		8.4			
Progression Factor, PF		1.000		1.000	1.000		1.000		1.000			
Delay Calibration, k		0.11		0.11	0.11		0.31		0.11			
Incremental Delay, d <sub>2</sub>		0.4		0.3	0.0		6.5		0.1			
Initial Queue Delay, d <sub>3</sub>		0.0		0.0	0.0		0.0		0.0			
Control Delay		19.8		9.5	7.3		29.3		8.5			
Lane Group LOS		B		A	A		C		A			
Approach Delay	19.8			8.1			22.1					
Approach LOS	B			A			C					
Intersection Delay	16.4			X <sub>c</sub> = 0.63			Intersection LOS			B		

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	JAG		Intersection	US 6 & 202/ western access				
Agency/Co.	TMA		Jurisdiction	Town of Southeast				
Date Performed	10/18/07		Analysis Year	Build Condition				
Analysis Time Period	Saturday Peak Hour							
Project Description <i>Stateline</i>								
East/West Street: <i>US Route 6 &amp; 202</i>			North/South Street: <i>Western Access right in/out</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		756	3	1	718			
Peak-Hour Factor, PHF	1.00	0.84	0.84	0.89	0.89	1.00		
Hourly Flow Rate, HFR (veh/h)	0	900	3	1	806	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	2	0	0	2	0		
Configuration		T	TR	LT	T			
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)	3		1					
Peak-Hour Factor, PHF	0.95	1.00	0.95	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	3	0	1	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)	-3			0				
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	0	0	0	0		
Configuration		LR						
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration		LT		LR				
v (veh/h)		1		4				
C (m) (veh/h)		761		188				
v/c		0.00		0.02				
95% queue length		0.00		0.07				
Control Delay (s/veh)		9.7		24.6				
LOS		A		C				
Approach Delay (s/veh)	--	--	24.6					
Approach LOS	--	--	C					

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information					
Analyst	JAG		Intersection	US 6 & 202/ Center access				
Agency/Co.	TMA		Jurisdiction	Town of Southeast				
Date Performed	10/18/07		Analysis Year	Build Condition				
Analysis Time Period	Saturday Peak Hour							
Project Description <i>Stateline</i>								
East/West Street: <i>US Route 6 &amp; 202</i>			North/South Street: <i>Center Access right in/out</i>					
Intersection Orientation: <i>East-West</i>			Study Period (hrs): <i>0.25</i>					
Vehicle Volumes and Adjustments								
Major Street	Eastbound			Westbound				
Movement	1	2	3	4	5	6		
	L	T	R	L	T	R		
Volume (veh/h)		387	370		719			
Peak-Hour Factor, PHF	1.00	0.84	0.84	1.00	0.89	1.00		
Hourly Flow Rate, HFR (veh/h)	0	460	440	0	807	0		
Percent Heavy Vehicles	0	--	--	0	--	--		
Median Type	<i>Undivided</i>							
RT Channelized			0			0		
Lanes	0	2	0	0	2	0		
Configuration		T	TR		T			
Upstream Signal		0			0			
Minor Street	Northbound			Southbound				
Movement	7	8	9	10	11	12		
	L	T	R	L	T	R		
Volume (veh/h)			46					
Peak-Hour Factor, PHF	1.00	1.00	0.95	1.00	1.00	1.00		
Hourly Flow Rate, HFR (veh/h)	0	0	48	0	0	0		
Percent Heavy Vehicles	0	0	0	0	0	0		
Percent Grade (%)		-3			0			
Flared Approach		N			N			
Storage		0			0			
RT Channelized			0			0		
Lanes	0	0	1	0	0	0		
Configuration			R					
Delay, Queue Length, and Level of Service								
Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration					R			
v (veh/h)					48			
C (m) (veh/h)					562			
v/c					0.09			
95% queue length					0.28			
Control Delay (s/veh)					12.0			
LOS					B			
Approach Delay (s/veh)	--	--	12.0					
Approach LOS	--	--	B					

<b>HCS+™ DETAILED REPORT</b>												
<b>General Information</b>						<b>Site Information</b>						
Analyst JAG						Intersection US Rt 6 / NYS Rt 121						
Agency or Co. TMA						Area Type All other areas						
Date Performed 10/31/2007						Jurisdiction Town of Southeast						
Time Period A.M. Peak Hour						Analysis Year Improved without diversion						
						Project ID Stateline						
<b>Volume and Timing Input</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>l</sub>		2	1	0	2		1		1			
Lane Group		T	R		LT		L		R			
Volume, V (vph)		309	283	250	994		327		34			
% Heavy Vehicles, %HV		5	5	5	5		4		4			
Peak-Hour Factor, PHF		0.69	0.69	0.92	0.92		0.86		0.86			
Pretimed (P) or Actuated (A)		P	P	A	A		A		A			
Start-up Lost Time, I <sub>l</sub>		2.0	2.0		2.0		2.0		2.0			
Extension of Effective Green, e		2.0	2.0		2.0		2.0		2.0			
Arrival Type, AT		3	3		3		3		3			
Unit Extension, UE		3.0	3.0		3.0		3.0		3.0			
Filtering/Metering, I		1.000	1.000		1.000		1.000		1.000			
Initial Unmet Demand, Q <sub>b</sub>		0.0	0.0		0.0		0.0		0.0			
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0			
Lane Width		12.0	12.0		12.0		14.0		14.0			
Parking / Grade / Parking	N	0	N	N	0	N	N	-4	N			
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0	0		0		0		0			
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2			3.2				
Phasing	EW Perm	WB Only	03		04		NB Only	06		07		08
Timing	G = 25.0	G = 10.0	G =		G =		G = 15.0	G =		G =		G =
	Y = 5	Y = 5	Y =		Y =		Y = 5	Y =		Y =		Y =
Duration of Analysis, T = 0.25							Cycle Length, C = 65.0					
<b>Lane Group Capacity, Control Delay, and LOS Determination</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		448	410		1352		380		40			
Lane Group Capacity, c		1325	1065		1772		436		780			
v/c Ratio, X		0.34	0.38		0.76		0.87		0.05			
Total Green Ratio, g/C		0.38	0.69		0.62		0.23		0.46			
Uniform Delay, d <sub>1</sub>		14.1	4.2		9.1		24.1		9.7			
Progression Factor, PF		1.000	1.000		1.000		1.000		1.000			
Delay Calibration, k		0.50	0.11		0.32		0.40		0.11			
Incremental Delay, d <sub>2</sub>		0.7	0.2		2.0		17.3		0.0			
Initial Queue Delay, d <sub>3</sub>		0.0	0.0		0.0		0.0		0.0			
Control Delay		14.8	4.4		11.1		41.3		9.7			
Lane Group LOS		B	A		B		D		A			
Approach Delay	9.9			11.1			38.3					
Approach LOS	A			B			D					
Intersection Delay	15.0			X <sub>c</sub> = 0.96			Intersection LOS			B		

<b>HCS+™ DETAILED REPORT</b>												
<b>General Information</b>						<b>Site Information</b>						
Analyst JAG						Intersection US Rt 6 / NYS Rt 121						
Agency or Co. TMA						Area Type All other areas						
Date Performed 10/31/2007						Jurisdiction Town of Southeast						
Time Period P.M. Peak Hour						Analysis Year Improved without diversion						
						Project ID Stateline						
<b>Volume and Timing Input</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>1</sub>		2	1	0	2		1		1			
Lane Group		T	R		LT		L		R			
Volume, V (vph)		753	197	113	504		256		118			
% Heavy Vehicles, %HV		4	4	3	3		2		2			
Peak-Hour Factor, PHF		0.89	0.89	0.97	0.97		0.95		0.95			
Pretimed (P) or Actuated (A)		P	P	A	A		A		A			
Start-up Lost Time, I <sub>1</sub>		2.0	2.0		2.0		2.0		2.0			
Extension of Effective Green, e		2.0	2.0		2.0		2.0		2.0			
Arrival Type, AT		3	3		3		3		3			
Unit Extension, UE		3.0	3.0		3.0		3.0		3.0			
Filtering/Metering, I		1.000	1.000		1.000		1.000		1.000			
Initial Unmet Demand, Q <sub>b</sub>		0.0	0.0		0.0		0.0		0.0			
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0			
Lane Width		12.0	12.0		12.0		14.0		14.0			
Parking / Grade / Parking	N	0	N	N	0	N	N	-4	N			
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0	0		0		0		0			
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2			3.2				
Phasing	EW Perm	WB Only	03		04		NB Only	06		07		08
Timing	G = 25.0	G = 10.0	G =		G =		G = 15.0	G =		G =		G =
	Y = 5	Y = 5	Y =		Y =		Y = 5	Y =		Y =		Y =
Duration of Analysis, T = 0.25							Cycle Length, C = 65.0					
<b>Lane Group Capacity, Control Delay, and LOS Determination</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		846	221		636		269		124			
Lane Group Capacity, c		1338	1075		1615		444		795			
v/c Ratio, X		0.63	0.21		0.39		0.61		0.16			
Total Green Ratio, g/C		0.38	0.69		0.62		0.23		0.46			
Uniform Delay, d <sub>1</sub>		16.3	3.6		6.3		22.4		10.2			
Progression Factor, PF		1.000	1.000		1.000		1.000		1.000			
Delay Calibration, k		0.50	0.11		0.11		0.19		0.11			
Incremental Delay, d <sub>2</sub>		2.3	0.1		0.2		2.4		0.1			
Initial Queue Delay, d <sub>3</sub>		0.0	0.0		0.0		0.0		0.0			
Control Delay		18.5	3.7		6.5		24.7		10.2			
Lane Group LOS		B	A		A		C		B			
Approach Delay	15.5			6.5			20.2					
Approach LOS	B			A			C					
Intersection Delay	13.6			X <sub>c</sub> = 0.74			Intersection LOS			B		

**HCS+™ DETAILED REPORT**

General Information		Site Information	
Analyst	JAG	Intersection	US Rt 6 / NYS Rt 121
Agency or Co.	TMA	Area Type	All other areas
Date Performed	10/31/2007	Jurisdiction	Town of Southeast
Time Period	Saturday Peak Hour	Analysis Year	Build Condition
		Project ID	Stateline

**Volume and Timing Input**

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>l</sub>		2	1	0	2		1		1			
Lane Group		T	R		LT		L		R			
Volume, V (vph)		672	194	127	596		179		104			
% Heavy Vehicles, %HV		1	1	1	1		2		2			
Peak-Hour Factor, PHF		0.84	0.84	0.90	0.90		0.94		0.94			
Pretimed (P) or Actuated (A)		P	P	A	A		A		A			
Start-up Lost Time, I <sub>1</sub>		2.0	2.0		2.0		2.0		2.0			
Extension of Effective Green, e		2.0	2.0		2.0		2.0		2.0			
Arrival Type, AT		3	3		3		3		3			
Unit Extension, UE		3.0	3.0		3.0		3.0		3.0			
Filtering/Metering, I		1.000	1.000		1.000		1.000		1.000			
Initial Unmet Demand, Q <sub>b</sub>		0.0	0.0		0.0		0.0		0.0			
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0			
Lane Width		12.0	12.0		12.0		14.0		14.0			
Parking / Grade / Parking	N	0	N	N	0	N	N	-4	N			
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0	0		0		0		0			
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2			3.2				
Phasing	EW Perm	WB Only	03		04		NB Only	06		07		08
Timing	G = 25.0	G = 10.0	G =		G =		G = 15.0	G =		G =		G =
	Y = 5	Y = 5	Y =		Y =		Y = 5	Y =		Y =		Y =
Duration of Analysis, T = 0.25							Cycle Length, C = 65.0					

**Lane Group Capacity, Control Delay, and LOS Determination**

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		800	231		803		190		111			
Lane Group Capacity, c		1378	1107		1653		444		795			
v/c Ratio, X		0.58	0.21		0.49		0.43		0.14			
Total Green Ratio, g/C		0.38	0.69		0.62		0.23		0.46			
Uniform Delay, d <sub>1</sub>		15.8	3.6		6.9		21.3		10.1			
Progression Factor, PF		1.000	1.000		1.000		1.000		1.000			
Delay Calibration, k		0.50	0.11		0.11		0.11		0.11			
Incremental Delay, d <sub>2</sub>		1.8	0.1		0.2		0.7		0.1			
Initial Queue Delay, d <sub>3</sub>		0.0	0.0		0.0		0.0		0.0			
Control Delay		17.6	3.7		7.1		22.0		10.2			
Lane Group LOS		B	A		A		C		B			
Approach Delay	14.5			7.1			17.6					
Approach LOS	B			A			B					
Intersection Delay	12.2			X <sub>c</sub> = 0.82			Intersection LOS			B		

<b>HCS+™ DETAILED REPORT</b>												
<b>General Information</b>						<b>Site Information</b>						
Analyst JAG						Intersection US Rt 6 / NYS Rt 121						
Agency or Co. TMA						Area Type All other areas						
Date Performed 10/31/2007						Jurisdiction Town of Southeast						
Time Period A.M. Peak Hour						Analysis Year Improved Sensitivity						
						Project ID Stateline						
<b>Volume and Timing Input</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes, N <sub>l</sub>		2	1	0	2		1		1			
Lane Group		T	R		LT		L		R			
Volume, V (vph)		309	283	250	694		627		34			
% Heavy Vehicles, %HV		5	5	5	5		4		4			
Peak-Hour Factor, PHF		0.69	0.69	0.92	0.92		0.86		0.86			
Pretimed (P) or Actuated (A)		P	P	A	A		A		A			
Start-up Lost Time, I <sub>l</sub>		2.0	2.0		2.0		2.0		2.0			
Extension of Effective Green, e		2.0	2.0		2.0		2.0		2.0			
Arrival Type, AT		3	3		3		3		3			
Unit Extension, UE		3.0	3.0		3.0		3.0		3.0			
Filtering/Metering, I		1.000	1.000		1.000		1.000		1.000			
Initial Unmet Demand, Q <sub>b</sub>		0.0	0.0		0.0		0.0		0.0			
Ped / Bike / RTOR Volumes	0	0	0	0	0		0	0	0			
Lane Width		12.0	12.0		12.0		14.0		14.0			
Parking / Grade / Parking	N	0	N	N	0	N	N	-4	N			
Parking Maneuvers, N <sub>m</sub>												
Buses Stopping, N <sub>b</sub>		0	0		0		0		0			
Min. Time for Pedestrians, G <sub>p</sub>		3.2			3.2			3.2				
Phasing	EW Perm	WB Only	03		04		NB Only	06		07		08
Timing	G = 10.0	G = 10.0	G =		G =		G = 25.0	G =		G =		G =
	Y = 5	Y = 5	Y =		Y =		Y = 5	Y =		Y =		Y =
Duration of Analysis, T = 0.25							Cycle Length, C = 60.0					
<b>Lane Group Capacity, Control Delay, and LOS Determination</b>												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Adjusted Flow Rate, v		448	410		1026		729		40			
Lane Group Capacity, c		574	1025		1234		787		1127			
v/c Ratio, X		0.78	0.40		0.83		0.93		0.04			
Total Green Ratio, g/C		0.17	0.67		0.42		0.42		0.67			
Uniform Delay, d <sub>1</sub>		23.9	4.5		15.6		16.6		3.4			
Progression Factor, PF		1.000	1.000		1.000		1.000		1.000			
Delay Calibration, k		0.50	0.11		0.37		0.44		0.11			
Incremental Delay, d <sub>2</sub>		10.1	0.3		5.0		16.9		0.0			
Initial Queue Delay, d <sub>3</sub>		0.0	0.0		0.0		0.0		0.0			
Control Delay		34.1	4.8		20.6		33.5		3.4			
Lane Group LOS		C	A		C		C		A			
Approach Delay	20.1			20.6			31.9					
Approach LOS	C			C			C					
Intersection Delay	23.7			X <sub>c</sub> = 0.96			Intersection LOS			C		