



November 1, 2010

Proposed Conservation Subdivision 23-Lot Residential Subdivision

Harris Road Town of Bedford Westchester County, New York

STORMWATER POLLUTION PREVENTION PLAN

Prepared for:

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I Rudolph C. Petruccelli, PE certifies that this Stormwater Pollution Prevention Plan has been prepared in accordance with NYSDEC rules and regulations and in accordance with the Town of Bedford Code Section 103.

November 1, 2010



TRIPI CONSERVATION SUBDIVISION

TOWN OF BEDFORD

WESTCHESTER COUNTY, NEW YORK

Stormwater Pollution Prevention Plan (SWPPP)

TABLE OF CONTENTS

1	GENER/	<u>AL</u>
	A.	Introduction
	В.	Existing Site Description
	a.	C. Project Description
		i. I - Soils
		ii. II - Grading and Drainage
	C.	Storm Water Management Methodology
		iii. I - Standards
		iv. II - Results
2	Pre-De	velopment and Post-Development Peak Flow Summary
3	CONST	RUCTION AND MAINTENANCE DESCRIPTION
	A.	Erosion and Sediment Control Plan
		I. Temporary Control Measures
		v. II. Permanent Control Measures
		vi. III. Pollution Prevention Measures and Materials Storage/Disposal
	B.	Narrative Report
	C.	Construction Sequencing
	D.	Construction Inspection Stages
	E.	Storm Water Management Facilities Maintenance Program
	F.	Conclusions
4	<u>APPENI</u>	DIX A - Pollutant Loading Analysis
5	<u>APPENI</u>	DIX B - Soils Information, Fema map
6	<u>APPENI</u>	DIX C - Stormwater Management Report
	A.	Pre-Development Drainage Divide Map and Routings
	B.	Post-Development Drainage Divide Map and Routings
7	APPENI	DIX D - Operator and Contractor Certifications
8	APPENI	DIX E - Maintenance and Inspection Reports and Construction Checklist

1. GENERAL

A. Introduction

This Stormwater Pollution Prevention Plan has been prepared pursuant to the Phase II regulations under General Permit (GP) 0-08-001 as required by the New York State Department of Environmental Conservation (NYSDEC).

The proposal is for (23) twenty three lot conservation subdivision on a 25.59 acre site in the Town of Bedford, Westchester County, New York. The (21) Twenty one clustered lots will be accessed by a proposed loop road which will be extended From New Street. The remaining two (2) individual lots will conform to existing zoning requirements and will have frontage to Harris Road.

Site construction for this project consists of a thirty (30) foot wide loop road which will be with curbs and paved, and have an emergency access road from Harris Road which will be without curbs. The first 200 feet of the emergency access road paved and the remaining section connects to the proposed loop road will be graded and kept as a dirt road.

All residences will be serviced with individual driveways and town water. The clustered lots will be serviced by a community effluent septic system, with each house having a septic tank, pump tank and valve tank. The individual lots will be serviced by individual sewage disposal systems on their property.

The stormwater runoff from the new impervious surface created by the proposed clustered lots will be directed to new stormwater management basins and subsurface exfiltration chambers which have been designed in accordance with the New York State Stormwater (NYSDEC) Design Manual and the New York City Department of Environmental Conservation (NYCDEP) regulations.

B. Existing Site Description

The site is located off of Harris Road and New Street in the Town of Bedford, Westchester County, New York and lies within the Muscoot Watershed which is East of Hudson (EOH). There is an existing residence on the property with access from Harris Road. In addition, there is a through dirt driveway which connects New Street to Harris Road. The principal structure is serviced by town water and septic system. The remainder of the site is mainly wooded with some areas of lawn near the existing residence. Also, the site consists of approximately 6.3 acres of slopes in excess of 25%.

The site runoff is directed to (8) eight drainage areas which discharge to (7) seven distinct discharge points, this can be seen on the pre-development drainage map. These points have been selected based on the topography available on the survey. Based on the USGS maps the receiving water is the Stone Hill River and the

stormwater interactive map provided on the NYSDEC website illustrates that the site is in a TMDL watershed but does not contain and is not adjacent to any 303d listed streams.

The stormwater runoff from the site is conveyed via overland flow to the discharge points illustrated on the Pre-Development Map.

C. Project Description

The proposal is for (23) twenty three lot conservation subdivision of which (21) twenty one are conservation lots and (2) two individual lots meeting existing zoning on a 25.59 acre site in the Town of Bedford, Westchester County, New York. Site construction for this project consists of (1) one loop roadway which is approximately 2,230 feet in length and connected to New Street, and an approximately 925 foot emergency access roadway which begins at Harris Road and ends at sta. 8+50 of the loop roadway. There are (21) twenty one new residences in the conservation development, one new individual house on a new conforming lot and one existing residence. The conservation lots will be serviced by a community effluent septic system and the other two lots by individual septic systems. All lots will be serviced by town water. Stormwater management facilities will be constructed to satisfy NYSDEC and NYSDEP requirements. The total land disturbance associated with this construction is approximately 13.06 acres.

I - Soils

The soils in the project area is mapped as CsD (Charlton-Chatfield) and CrC (Chatfield-Charlton Complex), hilly and very rocky. These soils are classified as 'B' soil". These soils are considered to have moderate runoff potential and are moderately drained.

II - Grading and Drainage

The grading and drainage plan has been designed to capture and treat the stormwater runoff from the new and existing impervious surfaces and disturbed areas at each improvement location. Stormwater will be collected through catch basins and underground pipes and discharged into stormwater basins, subsurface exfiltration chambers and/or detention ponds that have been designed in accordance with the NYS Stormwater Management Design Manual, NYCDEP criteria, and the Phosphorus Removal Section of the SWM. Basin designs have been included in this application and are designated as NYSDEC I-2 Infiltration Basins and the exfiltration chamber are designed as NYSDEC I-4 basins.

The ponds have been designed for Water Quality and Flood Control, mitigating the post-

development rate of stormwater runoff to the pre-development rate.

Storm water runoff rates for the 1 (NYSDEC Stream Channel Protection and water quality volume), 2 year storm event, 10 (NYSDEC Overbank Protection), and 100-year (NYSDEC Extreme Flood) storm events have been analyzed and routed in the pre-development and post-development condition utilizing computer software "Hydrocad" The design assumptions are provided in the appendix of this report.

It is expected that the project will take approximately 48 months to construct from the time of ground breaking to final completion, with work commencing shortly after the receipt of all necessary approvals. Throughout the construction process strict adherence to the Erosion Control Plans and Specifications will be maintained to ensure all sediment is contained within the improvement areas.

Storm water management design also provides for water quality treatment such that the project areas will not represent a negative impact or degradation in water quality to any reservoir, stream, wetlands or watercourses.

D. Storm Water Management Methodology

Storm water quality and quantity has been analyzed in accordance with the guidelines set forth in the New York State General Permit for Storm Water Discharge, GP 0-08-001. The water quality volume was computed from the runoff produced by the 1 year 24 hour event. Water Quality volume and invert elevations of the low flow orifices have been calculated for each of the drainage areas and are included in the Appendix. The low flow orifices in each basin have been sized to release the water quality volume over a 24 hour period. Additional control devices are included in the outlet structures to control the rate of runoff from the 1, 2, 10, and 100 year 24 hour storm events to the pre-construction runoff rate. Velocity dissipators are specified at each inlet and outlet and shall be a riprap pad per "New York Guidelines for Urban Erosion & Sediment Control".

The proposed ponds are designed as Infiltration basins (NYSDEC I-2) and include hydrodynamic pretreatment separators at each basin that can accommodate the 1 year storm event.

The Pre and Post-Construction Drainage Divide Maps are included in the Appendix of this report. The Times of Concentration, coverage types, and hydrograph/stormwater calculations for the pre and post construction conditions are provided in the stormwater routings and are also provided in the Appendix.

Storm water quality and quantity computations are based upon the following publications.

- Soil Conservation Service (SCS) TR-20
- Urban Hydrology for Small Watersheds TR-55
- NYSDEC 'New York State Stormwater Management Design Manual', latest edition

- Controlling Urban Runoff: A practical Manual for Planning and Designing Urban BMP'S, by the Metropolitan Washington Council of Governments.
- Computer software Hydrocad has been utilized for the stormwater analysis. This program is on USDA Soil Conservation Service (SCS) Technical Release 55 (TR 55)

Compliance with Better Site Design (BSD):

Chapter 10 "Enhance Phosphorus Removal Standards" section 10.3.4 of the NYS Stromwater Manual indicates (4) four goals to meet treatment performance based on a BSD. The goals and project compliance are as follows:

Goal 1: Reduce Runoff Volumes:

The project has been designed to reduce the total amount of impervious surfaces and to promote overland flow through vegetated areas. The subdivision layout, unlike standard subdivisions of today incorporates some tuck-under garages and minimal driveway back up areas adjacent to the garages. Additionally the driveways are proposed to have curbing so that run off from these impervious surfaces is conveyed via pipe to stormwater basins.

Goal 2: Achieving Effluent Concentrations for Particulate Phosphorus:

By implementing the stormwater practices listed in section 10.4 of the NYS stormwater manual this goal is achieved. Additionally hydrodynamic separators that are equipped with an internal bypass have been incorporated into the design as pre-treatment devices. These chambers are allowed under the NYSDEC regulations and have been sized based on the 1 year storm event.

Goal 3: Achieving Effluent Concentrations for Dissolved Phosphorus:

By implementing those stormwater practices listed in section 10.4 of the NYS stormwater manual this goal has been achieved. Considering that infiltration basins and hydrodynamic separators have been utilized for the proposed drainage areas, the concentration dissolved phosphorous will be reduced by allowing the stormwater to come in contact with the substrate soils

Goal 4: Achieving Effluent Concentrations for Dissolved Phosphorus:

By implement those stormwater practices listed in section 10.4 of the NYS stormwater manual this goal is achieved. Considering that gravel trenches and infiltration basins have been utilized for a majority of the proposed drainage areas the concentration of dissolved phosphorous will be reduced by allowing the

stormwater to come in contact with the substrate soils.

I - STANDARDS

- Peak flow rates have been controlled to ensure that the post-development rate of runoff from the site will not exceed pre-development rates for 1, 10, and 100 year 24 hour storm events.
- Pollutant loading is controlled by means of detaining the runoff generated from the water quality storm event for longer than 24 hours.
- The proposed structures will be constructed above the 100-year floodplain.

II - Summary of Results

The results of the hydrographs from the contributing watersheds were compared under pre and post development conditions. The analysis consists of (5) five design points each noted on the pre and post development drainage maps. The results are summarized in the following table:

2. Pre-Development & Post-Development Peak Flow Summary

Table 1. Summary of Flows DP-1

Design Storm (yr)	Pre-Development Peak runoff (cfs)	Post-Development Peak runoff (cfs)	Post-Development % Reduction w/ Mitigation
1	1.44	1.20	16.67%
2	3.94	3.27	17.01%
10	12.15	10.00	17.70%
100	30.24	24.82	17.92%

Table 2. Summary of Flows DP-2

Design Storm (yr)	Pre-Development Peak runoff (cfs)	Post-Development Peak runoff (cfs)	Post-Development % Reduction w/ Mitigation		
1	0.76	0.21	72.37%		
2	1.89	0.50	73.54%		
10	5.35	1.34	74.95%		
100	12.69	3.10	75.57%		

Table 3. Summary of Flows DP-3

Design Storm (yr)	Pre-Development Peak runoff (cfs)	Post-Development Peak runoff (cfs)	Post-Development % Reduction w/ Mitigation		
1	0.66	0.41	37.88%		
2	1.38	0.94	31.88%		
10	3.36	2.52	25.00%		
100	7.30	5.81	20.41%		

Table 4. Summary of Flows DP-4

Design Storm (yr)	Pre-Development Peak runoff (cfs)	Post-Development Peak runoff (cfs)	Post-Development % Reduction w/ Mitigation
1	0.46	0.10	78.26%
2	1.25	0.40	68.00%
10	3.94	1.58	59.90%
100	9.81	4.41	55.05%

Table 5. Summary of Flows DP-5

Design Storm (yr)	Pre-Development Peak runoff (cfs)	Post-Development Peak runoff (cfs)	Post-Development % Reduction w/ Mitigation
1	1.32	0.68	48.48%
2	2.36	1.45	38.56%
10	4.96	3.53	28.83%
100	9.90	7.69	22.32%

Table 6. Summary of Flows DP-6

Design Storm (yr)	Pre-Development Peak runoff (cfs)	Post-Development Peak runoff (cfs)	Post-Development % Reduction w/ Mitigation			
1	0.68	0.51	25.00%			
2	1.45	1.35	6.90%			
10	3.52	3.18	9.66%			
100	7.66	7.58	1.04%			

Table 7. Summary of Flows DP-7

Design Storm (yr)	Pre-Development Peak runoff (cfs)	Post-Development Peak runoff (cfs)	Post-Development % Reduction w/ Mitigation
1	1.06	0.66	37.74%
2	2.16	1.57	27.31%
10	5.10	4.11	19.41%
100	10.87	9.33	14.17%

Table 8. Summary of Flows for Entire Site

Design Storm (yr)	Pre-Development Peak runoff (cfs)	Post-Development Peak runoff (cfs)	Post-Development % Reduction w/ Mitigation
1	6.38	3.77	40.91%
2	14.43	9.48	34.30%
10	38.38	26.26	31.58%
100	88.47	62.74	29.08%

3. CONSTRUCTION AND MAINTENANCE DESCRIPTION

A. Erosion and Sediment Control Plan

I. Temporary Structural Measures:

The temporary soil erosion and sediment control devices include protective earthmoving procedures and grading practices, vegetated cover, hay bales, silt fencing, stabilized construction entrance, dust control, construction road stabilization, silt traps, inlet protections and sediment basins. The methodology of the plan is to control erosion & sedimentation, and to re-establish vegetation as soon as possible. These temporary controls will be installed prior to commencement of earthmoving activities where possible.

All proposed erosion and sediment controls and details as well as the stormwater management facilities are shown on various plans prepared by Petruccelli Engineering. All proposed soil erosion and sediment control practices are designed in accordance with the following publications:

- New York State Guidelines for Urban Erosion and Sediment Control, latest edition
- New York State General Permit for Stormwater Discharges, GPO-08-001 (General Permit)
- "Reducing the Impacts of Stormwater Runoff from New Development", as published by the New York State Department of Environmental Conservation (NYSDEC), second edition, April 1993.

II. Permanent Structural Measures:

The temporary sediment traps will be installed to prevent sediments from entering the infiltration basins. Once the disturbance areas have been stabilized these sediment traps will be removed. In addition, rock outlet protections will be installed at the inflow of stormwater facilities. All other temporary devices such as silt fencing, hay bales and diversions will be removed during the course of construction.

III. Pollution Prevention Measures and Materials Storage/Disposal:

The construction materials and vehicles expected to be present during construction include but are not limited to drainage pipe, pre-cast concrete drainage structures, earth moving equipment, concrete trucks, asphalt trucks, pavement marking machinery, and worker vehicles.

All construction related debris will be collected and removed from the area on a regular basis.

Concrete wash out areas will be provided where necessary and existing and or excess asphalt material will be removed from the site and disposed of in the proper manner.

Sediment spoils will be disposed in an approved off-site location along with temporary erosion control devices.

B. Narrative Report

The primary goal of the soil erosion and sediment control measures is to reduce soil erosion from areas stripped of vegetation during and after construction, and to prevent discharge of silt offsite. Erosion control barriers shall be placed around exposed areas during construction. The barriers shall consist of staked haybales or silt fence. Temporary sediment basins or traps will be used at stormwater collection points to allow sediment removal prior to releasing the stormwater offsite.

Any areas stripped of vegetation during construction will be left bare for the shortest time possible. Any topsoil removed during construction will be temporarily stockpiled for future use in grading and landscaping. A stockpile location has been provided on the Erosion Control Plan and shall be contained within a silt fence barrier.

Temporary vegetation will be established to protect exposed soil areas during construction. If growing conditions are not suitable for the temporary vegetation, mulch will be used. Materials that may be used for mulching include; straw, hay, salt hay, wood fiber, synthetic soil stabilizers, mulch netting, and sod. A permanent vegetative cover will be established upon completion of construction of those areas that have been brought to finish grade and to remain undisturbed.

A temporary stabilized construction entrance comprised of three inches clean stone will be constructed at the entrances to the site. The purpose of a stabilized entrance is to remove soil from the construction vehicle tires prior to exiting the site and traveling on the existing roadways. During construction, inlet protection will be installed at each storm sewer inlet to minimize the conveyance of silt and sediment through the storm sewer system.

C. Construction Sequencing

The following is a detailed Construction Sequencing:

- 1. Install stabilized construction entrance at the site access point.
- 2. Install silt fence and barrier fence and tree protection as shown.
- 3. Install inlet protection as required.
- 4. Minimize clearing within the limits of disturbance as required for construction.
- 5. Install soil stockpiling protection.
- 6. Install temporary sediment basins at the locations of micro-pool detention to intercept and detain the sediment during construction. At the time of construction convert ponds into permanent stormwater detention ponds.
- 7. Excavate for buildings, roads and utilities and stockpile topsoil.
- 8. Perform temporary stabilization over all disturbed soil areas.
- Remove temporary soil erosion and sediment control measures such as vegetative measures

D. Construction Inspection Stages

The contractor is directed to the town of Bedford code, section 103-10 a(1)(a) which specifies the staging of construction inspections for erosion and sediment controls. The contractor shall notify the town of Bedford enforcement official at least 48 hours before any of the following items are required by stormwater management officer:

- 1. Start of construction
- 2. Installation of sediment and erosion control measures
- 3. Completion of site clearing
- 4. Completion of rough grading
- 5. Completion of final grading
- 6. Close of the construction season
- 7. Completion of final landscaping
- 8. Successful establishment of landscaping in public areas

E. Storm Water Management Facilities Maintenance Program

The following maintenance program is proposed in order to maintain the proper function of all drainage and erosion and sediment control facilities:

 Mow the side slopes and bottom of the basin as necessary to maintain their appearance but not less than twice a year. Inspect basin and if necessary remove invasive woody vegetation to prevent it from becoming established within the basin.

- During mowing operations, litter and debris will be removed from vegetated swale, extended detention basin and the outlet control structures.
- During the construction of the project, the site erosion and sediment control measures as well as
 basin embankments and outlet structure will be inspected by the project superintendent once a
 week and/or immediately following a rainstorm. Any repairs required will be performed in a timely
 manner. All sediment removal and/or repairs will be followed immediately by re-vegetation.
- All disturbed areas will be stabilized and the sediment build up (50% of sediment basin) in the
 basins removed before the basin is fined graded and landscaped. After the construction is
 completed, any areas disturbed will be stabilized immediately after the required work is
 completed.
- The Owner will inspect the facilities once a month, and once a year by a Professional Engineer. A report by the Professional Engineer will be submitted to the Owner in the event deficiencies are found. In addition, the Owner will inspect the system after each major storm event to ensure the small orifices and inlets remain open. Specific attention will be paid to the following:
 - Evidence of clogging of outlet control device.
 - Erosion of the flow path through the detention facility.
 - Subsidence, erosion, cracking or tree growth on the embankments.
 - Accumulation of sediment.
- Clean catch basins and other drainage structures from silt regularly, but not less than twice a year. Remove sediment build up in the basin as required, but a minimum of every five years. A rubber-tired backhoe with a minimum reach of 25' will be used to remove silt accumulation. Laborers with shovel and wheelbarrows will be used to maintain the embankment slopes, to repair minor erosion problems and remove minor accumulation of silt. The use of hand labor will also minimize the disturbance of stabilized areas and the established vegetation. A rubber-tired backhoe has the reach and maneuverability to maintain these basins from the adjacent areas.
- Tree growth on the downstream face of the detention ponds will also be monitored regularly.
 Trees that develop shall be removed during routine maintenance of the basin.
- The owner will take all necessary measures to have seeps, leeks, and/or settlements on the
 embankment of detention pond repaired. If seeps, leaks, and/or settlement are discovered the
 owner will be obligated to contract a licensed engineer to assess the problem and offer solutions
 to repair.
- Restore and re-seed any eroded areas and gullies as soon as possible.

• The Stormwater Management facilities Maintenance Program will be managed by:

E. Conclusions

The incorporation of the Best Management Practices will significantly reduce the pollutant loadings in the post-construction condition by capturing and treating the runoff from the new and existing impervious surfaces and disturbed areas to the greatest extent possible. This plan meets the requirements of the NYSDEC & NYCDEP for Water Quality and Quantity, providing minimal impact to downstream waters.



<u>APPENDIX A</u>

- POLLUTANT LOADING ANALYSIS



Tripi Conservation Subdivision Harris Road Town of Bedford Westchester County, NY

POLLUTANT LOADING ANALYSIS

	PRE-DEVELOPMENT CONDITIONS AND QUALITY IMPACTS										
SUB-	LANDLICE	AREA	HYDROLOGIC	ANNUAL LOADING RATES (LB/AC/YR				ANNUAL LOADS (LB/YR)			
AREA	LAND USE	(ACRES)	SOIL GROUP	BOD	TSS	TP	TN	BOD	TSS	TP	TN
1	Meadow	14.001	В	7	214	0.32	3.85	98.0	2989.2	4.4	53.9
I	Developed	0.547	В	53.9	523	1.68	9.1	29.5	286.0	0.9	5.0
2	Meadow	4.697	В	7	214	0.32	3.85	32.9	1002.8	1.5	18.1
	Developed	0.554	В	53.9	523	1.68	9.1	29.9	289.7	0.9	5.0
3	Meadow	2.366	В	7	214	0.32	3.85	16.6	505.1	0.7	9.1
3	Developed	0.052	В	53.9	523	1.68	9.1	2.8	27.2	0.1	0.5
4	Meadow	4.018	В	7	214	0.32	3.85	28.1	857.8	1.3	15.5
4	Developed	0.253	В	53.9	523	1.68	9.1	13.6	132.3	0.4	2.3
5	Meadow	2.561	В	7	214	0.32	3.85	17.9	546.8	0.8	9.9
	Developed	0.575	В	53.9	523	1.68	9.1	31.0	300.7	1.0	5.2
6	Meadow	2.273	В	7	214	0.32	3.85	15.9	485.3	0.7	8.8
U	Developed	0.045	В	53.9	523	1.68	9.1	2.4	23.5	0.1	0.4
7	Meadow	2.890	В	7	214	0.32	3.85	20.2	617.0	0.9	11.1
	Developed	0.235	В	53.9	523	1.68	9.1	12.7	122.9	0.4	2.1
		35.1		TOT	ALS	351.5	8186.4	14.1	146.9		

Tripi Conservation Subdivision Harris Road Town of Bedford Westchester County, NY

	POST-DEVELOPMENT CONDITIONS AND QUALITY IMPACTS										
SUB-	LANDUGE	AREA	HYDROLOGIC		ANNUAL LOADING RATES			ANNUAL LOADS (LB/YR)			
AREA	LAND USE	(ACRES)	SOIL GROUP	BOD	TSS	TP	TN	BOD	TSS	TP	TN
	Meadow	9.824	В	7	214	0.32	3.85	68.8	2097.4	3.1	37.8
1	Grass	2.770	В	7	214	0.32	3.85	19.4	591.4	0.9	10.7
	Developed	0.698	В	53.9	523	1.68	9.1	37.6	365.0	1.2	6.4
	Grass	3.319	В	7	214	0.32	3.85	23.2	708.6	1.0	12.8
2A	Meadow	0.520	В	7	214	0.32	3.85	3.6	111.0	0.2	2.0
	Developed	1.912	В	53.9	523	1.68	9.1	103.1	999.8	3.2	17.4
2B	Developed	0.459	В	53.9	523	1.68	9.1	24.7	240.0	0.8	4.2
	Meadow	1.297	В	7	214	0.32	3.85	9.1	276.9	0.4	5.0
3	Grass	1.144	В	7	214	0.32	3.85	8.0	244.2	0.4	4.4
	Developed	0.212	В	53.9	523	1.68	9.1	11.4	110.9	0.4	1.9
	Meadow	0.730	В	7	214	0.32	3.85	5.1	155.9	0.2	2.8
4	Grass	0.451	В	7	214	0.32	3.85	3.2	96.3	0.1	1.7
	Developed	0.093	В	53.9	523	1.68	9.1	5.0	48.6	0.2	0.8
	Meadow	2.327	В	7	214	0.32	3.85	16.3	496.8	0.7	9.0
5	Grass	0.545	В	7	214	0.32	3.85	3.8	116.4	0.2	2.1
	Developed	0.016	В	53.9	523	1.68	9.1	0.9	8.4	0.0	0.1
	Meadow	0.893	В	7	214	0.32	3.85	6.3	190.7	0.3	3.4
6A	Developed	1.109	В	53.9	523	1.68	9.1	59.8	579.9	1.9	10.1
	Grass	0.935	В	7	214	0.32	3.85	6.5	199.6	0.3	3.6
6B	Developed	0.231	В	53.9	523	1.68	9.1	12.5	120.8	0.4	2.1
6C	Meadow	0.303	В	7	214	0.32	3.85	2.1	64.7	0.1	1.2
	Meadow	1.340	В	7	214	0.32	3.85	9.4	286.1	0.4	5.2
7	Grass	1.375	В	7	214	0.32	3.85	9.6	293.6	0.4	5.3
	Developed	0.291	В	53.9	523	1.68	9.1	15.7	152.2	0.5	2.6
	Meadow	0.954	В	7	214	0.32	3.85	6.7	203.7	0.3	3.7
8	Grass	0.977	В	7	214	0.32	3.85	6.8	208.6	0.3	3.8
	Developed	0.362	В	53.9	523	1.68	9.1	19.5	189.3	0.6	3.3
			TOT	ALS	498.1	9156.6	18.4	163.3			

Tripi Conservation Subdivision Harris Road Town of Bedford Westchester County, NY

	TREATED LOADS											
SUB-	BEST MANAGEMENT PRACTICE	RI	REMOVAL RATE (%)				ANNUAL REMOVED LOADS (LB/YR) W/ TREATMENT					
AREA		BOD	TSS	TP	TN	BOD	TSS	TP	TN			
1	NONE	0	0	0	0	0.0	0.0	0.0	0.0			
2A	INFILTRATION BASIN	90	90	70	70	116.9	1,637.5	3.1	22.5			
2B	DRY DETENTION POND	60	87	40	35	14.8	208.8	0.3	1.5			
3	NONE	0	0	0	0	0.0	0.0	0.0	0.0			
4	NONE	0	0	0	0	0.0	0.0	0.0	0.0			
5	NONE	0	0	0	0	0.0	0.0	0.0	0.0			
6A	DRY DETENTION POND	60	87	40	35	43.5	844.1	1.0	6.0			
6B	INFILTRATION BASIN	90	90	70	70	11.2	108.7	0.3	1.5			
6C	NONE	0	0	0	0	0.0	0.0	0.0	0.0			
7	NONE	0	0	0	0	0.0	0.0	0.0	0.0			
8	NONE	0	0	0	0	0.0	0.0	0.0	0.0			
		•		TOT	ALS	186.5	2799.0	4.7	31.5			

SUMMARY TABLE	BOD (LB/YR)	TSS (LB/YR)	TP (LB/YR)	TN (LB/YR)
PREDEVELOPMENT	351.5	8186.4	14.1	146.9
POST DEVELOPMENT WITHOUT TREATMENT	498.1	9156.6	18.4	163.3
REMOVED LOADS (LB/YR) W/ TREATMENT	186.5	2799.0	4.7	31.5
POST DEVELOPMENT WITH TREATMENT	311.5	6357.5	13.7	131.9
% CHANGE OF PRE-DEVELOPMENT RATES WITH TREATMENT	11.4%	22.3%	2.7%	10.2%

Note: Each area infiltration basin is proposed with a hydrodynamic seperator which will remove BOD, TSS, TP, &TN rates beyond those shown above.



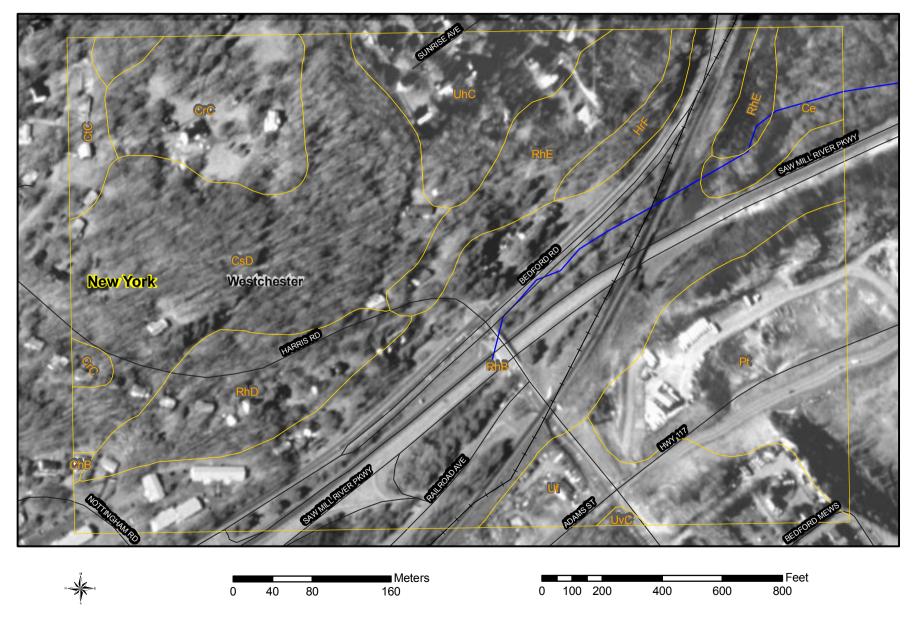
<u>APPENDIX B</u>

- SOILS INFORMATION
- FEMA MAP



SOIL SURVEY OF WESTCHESTER COUNTY, NEW YORK

Tripi Soils Map





SOIL SURVEY OF WESTCHESTER COUNTY, NEW YORK

Tripi Soils Map

MAP INFORMATION MAP LEGEND Soil Map Units Source of Map: Natural Resources Conservation Service Cities Web Soil Survey URL: http://websoilsurvey.nrcs.usda.gov **Detailed Counties Detailed States** Coordinate System: UTM Zone 18 Interstate Highways Roads Soil Survey Area: Westchester County, New York Rails Spatial Version of Data: 1 Water Soil Map Compilation Scale: 1:12000 Hydrography Oceans AYAYAY Escarpment, bedrock vvvvvv Escarpment, non-bedrock Gulley IIIIIIIIIII Levee Slope Blowout \odot \boxtimes Borrow Pit Clay Spot Depression, closed **Eroded Spot** Gravel Pit Gravelly Spot Gulley Λ Lava Flow Map comprised of aerial images photographed on these dates: Landfill 4/12/1991 Marsh or Swamp Miscellaneous Water Rock Outcrop Saline Spot Sandy Spot Slide or Slip Sinkhole Sodic Spot The orthophoto or other base map on which the soil lines were compiled and Spoil Area digitized probably differs from the background imagery displayed on these maps. Û Stony Spot As a result, some minor shifting of map unit boundaries may be evident. Very Stony Spot



•

Perennial Water Wet Spot

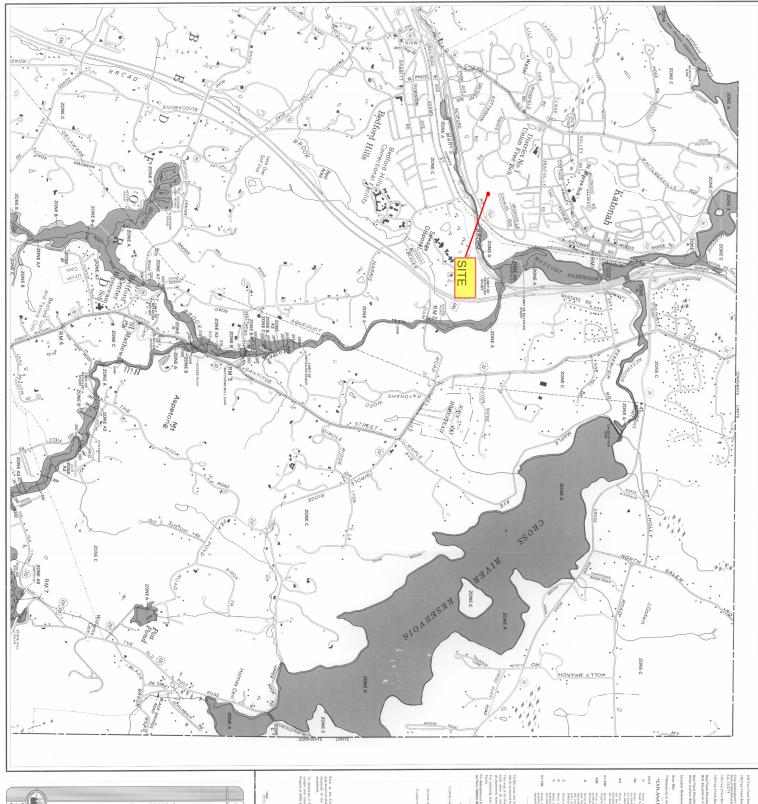
Map Unit Legend Summary

Westchester County, New York

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
Ce	Carlisle muck	2.7	2.7
ChB	Charlton loam, 2 to 8 percent slopes	0.1	0.1
CrC	Charlton-Chatfield complex, rolling, very rocky	6.9	7.1
CsD	Chatfield-Charlton complex, hilly, very rocky	18.9	19.4
CtC	Chatfield-Hollis-Rock outcrop complex, rolling	1.6	1.6
HrF	Hollis-Rock outcrop complex, very steep	1.5	1.5
Pt	Pits, gravel	11.6	11.8
RhB	Riverhead loam, 3 to 8 percent slopes	29.6	30.3
RhD	Riverhead loam, 15 to 25 percent slopes	6.3	6.5
RhE	Riverhead loam, 25 to 50 percent slopes	6.7	6.8
Uf	Urban land	6.0	6.1
UhC	Urban land-Charlton complex, 8 to 15 percent slopes	5.8	6.0
UvC	Urban land-Riverhead complex, 8 to 15 percent slopes	0.1	0.1







COMMUNITY-PANEL NUMBER
360903 0C10 C
EFFECTIVE DATE:
DECEMBER 4, 1979

PANEL 10 OF 20

TOWN OF
BEDFORD,
NEW YORK
WESTCHESTER COUNTY

FIRM
FLOOD INSURANCE RATE MAP

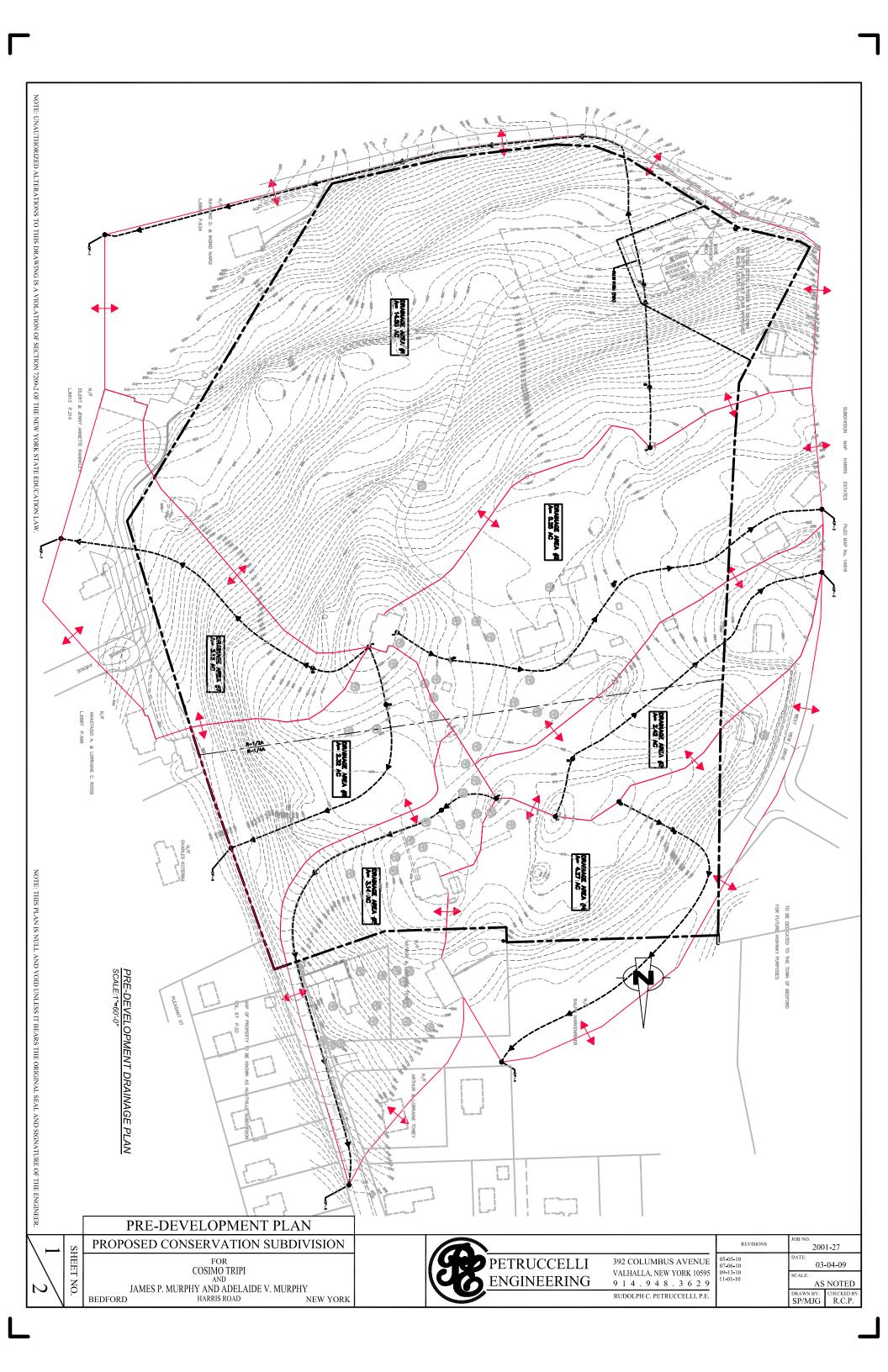
NATIONAL FLOOD INSURANCE PROGRAM



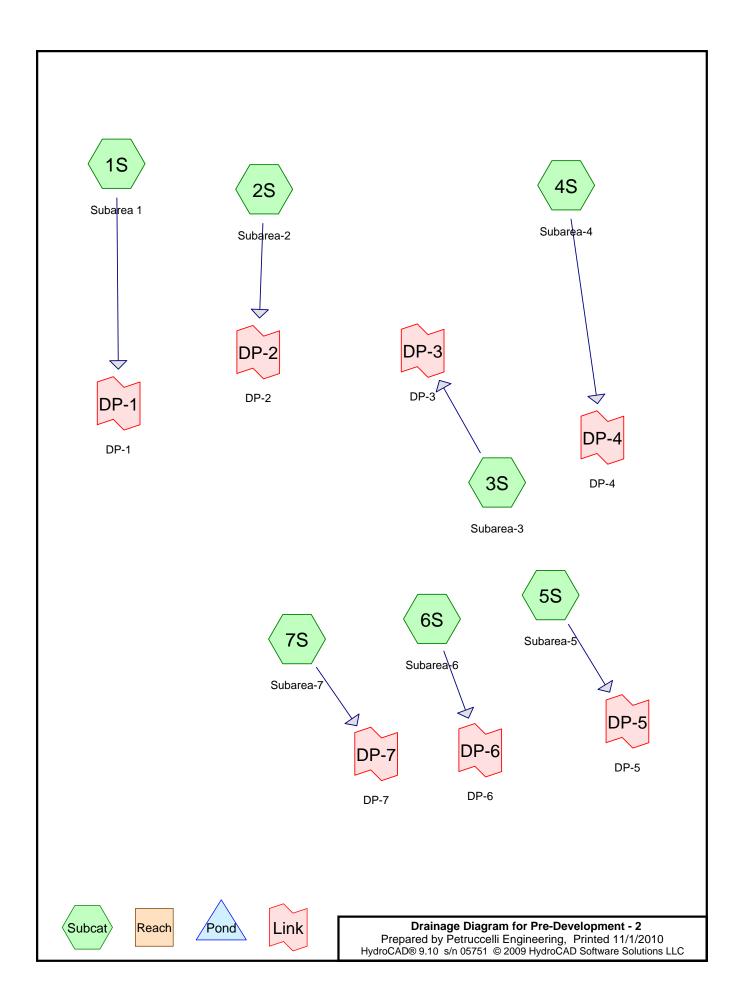
<u>APPENDIX C</u>

- PRE-DEVELOPMENT DRAINAGE DIVIDE MAP AND ROUTINGS
- POST-DEVELOPMENT DRAINAGE DIVIDE MAP AND ROUTINGS











Pre-Development - 2
Prepared by Petruccelli Engineering
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Area Listing (all nodes)

	Area	CN	Description
	(acres)		(subcatchment-numbers)
22.716 58		58	Woods/grass comb., Good, HSG B (1S, 2S, 4S)
	10.090	65	Woods/grass comb., Fair, HSG B (3S, 5S, 6S, 7S)
	2.261	98	Paved roads w/curbs & sewers (1S, 2S, 3S, 4S, 5S, 6S, 7S)
	35.066		TOTAL AREA

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Page 3

Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
32.806	HSG B	1S, 2S, 3S, 4S, 5S, 6S, 7S
0.000	HSG C	
0.000	HSG D	
2.261	Other	1S, 2S, 3S, 4S, 5S, 6S, 7S
35.066		TOTAL AREA

Time span=0.00-250.00 hrs, dt=0.01 hrs, 25001 points Runoff by SCS TR-20 method, UH=SCS Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment 1S: Subarea 1	Runoff Area=14.548 ac 3.76% Impervious Runoff Depth=0.26" Flow Length=1,389' Tc=25.0 min CN=60 Runoff=1.44 cfs 0.321 af
Subcatchment 2S: Subarea-2	Runoff Area=228,729 sf 10.55% Impervious Runoff Depth=0.32" Flow Length=821' Tc=21.0 min CN=62 Runoff=0.76 cfs 0.141 af
Subcatchment3S: Subarea-3	Runoff Area=105,316 sf 2.16% Impervious Runoff Depth=0.45" Flow Length=634' Tc=17.0 min CN=66 Runoff=0.66 cfs 0.091 af
Subcatchment 4S: Subarea-4	Runoff Area=186,025 sf 5.92% Impervious Runoff Depth=0.26" Flow Length=680' Tc=19.6 min CN=60 Runoff=0.46 cfs 0.094 af
Subcatchment 5S: Subarea-5	Runoff Area=136,628 sf 18.34% Impervious Runoff Depth=0.65" Flow Length=877' Tc=21.9 min CN=71 Runoff=1.32 cfs 0.169 af
Subcatchment 6S: Subarea-6	Runoff Area=100,951 sf 1.94% Impervious Runoff Depth=0.45" Flow Length=504' Tc=13.5 min CN=66 Runoff=0.68 cfs 0.087 af
Subcatchment7S: Subarea-7	Runoff Area=136,133 sf 7.51% Impervious Runoff Depth=0.49" Flow Length=594' Tc=12.8 min CN=67 Runoff=1.06 cfs 0.127 af
Link DP-1: DP-1	Inflow=1.44 cfs 0.321 af Primary=1.44 cfs 0.321 af
Link DP-2: DP-2	Inflow=0.76 cfs 0.141 af Primary=0.76 cfs 0.141 af
Link DP-3: DP-3	Inflow=0.66 cfs 0.091 af Primary=0.66 cfs 0.091 af
Link DP-4: DP-4	Inflow=0.46 cfs 0.094 af Primary=0.46 cfs 0.094 af
Link DP-5: DP-5	Inflow=1.32 cfs 0.169 af Primary=1.32 cfs 0.169 af
Link DP-6: DP-6	Inflow=0.68 cfs 0.087 af Primary=0.68 cfs 0.087 af
Link DP-7: DP-7	Inflow=1.06 cfs 0.127 af Primary=1.06 cfs 0.127 af

Total Runoff Area = 35.066 ac Runoff Volume = 1.031 af Average Runoff Depth = 0.35" 93.55% Pervious = 32.806 ac 6.45% Impervious = 2.261 ac

Page 5

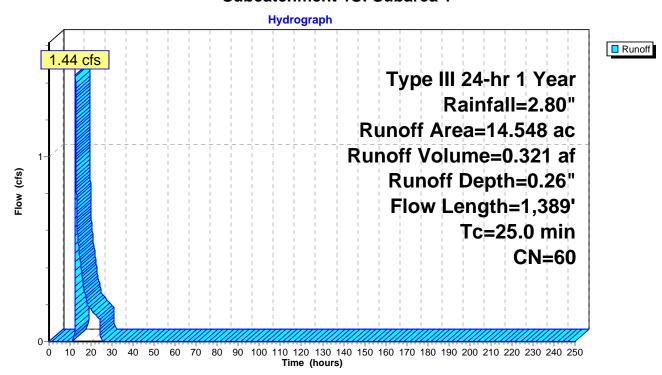
Summary for Subcatchment 1S: Subarea 1

Runoff = 1.44 cfs @ 12.58 hrs, Volume= 0.321 af, Depth= 0.26"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 1 Year Rainfall=2.80"

Area	(ac) C	N Desc	cription				
0.547 98 Paved roads w/curbs & sewers							
14.001 58 Woods/grass comb., Good, HSG B							
14.	548 6	0 Weig	ghted Aver	age			
14.	001	96.2	4% Pervio	us Area			
0.	547	3.76	% Impervi	ous Area			
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·		
10.8	100	0.1000	0.15		Sheet Flow, 1 to 2		
					Woods: Light underbrush n= 0.400 P2= 3.50"		
0.9	171	0.3600	3.00		Shallow Concentrated Flow, 2 to 3		
					Woodland Kv= 5.0 fps		
13.2	1,118	0.0800	1.41		Shallow Concentrated Flow, 3 to DP-1		
	,				Woodland Kv= 5.0 fps		
25.0	1,389	Total			<u> </u>		

Subcatchment 1S: Subarea 1



Page 6

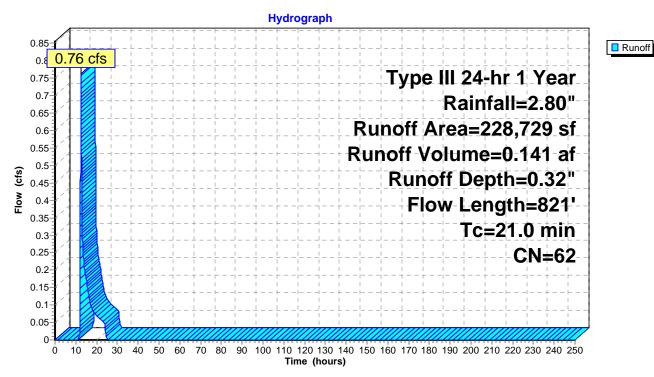
Summary for Subcatchment 2S: Subarea-2

Runoff = 0.76 cfs @ 12.48 hrs, Volume= 0.141 af, Depth= 0.32"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 1 Year Rainfall=2.80"

_	Α	rea (sf)	CN D	Description						
24,134 98 Paved roads w/curbs & sewers										
_	2	04,595	58 V	Voods/gras	ss comb., G	Good, HSG B				
228,729 62 Weighted Average										
	2	04,595	8	9.45% Per	vious Area					
		24,134	1	0.55% Imp	pervious Ar	ea				
	Tc	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	9.4	100	0.1400	0.18		Sheet Flow, 1 to 2				
						Woods: Light underbrush n= 0.400 P2= 3.50"				
	11.6	721	0.0430	1.04		Shallow Concentrated Flow, 2 to 3				
						Woodland Kv= 5.0 fps				
	21.0	821	Total	-						

Subcatchment 2S: Subarea-2



Page 7

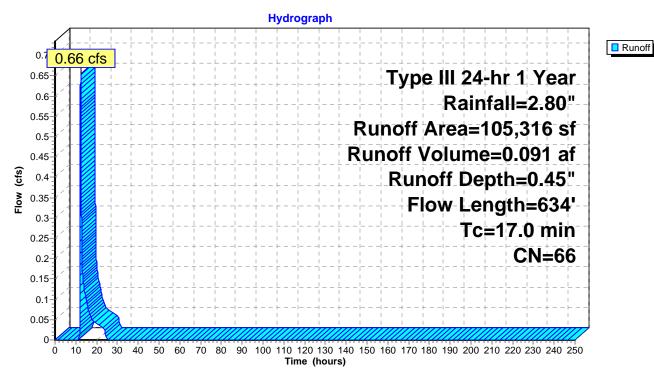
Summary for Subcatchment 3S: Subarea-3

Runoff = 0.66 cfs @ 12.30 hrs, Volume= 0.091 af, Depth= 0.45"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 1 Year Rainfall=2.80"

	Α	rea (sf)	CN D	N Description						
		2,270	98 P	aved road	s w/curbs &	& sewers				
_	1	03,046	65 V	Voods/gras	ss comb., F	air, HSG B				
105,316 66 Weighted Average										
	1	03,046	9	7.84% Per	vious Area					
		2,270	2	.16% Impe	ervious Area	a				
	Tc	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	9.9	100	0.1250	0.17		Sheet Flow, 1 to 2				
						Woods: Light underbrush n= 0.400 P2= 3.50"				
	7.1	534	0.0620	1.24		Shallow Concentrated Flow, 2 to DP-3				
						Woodland Kv= 5.0 fps				
	17.0	634	Total							

Subcatchment 3S: Subarea-3



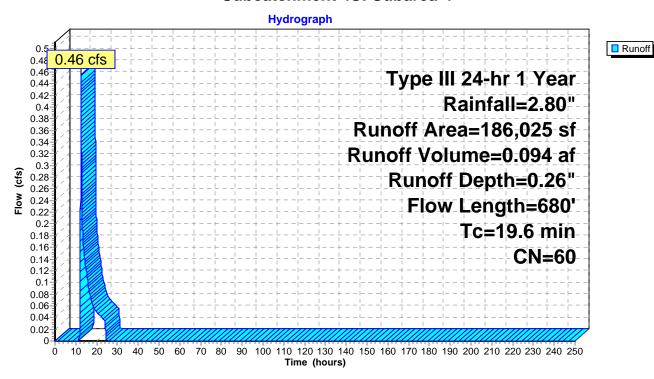
Summary for Subcatchment 4S: Subarea-4

Runoff = 0.46 cfs @ 12.50 hrs, Volume= 0.094 af, Depth= 0.26"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 1 Year Rainfall=2.80"

_	Α	rea (sf)	CN D	CN Description						
		11,012	98 F	& sewers						
_	1	75,013	58 V	Voods/gras	ss comb., G	Good, HSG B				
186,025 60 Weighted Average										
	1	75,013	9	4.08% Per	vious Area					
		11,012	5	.92% Impe	ervious Area	a				
	Тс	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	9.4	100	0.1400	0.18		Sheet Flow, 1 to 2				
						Woods: Light underbrush n= 0.400 P2= 3.50"				
	10.2	580	0.0360	0.95		Shallow Concentrated Flow, 2 to DP-4				
						Woodland Kv= 5.0 fps				
	19.6	680	Total							

Subcatchment 4S: Subarea-4



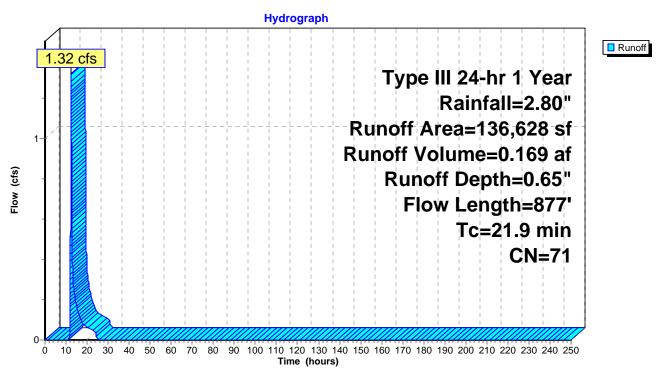
Summary for Subcatchment 5S: Subarea-5

Runoff = 1.32 cfs @ 12.35 hrs, Volume= 0.169 af, Depth= 0.65"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 1 Year Rainfall=2.80"

	Area (sf) CN Description							
		25,055		aved road				
_	1	11,573	65 V	Voods/gras	ss comb., F	air, HSG B		
136,628 71 Weighted Average								
	1	11,573			vious Area			
		25,055	1	8.34% Imp	ervious Ar	ea		
		•						
	Tc	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·		
	13.7	100	0.0550	0.12		Sheet Flow, 1 to 2		
						Woods: Light underbrush n= 0.400 P2= 3.50"		
	8.2	777	0.0990	1.57		Shallow Concentrated Flow, 2 to DP-5		
	_			_		Woodland Kv= 5.0 fps		
_	21.9	877	Total			•		

Subcatchment 5S: Subarea-5



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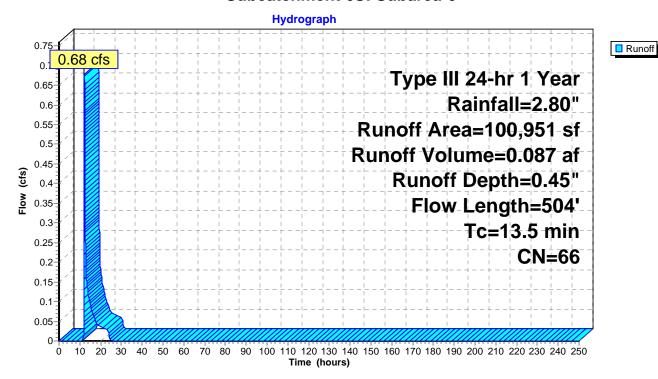
Runoff = 0.68 cfs @ 12.24 hrs, Volume= 0.087 af, Depth= 0.45"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 1 Year Rainfall=2.80"

	Α	rea (sf)	CN E	N Description						
		1,957			s w/curbs &					
_		98,994	65 V	Voods/gras	ss comb., F	air, HSG B				
100,951 66 Weighted Average										
		98,994	9	8.06% Per	vious Area					
		1,957	1	.94% Impe	ervious Area	a				
	Tc	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	9.7	100	0.1300	0.17		Sheet Flow, 1 to 2				
						Woods: Light underbrush n= 0.400 P2= 3.50"				
	3.8	404	0.1260	1.77		Shallow Concentrated Flow, 2 to DP-6				
						Woodland Kv= 5.0 fps				
	13.5	504	Total							

Summary for Subcatchment 6S: Subarea-6

Subcatchment 6S: Subarea-6



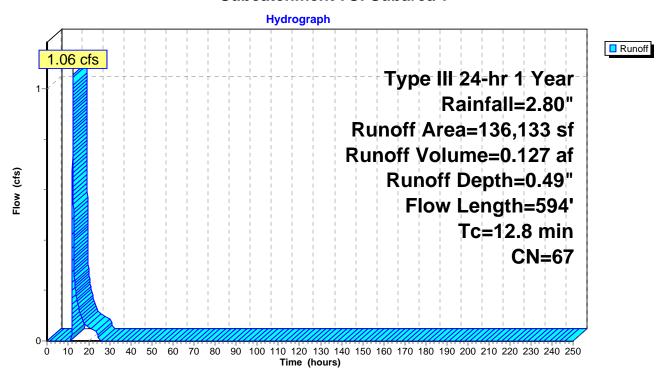
Summary for Subcatchment 7S: Subarea-7

Runoff = 1.06 cfs @ 12.22 hrs, Volume= 0.127 af, Depth= 0.49"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 1 Year Rainfall=2.80"

_	Α	rea (sf)	CN E	Description						
	10,223 98 Paved roads w/curbs & sewers									
_	1	25,910	65 V	Voods/gras	ss comb., F	air, HSG B				
136,133 67 Weighted Average										
	1	25,910	9	2.49% Per	vious Area					
		10,223	7	.51% lmpe	ervious Are	a				
	Tc	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	8.9	100	0.1600	0.19		Sheet Flow, 1 to 2				
						Woods: Light underbrush n= 0.400 P2= 3.50"				
	3.9	494	0.1780	2.11		Shallow Concentrated Flow, 2 to DP-7				
						Woodland Kv= 5.0 fps				
	12.8	594	Total							

Subcatchment 7S: Subarea-7



Summary for Link DP-1: DP-1

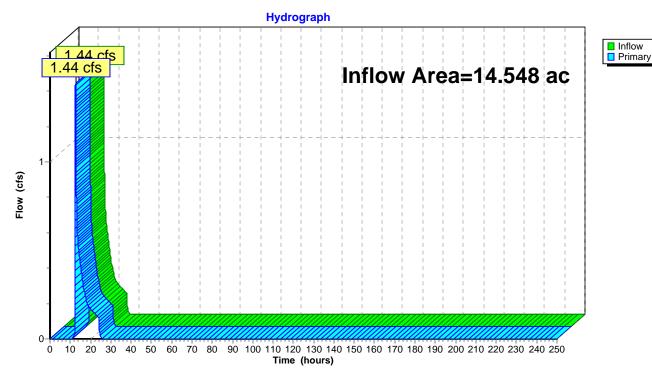
Inflow Area = 14.548 ac, 3.76% Impervious, Inflow Depth = 0.26" for 1 Year event

Inflow = 1.44 cfs @ 12.58 hrs, Volume= 0.321 af

Primary = 1.44 cfs @ 12.58 hrs, Volume= 0.321 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-1: DP-1



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Summary for Link DP-2: DP-2

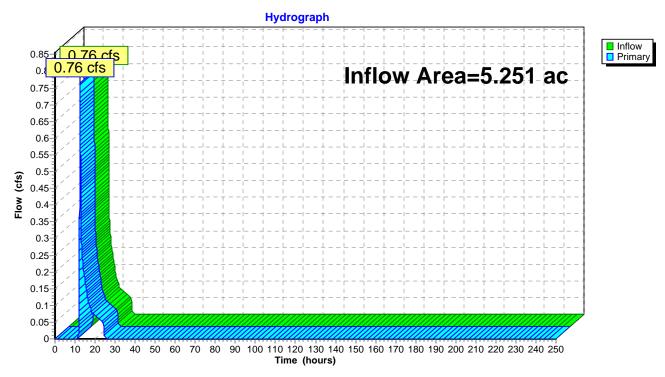
Inflow Area = 5.251 ac, 10.55% Impervious, Inflow Depth = 0.32" for 1 Year event

Inflow = 0.76 cfs @ 12.48 hrs, Volume= 0.141 af

Primary = 0.76 cfs @ 12.48 hrs, Volume= 0.141 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-2: DP-2



Summary for Link DP-3: DP-3

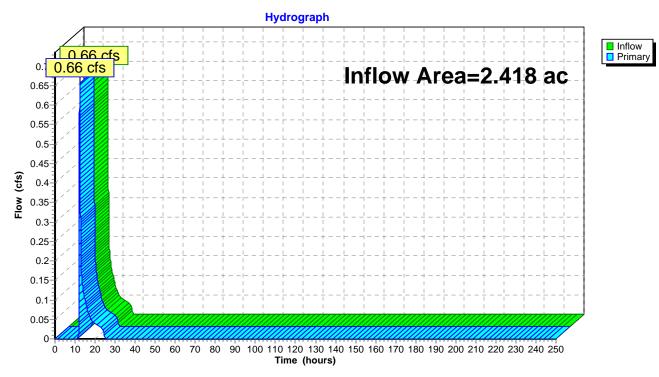
Inflow Area = 2.418 ac, 2.16% Impervious, Inflow Depth = 0.45" for 1 Year event

Inflow = 0.66 cfs @ 12.30 hrs, Volume= 0.091 af

Primary = 0.66 cfs @ 12.30 hrs, Volume= 0.091 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-3: DP-3



Summary for Link DP-4: DP-4

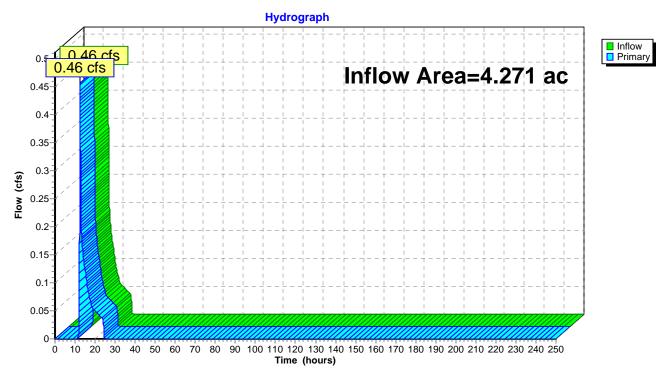
5.92% Impervious, Inflow Depth = 0.26" for 1 Year event Inflow Area = 4.271 ac,

Inflow 0.46 cfs @ 12.50 hrs, Volume= 0.094 af

0.46 cfs @ 12.50 hrs, Volume= 0.094 af, Atten= 0%, Lag= 0.0 min Primary

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-4: DP-4



Prepared by Petruccelli Engineering

Page 16

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Summary for Link DP-5: DP-5

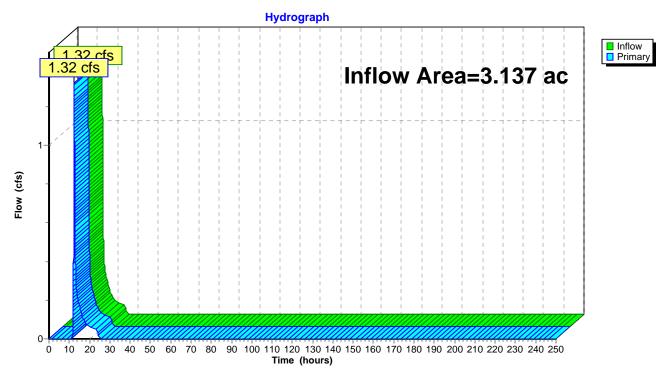
Inflow Area = 3.137 ac, 18.34% Impervious, Inflow Depth = 0.65" for 1 Year event

Inflow = 1.32 cfs @ 12.35 hrs, Volume= 0.169 af

Primary = 1.32 cfs @ 12.35 hrs, Volume= 0.169 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-5: DP-5



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Summary for Link DP-6: DP-6

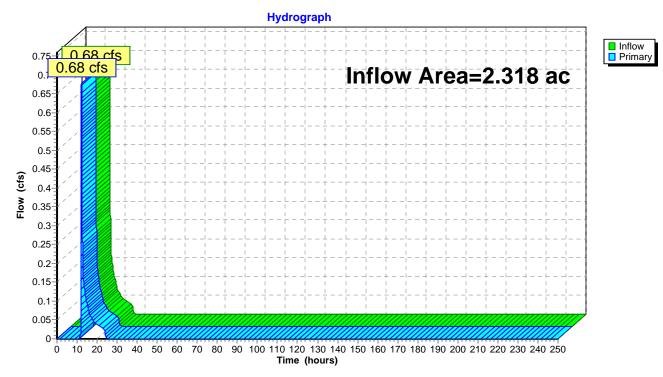
Inflow Area = 2.318 ac, 1.94% Impervious, Inflow Depth = 0.45" for 1 Year event

Inflow = 0.68 cfs @ 12.24 hrs, Volume= 0.087 af

Primary = 0.68 cfs @ 12.24 hrs, Volume= 0.087 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-6: DP-6



Summary for Link DP-7: DP-7

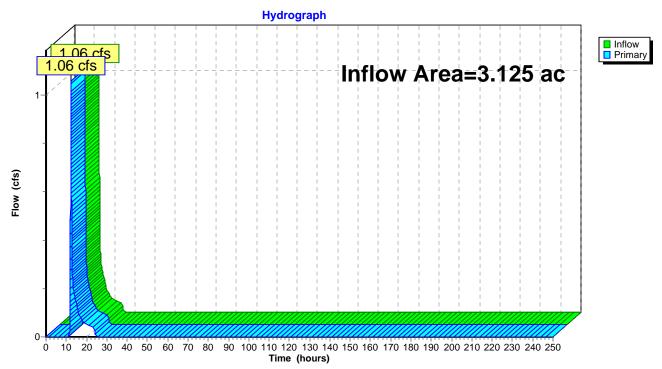
Inflow Area = 3.125 ac, 7.51% Impervious, Inflow Depth = 0.49" for 1 Year event

Inflow = 1.06 cfs @ 12.22 hrs, Volume= 0.127 af

Primary = 1.06 cfs @ 12.22 hrs, Volume= 0.127 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-7: DP-7



Time span=0.00-250.00 hrs, dt=0.01 hrs, 25001 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment 1S: Subarea 1	Runoff Area=14.548 ac 3.76% Impervious Runoff Depth=0.53" Flow Length=1,389' Tc=25.0 min CN=60 Runoff=3.94 cfs 0.644 af
Subcatchment 2S: Subarea-2	Runoff Area=228,729 sf 10.55% Impervious Runoff Depth=0.62" Flow Length=821' Tc=21.0 min CN=62 Runoff=1.89 cfs 0.269 af
Subcatchment3S: Subarea-3	Runoff Area=105,316 sf 2.16% Impervious Runoff Depth=0.80" Flow Length=634' Tc=17.0 min CN=66 Runoff=1.38 cfs 0.161 af
Subcatchment 4S: Subarea-4	Runoff Area=186,025 sf 5.92% Impervious Runoff Depth=0.53" Flow Length=680' Tc=19.6 min CN=60 Runoff=1.25 cfs 0.189 af
Subcatchment 5S: Subarea-5	Runoff Area=136,628 sf 18.34% Impervious Runoff Depth=1.06" Flow Length=877' Tc=21.9 min CN=71 Runoff=2.36 cfs 0.278 af
Subcatchment 6S: Subarea-6	Runoff Area=100,951 sf 1.94% Impervious Runoff Depth=0.80" Flow Length=504' Tc=13.5 min CN=66 Runoff=1.45 cfs 0.155 af
Subcatchment 7S: Subarea-7	Runoff Area=136,133 sf 7.51% Impervious Runoff Depth=0.85" Flow Length=594' Tc=12.8 min CN=67 Runoff=2.16 cfs 0.221 af
Link DP-1: DP-1	Inflow=3.94 cfs 0.644 af Primary=3.94 cfs 0.644 af
Link DP-2: DP-2	Inflow=1.89 cfs 0.269 af Primary=1.89 cfs 0.269 af
Link DP-3: DP-3	Inflow=1.38 cfs 0.161 af Primary=1.38 cfs 0.161 af
Link DP-4: DP-4	Inflow=1.25 cfs 0.189 af Primary=1.25 cfs 0.189 af
Link DP-5: DP-5	Inflow=2.36 cfs 0.278 af Primary=2.36 cfs 0.278 af
Link DP-6: DP-6	Inflow=1.45 cfs 0.155 af Primary=1.45 cfs 0.155 af
Link DP-7: DP-7	Inflow=2.16 cfs 0.221 af Primary=2.16 cfs 0.221 af

Page 20

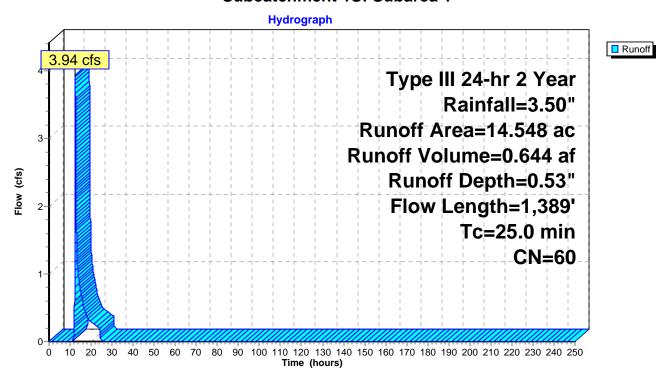
Summary for Subcatchment 1S: Subarea 1

Runoff = 3.94 cfs @ 12.47 hrs, Volume= 0.644 af, Depth= 0.53"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 2 Year Rainfall=3.50"

_	Area	(ac) C	N Desc	cription				
0.547 98 Paved roads w/curbs & sewers								
14.001 58 Woods/grass comb., Good, HSG B								
14.548 60 Weighted Average								
	14.	001	96.2	4% Pervio	us Area			
	0.	547	3.76	% Impervi	ous Area			
	Tc	Length	Slope	Velocity	Capacity	Description		
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
_	10.8	100	0.1000	0.15		Sheet Flow, 1 to 2		
						Woods: Light underbrush n= 0.400 P2= 3.50"		
	0.9	171	0.3600	3.00		Shallow Concentrated Flow, 2 to 3		
						Woodland Kv= 5.0 fps		
	13.2	1,118	0.0800	1.41		Shallow Concentrated Flow, 3 to DP-1		
		,				Woodland Kv= 5.0 fps		
-	25.0	1 389	Total			2.2.1.		

Subcatchment 1S: Subarea 1



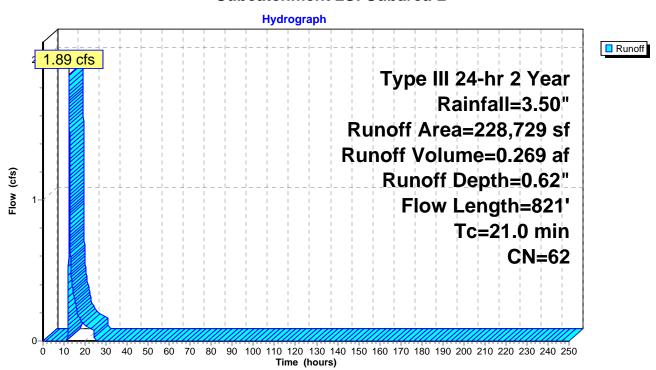
Summary for Subcatchment 2S: Subarea-2

Runoff = 1.89 cfs @ 12.37 hrs, Volume= 0.269 af, Depth= 0.62"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 2 Year Rainfall=3.50"

_	Area (sf) CN Description						
		24,134	98 P	aved road	s w/curbs &	& sewers	
_	2	04,595	58 V	Voods/gras	ss comb., G	Good, HSG B	
	2	28,729	62 V	Veighted A	verage		
	2	04,595	8	9.45% Per	vious Area		
		24,134	1	0.55% lmp	pervious Ar	ea	
	Tc	Length	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	9.4	100	0.1400	0.18		Sheet Flow, 1 to 2	
						Woods: Light underbrush n= 0.400 P2= 3.50"	
	11.6	721	0.0430	1.04		Shallow Concentrated Flow, 2 to 3	
						Woodland Kv= 5.0 fps	
_	21.0	821	Total			·	

Subcatchment 2S: Subarea-2



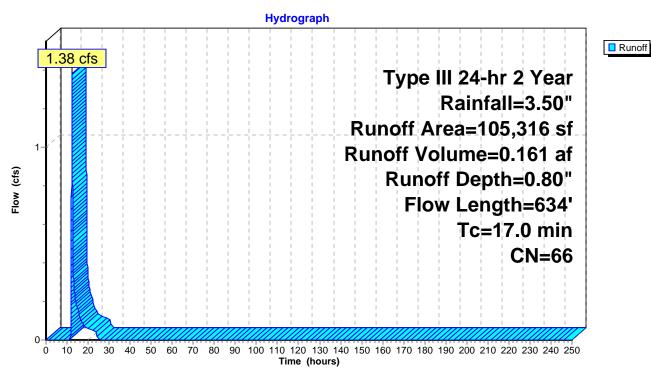
Summary for Subcatchment 3S: Subarea-3

Runoff = 1.38 cfs @ 12.27 hrs, Volume= 0.161 af, Depth= 0.80"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 2 Year Rainfall=3.50"

_	Area (sf) CN Description						
2,270 98 Paved roads w/curbs &							
	1	03,046	65 V	Voods/gras	ss comb., F	air, HSG B	
	1	05,316	66 V	Veighted A	verage		
	1	03,046	9	7.84% Per	vious Area		
		2,270	2	.16% Impe	ervious Are	a	
	Tc	Length	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	9.9	100	0.1250	0.17		Sheet Flow, 1 to 2	
						Woods: Light underbrush n= 0.400 P2= 3.50"	
	7.1	534	0.0620	1.24		Shallow Concentrated Flow, 2 to DP-3	
						Woodland Kv= 5.0 fps	
	17.0	634	Total			·	

Subcatchment 3S: Subarea-3



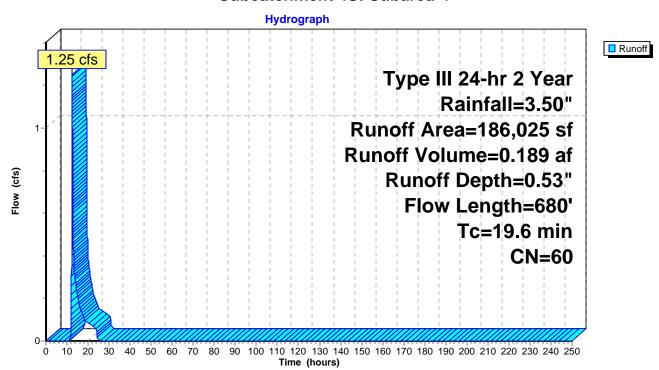
Summary for Subcatchment 4S: Subarea-4

1.25 cfs @ 12.37 hrs, Volume= Runoff 0.189 af, Depth= 0.53"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 2 Year Rainfall=3.50"

_	Area (sf) CN Description						
		11,012	98 F	aved road	s w/curbs &	& sewers	
	1	75,013	58 V	Voods/gras	ss comb., G	Good, HSG B	
	1	86,025	60 V	Veighted A	verage		
	1	75,013	9	4.08% Per	vious Area		
	11,012 5.92% Impervious Area					a	
	Тс	Length	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	9.4	100	0.1400	0.18		Sheet Flow, 1 to 2	
						Woods: Light underbrush n= 0.400 P2= 3.50"	
	10.2	580	0.0360	0.95		Shallow Concentrated Flow, 2 to DP-4	
_						Woodland Kv= 5.0 fps	
	19.6	680	Total	·			

Subcatchment 4S: Subarea-4



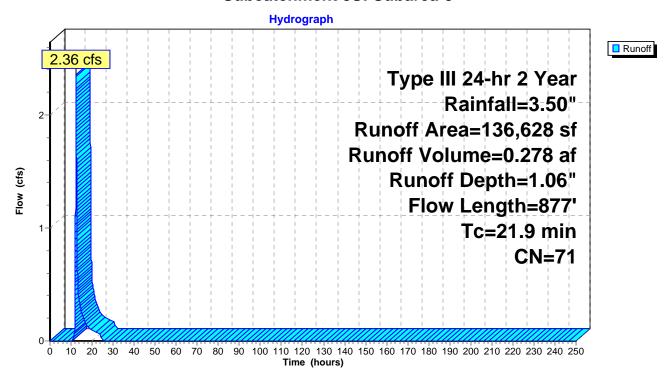
Summary for Subcatchment 5S: Subarea-5

Runoff = 2.36 cfs @ 12.34 hrs, Volume= 0.278 af, Depth= 1.06"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 2 Year Rainfall=3.50"

_	Area (sf) CN Description						
		25,055			s w/curbs &		
_	1	11,573	65 V	Voods/gras	ss comb., F	air, HSG B	
	1	36,628	71 V	Veighted A	verage		
	1	11,573	8	1.66% Per	vious Area		
		25,055	1	8.34% Imp	ervious Ar	ea	
	Тс	Length	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	13.7	100	0.0550	0.12		Sheet Flow, 1 to 2	
						Woods: Light underbrush n= 0.400 P2= 3.50"	
	8.2	777	0.0990	1.57		Shallow Concentrated Flow, 2 to DP-5	
						Woodland Kv= 5.0 fps	
	21.9	877	Total	-	-		

Subcatchment 5S: Subarea-5



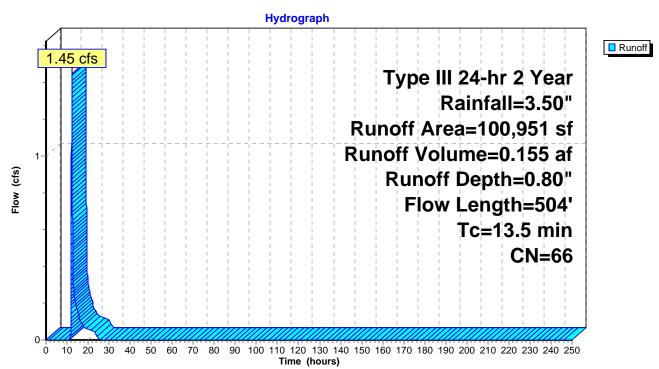
Summary for Subcatchment 6S: Subarea-6

Runoff = 1.45 cfs @ 12.21 hrs, Volume= 0.155 af, Depth= 0.80"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 2 Year Rainfall=3.50"

	Α	Area (sf) CN Description							
	1,957 98 Paved roads w/curbs & sewers								
_		98,994	65 V	Voods/gras	ss comb., F	air, HSG B			
	1	00,951	66 V	Veighted A	verage				
		98,994	9	8.06% Per	vious Area				
		1,957	1	.94% Impe	ervious Area	a			
	Тс	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	9.7	100	0.1300	0.17		Sheet Flow, 1 to 2			
						Woods: Light underbrush n= 0.400 P2= 3.50"			
	3.8	404	0.1260	1.77		Shallow Concentrated Flow, 2 to DP-6			
						Woodland Kv= 5.0 fps			
	13.5	504	Total						

Subcatchment 6S: Subarea-6



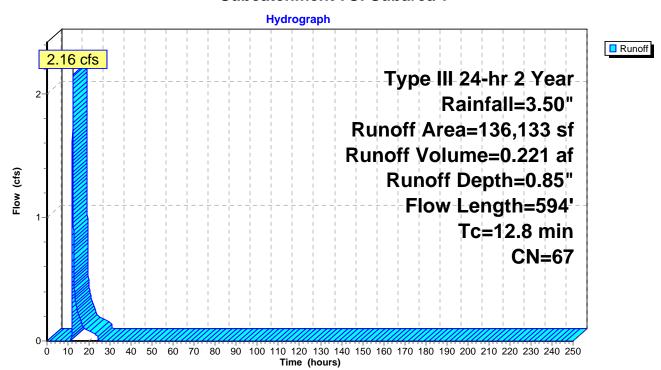
Summary for Subcatchment 7S: Subarea-7

Runoff = 2.16 cfs @ 12.20 hrs, Volume= 0.221 af, Depth= 0.85"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 2 Year Rainfall=3.50"

_	Α	Area (sf) CN Description						
		10,223			s w/curbs &			
_	1	25,910	65 V	Voods/gras	ss comb., F	air, HSG B		
	1	36,133	67 V	Veighted A	verage			
	1	25,910	9	2.49% Per	vious Area			
		10,223	7	".51% Impe	ervious Area	a		
	Tc	Length	Slope	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	8.9	100	0.1600	0.19		Sheet Flow, 1 to 2		
						Woods: Light underbrush n= 0.400 P2= 3.50"		
	3.9	494	0.1780	2.11		Shallow Concentrated Flow, 2 to DP-7		
						Woodland Kv= 5.0 fps		
	12.8	594	Total		-			

Subcatchment 7S: Subarea-7



Summary for Link DP-1: DP-1

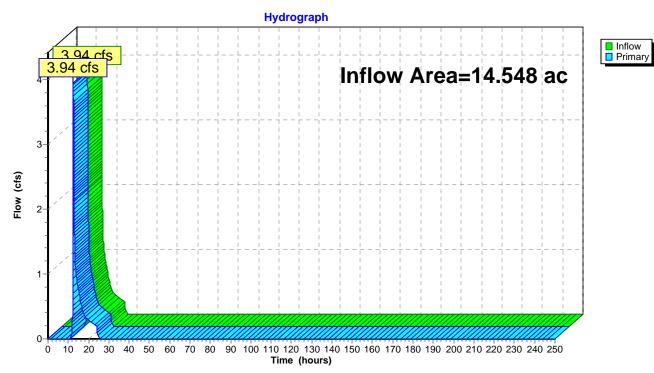
Inflow Area = 14.548 ac, 3.76% Impervious, Inflow Depth = 0.53" for 2 Year event

Inflow = 3.94 cfs @ 12.47 hrs, Volume= 0.644 af

Primary = 3.94 cfs @ 12.47 hrs, Volume= 0.644 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-1: DP-1



Summary for Link DP-2: DP-2

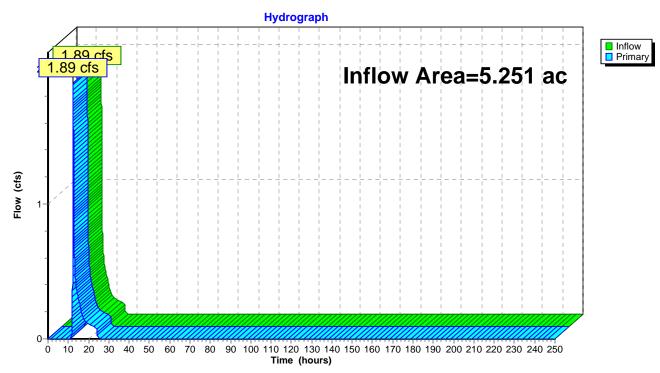
Inflow Area = 5.251 ac, 10.55% Impervious, Inflow Depth = 0.62" for 2 Year event

Inflow = 1.89 cfs @ 12.37 hrs, Volume= 0.269 af

Primary = 1.89 cfs @ 12.37 hrs, Volume= 0.269 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-2: DP-2



Page 29

Summary for Link DP-3: DP-3

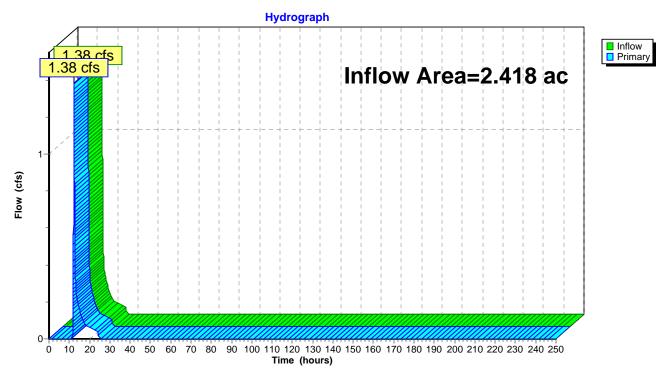
Inflow Area = 2.418 ac, 2.16% Impervious, Inflow Depth = 0.80" for 2 Year event

Inflow = 1.38 cfs @ 12.27 hrs, Volume= 0.161 af

Primary = 1.38 cfs @ 12.27 hrs, Volume= 0.161 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-3: DP-3



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Inflow Area = 4.271 ac, 5.92% Impervious, Inflow Depth = 0.53" for 2 Year event

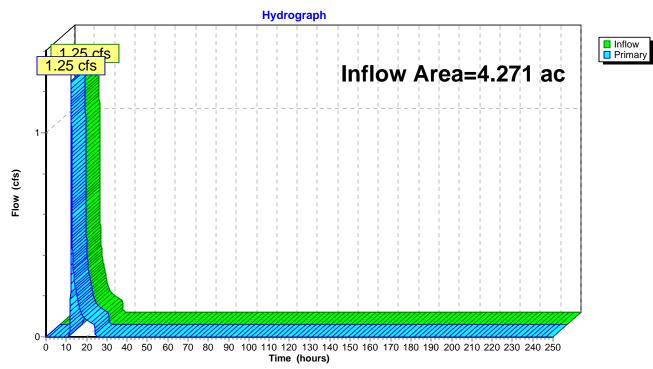
Inflow = 1.25 cfs @ 12.37 hrs, Volume= 0.189 af

Primary = 1.25 cfs @ 12.37 hrs, Volume= 0.189 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-4: DP-4

Summary for Link DP-4: DP-4



Summary for Link DP-5: DP-5

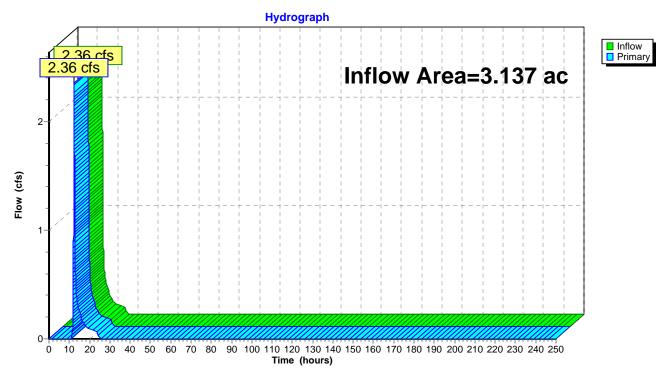
Inflow Area = 3.137 ac, 18.34% Impervious, Inflow Depth = 1.06" for 2 Year event

Inflow = 2.36 cfs @ 12.34 hrs, Volume= 0.278 af

Primary = 2.36 cfs @ 12.34 hrs, Volume= 0.278 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-5: DP-5



Prepared by Petruccelli Engineering

Page 32

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Summary for Link DP-6: DP-6

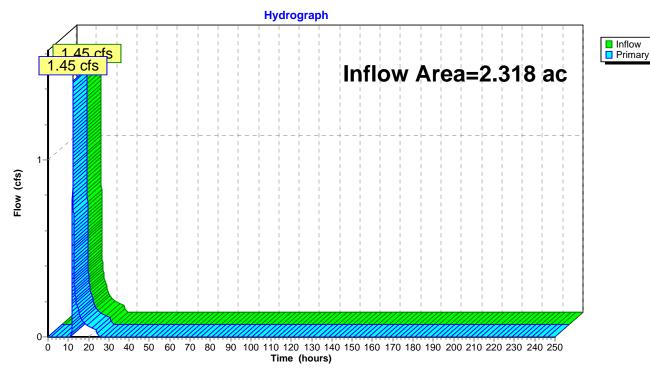
Inflow Area = 2.318 ac, 1.94% Impervious, Inflow Depth = 0.80" for 2 Year event

Inflow = 1.45 cfs @ 12.21 hrs, Volume= 0.155 af

Primary = 1.45 cfs @ 12.21 hrs, Volume= 0.155 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-6: DP-6



Summary for Link DP-7: DP-7

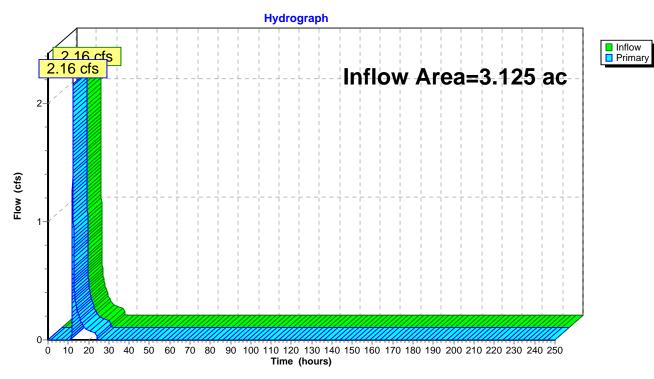
Inflow Area = 3.125 ac, 7.51% Impervious, Inflow Depth = 0.85" for 2 Year event

Inflow = 2.16 cfs @ 12.20 hrs, Volume= 0.221 af

Primary = 2.16 cfs @ 12.20 hrs, Volume= 0.221 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-7: DP-7



Printed 11/1/2010 Page 34

Time span=0.00-250.00 hrs, dt=0.01 hrs, 25001 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment1S: Subarea1	Runoff Area=14.548 ac 3.76% Impervious Runoff Depth=1.30" Flow Length=1,389' Tc=25.0 min CN=60 Runoff=12.15 cfs 1.577 af
Subcatchment 2S: Subarea-2	Runoff Area=228,729 sf 10.55% Impervious Runoff Depth=1.44" Flow Length=821' Tc=21.0 min CN=62 Runoff=5.35 cfs 0.629 af
Subcatchment 3S: Subarea-3	Runoff Area=105,316 sf 2.16% Impervious Runoff Depth=1.73" Flow Length=634' Tc=17.0 min CN=66 Runoff=3.36 cfs 0.348 af
Subcatchment 4S: Subarea-4	Runoff Area=186,025 sf 5.92% Impervious Runoff Depth=1.30" Flow Length=680' Tc=19.6 min CN=60 Runoff=3.94 cfs 0.463 af
Subcatchment 5S: Subarea-5	Runoff Area=136,628 sf 18.34% Impervious Runoff Depth=2.12" Flow Length=877' Tc=21.9 min CN=71 Runoff=4.96 cfs 0.553 af
Subcatchment 6S: Subarea-6	Runoff Area=100,951 sf 1.94% Impervious Runoff Depth=1.73" Flow Length=504' Tc=13.5 min CN=66 Runoff=3.52 cfs 0.334 af
Subcatchment7S: Subarea-7	Runoff Area=136,133 sf 7.51% Impervious Runoff Depth=1.80" Flow Length=594' Tc=12.8 min CN=67 Runoff=5.10 cfs 0.470 af
Link DP-1: DP-1	Inflow=12.15 cfs 1.577 af Primary=12.15 cfs 1.577 af
Link DP-2: DP-2	Inflow=5.35 cfs 0.629 af Primary=5.35 cfs 0.629 af
Link DP-3: DP-3	Inflow=3.36 cfs 0.348 af Primary=3.36 cfs 0.348 af
Link DP-4: DP-4	Inflow=3.94 cfs 0.463 af Primary=3.94 cfs 0.463 af
Link DP-5: DP-5	Inflow=4.96 cfs 0.553 af Primary=4.96 cfs 0.553 af
Link DP-6: DP-6	Inflow=3.52 cfs 0.334 af Primary=3.52 cfs 0.334 af
Link DP-7: DP-7	Inflow=5.10 cfs 0.470 af Primary=5.10 cfs 0.470 af

Total Runoff Area = 35.066 ac Runoff Volume = 4.374 af Average Runoff Depth = 1.50" 93.55% Pervious = 32.806 ac 6.45% Impervious = 2.261 ac

Page 35

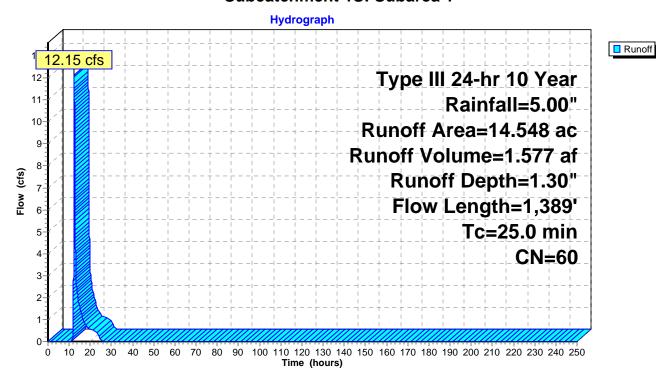
Summary for Subcatchment 1S: Subarea 1

Runoff = 12.15 cfs @ 12.39 hrs, Volume= 1.577 af, Depth= 1.30"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.00"

Area	(ac) C	N Desc	cription				
_	0.547 98 Paved roads w/curbs & sewers						
14.	001 5	<u>8 Woo</u>	ds/grass c	omb., Goo	d, HSG B		
14.	548 6	0 Weig	ghted Aver	age			
14.	001	96.2	4% Pervio	us Area			
0.	547	3.76	% Impervi	ous Area			
			•				
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
10.8	100	0.1000	0.15		Sheet Flow, 1 to 2		
					Woods: Light underbrush n= 0.400 P2= 3.50"		
0.9	171	0.3600	3.00		Shallow Concentrated Flow, 2 to 3		
					Woodland Kv= 5.0 fps		
13.2	1,118	0.0800	1.41		Shallow Concentrated Flow, 3 to DP-1		
	,				Woodland Kv= 5.0 fps		
25.0	1,389	Total			•		

Subcatchment 1S: Subarea 1



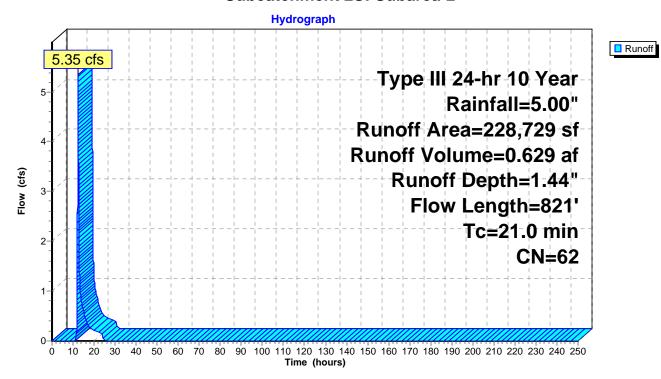
Summary for Subcatchment 2S: Subarea-2

Runoff = 5.35 cfs @ 12.32 hrs, Volume= 0.629 af, Depth= 1.44"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.00"

_	Area (sf) CN Description						
		24,134			s w/curbs &		
_	2	04,595	58 V	Voods/gras	ss comb., G	Good, HSG B	
	2	28,729	62 V	Veighted A	verage		
	2	04,595	8	9.45% Per	vious Area		
		24,134	1	0.55% Imp	ervious Ar	ea	
	Tc	Length	Slope	Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	9.4	100	0.1400	0.18		Sheet Flow, 1 to 2	
						Woods: Light underbrush n= 0.400 P2= 3.50"	
	11.6	721	0.0430	1.04		Shallow Concentrated Flow, 2 to 3	
						Woodland Kv= 5.0 fps	
-	21.0	821	Total			·	

Subcatchment 2S: Subarea-2



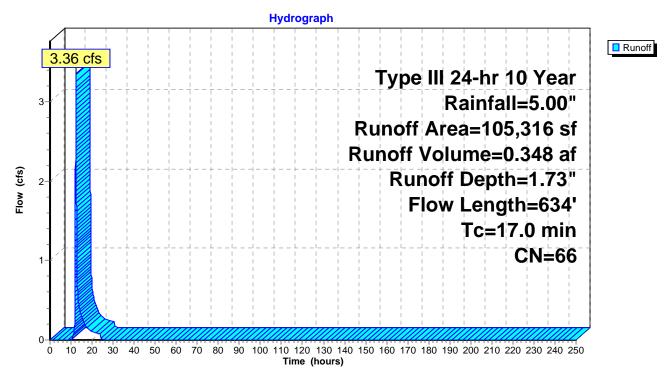
Summary for Subcatchment 3S: Subarea-3

Runoff = 3.36 cfs @ 12.25 hrs, Volume= 0.348 af, Depth= 1.73"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.00"

	Α	rea (sf)	CN D	Description						
		2,270 98 Paved roads w/curbs & sewers								
103,046 65 Woods/grass comb., Fair, HSG B						air, HSG B				
105,316 66 Weighted Average										
103,046 97.84% Pervious Area					vious Area					
2,270 2.16% Impervious Area					ervious Area	a				
	_									
	Tc	Length	Slope	Velocity	Capacity	Description				
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	9.9	100	0.1250	0.17		Sheet Flow, 1 to 2				
						Woods: Light underbrush n= 0.400 P2= 3.50"				
	7.1	534	0.0620	1.24		Shallow Concentrated Flow, 2 to DP-3				
						Woodland Kv= 5.0 fps				
	17.0	634	Total							

Subcatchment 3S: Subarea-3



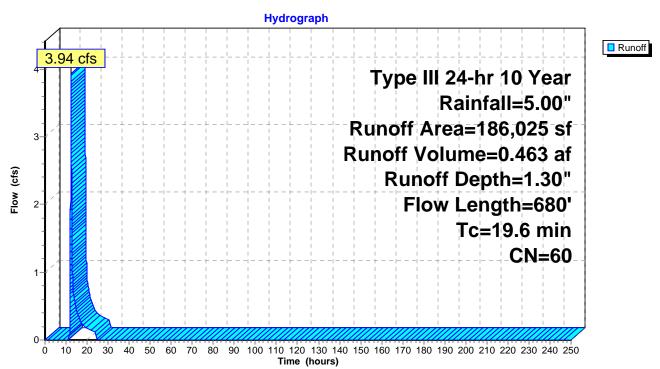
Summary for Subcatchment 4S: Subarea-4

Runoff = 3.94 cfs @ 12.30 hrs, Volume= 0.463 af, Depth= 1.30"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.00"

_	Area (sf) CN Description						
11,012 98 Paved roads w/curbs 8						& sewers	
_	1	75,013	58 V	Voods/gras	ss comb., G	Good, HSG B	
	186,025 60 Weighted Average						
175,013 94.08% Pervious Area					vious Area		
		11,012	5	.92% Impe	ervious Area	a	
	Тс	Length	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	9.4	100	0.1400	0.18		Sheet Flow, 1 to 2	
						Woods: Light underbrush n= 0.400 P2= 3.50"	
	10.2	580	0.0360	0.95		Shallow Concentrated Flow, 2 to DP-4	
						Woodland Kv= 5.0 fps	
	19.6	680	Total				

Subcatchment 4S: Subarea-4



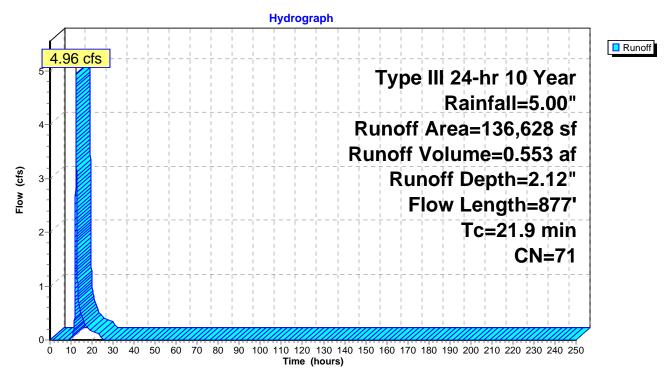
Summary for Subcatchment 5S: Subarea-5

Runoff = 4.96 cfs @ 12.31 hrs, Volume= 0.553 af, Depth= 2.12"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.00"

_	Α	Area (sf) CN Description							
		25,055			s w/curbs &				
_	1	11,573	65 V	Voods/gras	ss comb., F	air, HSG B			
	1	36,628	71 V	Veighted A	verage				
111,573 81.66% Pervious Area					vious Area				
		25,055	1	8.34% Imp	pervious Ar	ea			
	Tc	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	13.7	100	0.0550	0.12		Sheet Flow, 1 to 2			
						Woods: Light underbrush n= 0.400 P2= 3.50"			
	8.2	777	0.0990	1.57		Shallow Concentrated Flow, 2 to DP-5			
						Woodland Kv= 5.0 fps			
	21.9	877	Total	-					

Subcatchment 5S: Subarea-5



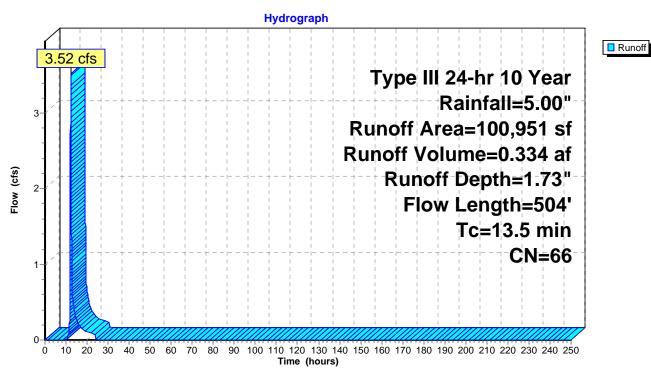
Summary for Subcatchment 6S: Subarea-6

Runoff = 3.52 cfs @ 12.20 hrs, Volume= 0.334 af, Depth= 1.73"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.00"

_	Α	rea (sf)	sf) CN Description							
1,957 98 Paved roads w/curbs 8										
_		98,994	65 V	Voods/gras	ss comb., F	air, HSG B				
100,951 66 Weighted Average					verage					
98,994 98.06% Pervious Area					vious Area					
		1,957	1	.94% Impe	ervious Area	a				
	·									
	Tc	Length	Slope	Velocity	Capacity	Description				
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
	9.7	100	0.1300	0.17		Sheet Flow, 1 to 2				
						Woods: Light underbrush n= 0.400 P2= 3.50"				
	3.8	404	0.1260	1.77		Shallow Concentrated Flow, 2 to DP-6				
						Woodland Kv= 5.0 fps				
	13.5	504	Total	-						

Subcatchment 6S: Subarea-6



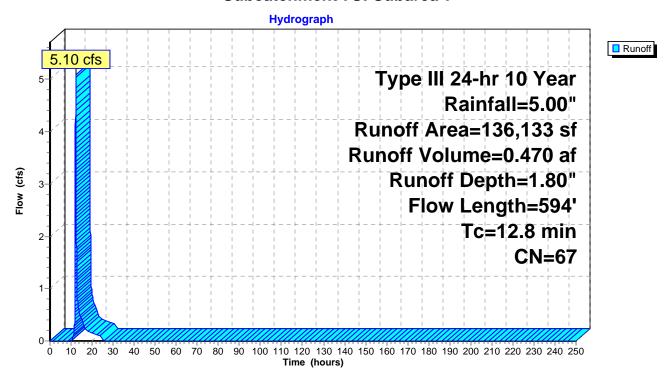
Summary for Subcatchment 7S: Subarea-7

Runoff = 5.10 cfs @ 12.19 hrs, Volume= 0.470 af, Depth= 1.80"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.00"

	Area (sf) CN Description						
	10,223 98 Paved roads w/curbs 8						
_	1	25,910	65 V	Voods/gras	ss comb., F	air, HSG B	
	136,133 67 Weighted Average						
	125,910 92.49% Pervious Area						
		10,223	7	.51% Impe	ervious Area	a	
	•						
	Tc	Length	Slope	Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	8.9	100	0.1600	0.19		Sheet Flow, 1 to 2	
						Woods: Light underbrush n= 0.400 P2= 3.50"	
	3.9	494	0.1780	2.11		Shallow Concentrated Flow, 2 to DP-7	
						Woodland Kv= 5.0 fps	
_	12.8	594	Total			·	

Subcatchment 7S: Subarea-7



Summary for Link DP-1: DP-1

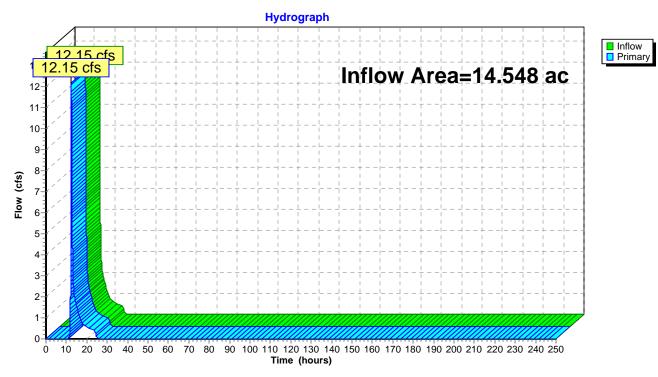
Inflow Area = 14.548 ac, 3.76% Impervious, Inflow Depth = 1.30" for 10 Year event

Inflow = 12.15 cfs @ 12.39 hrs, Volume= 1.577 af

Primary = 12.15 cfs @ 12.39 hrs, Volume= 1.577 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-1: DP-1



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Summary for Link DP-2: DP-2

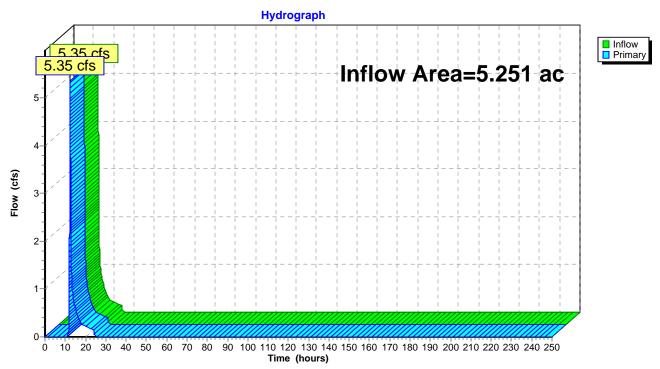
Inflow Area = 5.251 ac, 10.55% Impervious, Inflow Depth = 1.44" for 10 Year event

Inflow = 5.35 cfs @ 12.32 hrs, Volume= 0.629 af

Primary = 5.35 cfs @ 12.32 hrs, Volume= 0.629 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-2: DP-2



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Summary for Link DP-3: DP-3

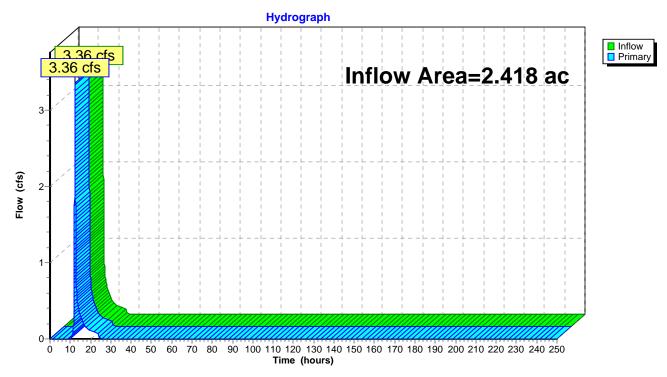
Inflow Area = 2.418 ac, 2.16% Impervious, Inflow Depth = 1.73" for 10 Year event

Inflow = 3.36 cfs @ 12.25 hrs, Volume= 0.348 af

Primary = 3.36 cfs @ 12.25 hrs, Volume= 0.348 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-3: DP-3



Summary for Link DP-4: DP-4

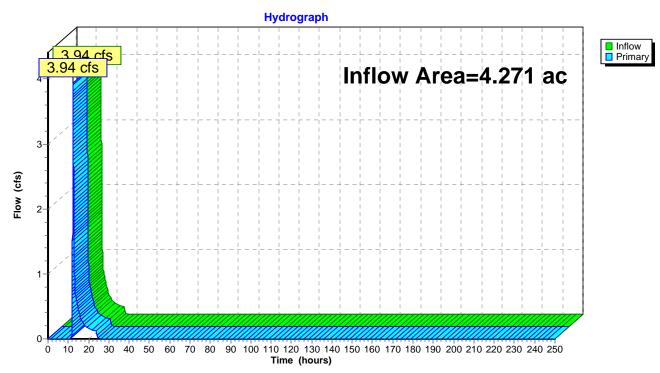
Inflow Area = 4.271 ac, 5.92% Impervious, Inflow Depth = 1.30" for 10 Year event

Inflow = 3.94 cfs @ 12.30 hrs, Volume= 0.463 af

Primary = 3.94 cfs @ 12.30 hrs, Volume= 0.463 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-4: DP-4



Summary for Link DP-5: DP-5

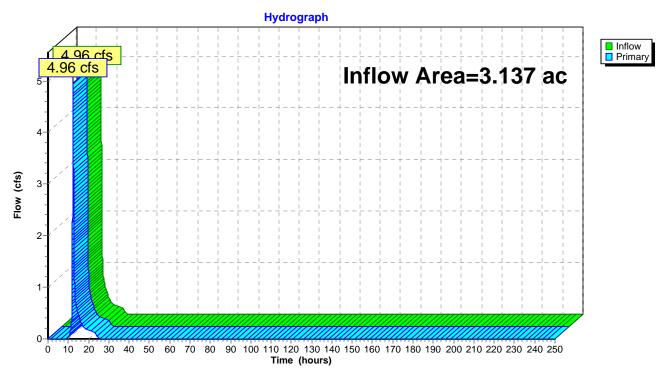
Inflow Area = 3.137 ac, 18.34% Impervious, Inflow Depth = 2.12" for 10 Year event

Inflow = 4.96 cfs @ 12.31 hrs, Volume= 0.553 af

Primary = 4.96 cfs @ 12.31 hrs, Volume= 0.553 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-5: DP-5



Summary for Link DP-6: DP-6

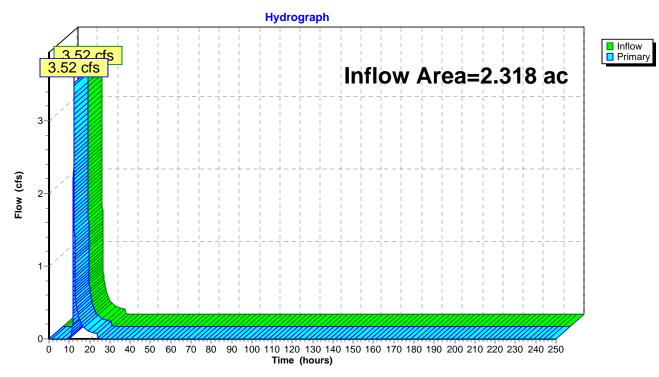
Inflow Area = 2.318 ac, 1.94% Impervious, Inflow Depth = 1.73" for 10 Year event

Inflow = 3.52 cfs @ 12.20 hrs, Volume= 0.334 af

Primary = 3.52 cfs @ 12.20 hrs, Volume= 0.334 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-6: DP-6



Summary for Link DP-7: DP-7

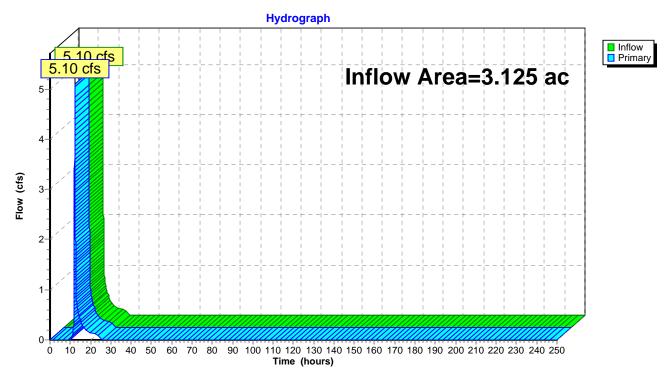
Inflow Area = 3.125 ac, 7.51% Impervious, Inflow Depth = 1.80" for 10 Year event

Inflow = 5.10 cfs @ 12.19 hrs, Volume= 0.470 af

Primary = 5.10 cfs @ 12.19 hrs, Volume= 0.470 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-7: DP-7



Printed 11/1/2010 Page 49

Time span=0.00-250.00 hrs, dt=0.01 hrs, 25001 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment1S: Subarea1	Runoff Area=14.548 ac 3.76% Impervious Runoff Depth=2.96" Flow Length=1,389' Tc=25.0 min CN=60 Runoff=30.24 cfs 3.592 af
Subcatchment 2S: Subarea-2	Runoff Area=228,729 sf 10.55% Impervious Runoff Depth=3.17" Flow Length=821' Tc=21.0 min CN=62 Runoff=12.69 cfs 1.389 af
Subcatchment3S: Subarea-3	Runoff Area=105,316 sf 2.16% Impervious Runoff Depth=3.60" Flow Length=634' Tc=17.0 min CN=66 Runoff=7.30 cfs 0.726 af
Subcatchment 4S: Subarea-4	Runoff Area=186,025 sf 5.92% Impervious Runoff Depth=2.96" Flow Length=680' Tc=19.6 min CN=60 Runoff=9.81 cfs 1.055 af
Subcatchment 5S: Subarea-5	Runoff Area=136,628 sf 18.34% Impervious Runoff Depth=4.15" Flow Length=877' Tc=21.9 min CN=71 Runoff=9.90 cfs 1.084 af
Subcatchment 6S: Subarea-6	Runoff Area=100,951 sf 1.94% Impervious Runoff Depth=3.60" Flow Length=504' Tc=13.5 min CN=66 Runoff=7.66 cfs 0.696 af
Subcatchment7S: Subarea-7	Runoff Area=136,133 sf 7.51% Impervious Runoff Depth=3.71" Flow Length=594' Tc=12.8 min CN=67 Runoff=10.87 cfs 0.966 af
Link DP-1: DP-1	Inflow=30.24 cfs 3.592 af Primary=30.24 cfs 3.592 af
Link DP-2: DP-2	Inflow=12.69 cfs 1.389 af Primary=12.69 cfs 1.389 af
Link DP-3: DP-3	Inflow=7.30 cfs 0.726 af Primary=7.30 cfs 0.726 af
Link DP-4: DP-4	Inflow=9.81 cfs 1.055 af Primary=9.81 cfs 1.055 af
Link DP-5: DP-5	Inflow=9.90 cfs 1.084 af Primary=9.90 cfs 1.084 af
Link DP-6: DP-6	Inflow=7.66 cfs 0.696 af Primary=7.66 cfs 0.696 af
Link DP-7: DP-7	Inflow=10.87 cfs 0.966 af Primary=10.87 cfs 0.966 af

Total Runoff Area = 35.066 ac Runoff Volume = 9.507 af Average Runoff Depth = 3.25" 93.55% Pervious = 32.806 ac 6.45% Impervious = 2.261 ac

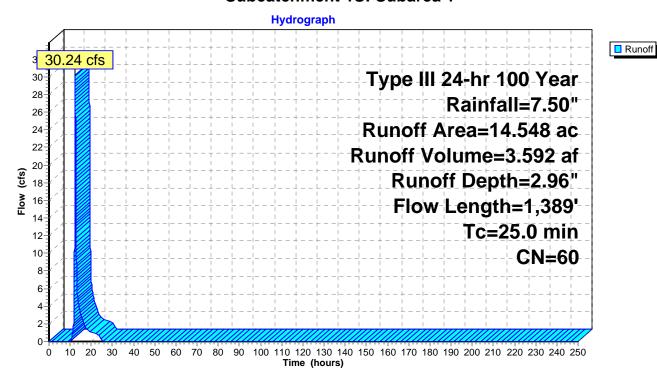
Summary for Subcatchment 1S: Subarea 1

30.24 cfs @ 12.36 hrs, Volume= 3.592 af, Depth= 2.96" Runoff

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 100 Year Rainfall=7.50"

_	Area	(ac) C	N Desc	cription		
	0.	547 9	8 Pave	ed roads w	ewers	
14.001 58 Woods/grass comb., Good, HSG B						
	14.	548 6	0 Weig	ghted Aver	age	
	14.	001	96.2	4% Pervio	us Area	
	0.	547	3.76	% Impervi	ous Area	
	Tc	Length	Slope	Velocity	Capacity	Description
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	10.8	100	0.1000	0.15		Sheet Flow, 1 to 2
						Woods: Light underbrush n= 0.400 P2= 3.50"
	0.9	171	0.3600	3.00		Shallow Concentrated Flow, 2 to 3
						Woodland Kv= 5.0 fps
	13.2	1,118	0.0800	1.41		Shallow Concentrated Flow, 3 to DP-1
		•				Woodland Kv= 5.0 fps
	25.0	1,389	Total			

Subcatchment 1S: Subarea 1



Page 51

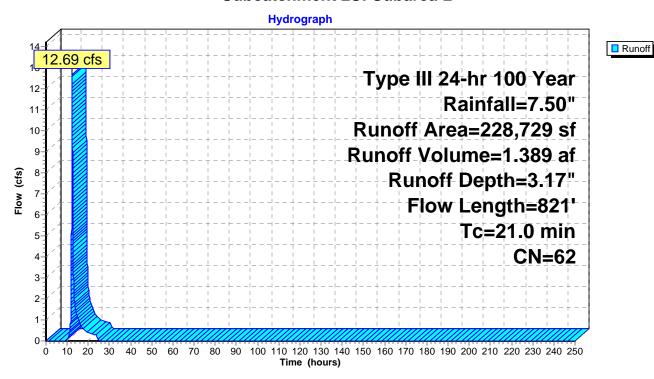
Summary for Subcatchment 2S: Subarea-2

Runoff = 12.69 cfs @ 12.30 hrs, Volume= 1.389 af, Depth= 3.17"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 100 Year Rainfall=7.50"

_	Α	Area (sf) CN Description							
24,134 98 Paved roads w/curbs &									
_	2	04,595	58 V	Voods/gras	ss comb., G	Good, HSG B			
228,729 62 Weighted Average					verage				
204,595 89.45% Pervious Area					vious Area				
		24,134	1	0.55% Imp	pervious Ar	ea			
	·								
	Tc	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	9.4	100	0.1400	0.18		Sheet Flow, 1 to 2			
						Woods: Light underbrush n= 0.400 P2= 3.50"			
	11.6	721	0.0430	1.04		Shallow Concentrated Flow, 2 to 3			
						Woodland Kv= 5.0 fps			
	21.0	821	Total	-					

Subcatchment 2S: Subarea-2



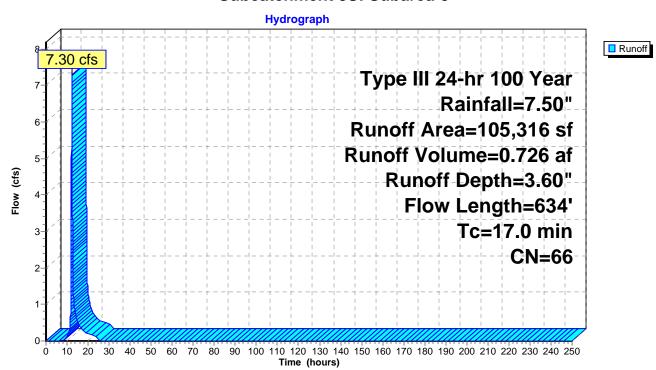
Summary for Subcatchment 3S: Subarea-3

Runoff = 7.30 cfs @ 12.24 hrs, Volume= 0.726 af, Depth= 3.60"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 100 Year Rainfall=7.50"

Area (sf) CN Description						
		2,270	98 P	aved road	s w/curbs &	& sewers
_	1	03,046	65 V	Voods/gras	ss comb., F	air, HSG B
	1	05,316	66 V	Veighted A	verage	
	1	03,046	9	7.84% Per	vious Area	
		2,270	2	.16% Impe	ervious Area	a
·						
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	9.9	100	0.1250	0.17		Sheet Flow, 1 to 2
						Woods: Light underbrush n= 0.400 P2= 3.50"
	7.1	534	0.0620	1.24		Shallow Concentrated Flow, 2 to DP-3
						Woodland Kv= 5.0 fps
	17.0	634	Total			

Subcatchment 3S: Subarea-3



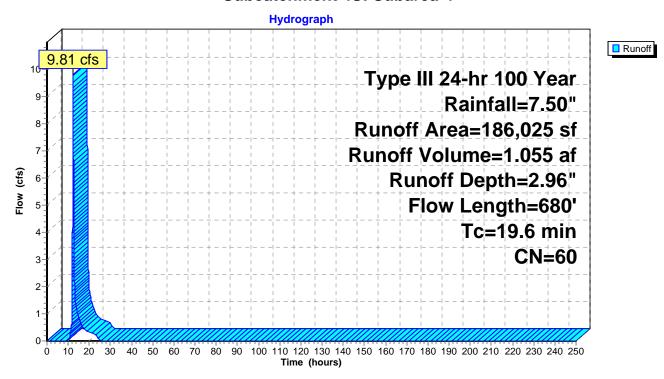
Summary for Subcatchment 4S: Subarea-4

Runoff = 9.81 cfs @ 12.28 hrs, Volume= 1.055 af, Depth= 2.96"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 100 Year Rainfall=7.50"

_	Area (sf) CN Description						
11,012 98 Paved roads w/curbs 8						& sewers	
_	1	75,013	58 V	Voods/gras	ss comb., G	Good, HSG B	
	186,025 60 Weighted Average						
175,013 94.08% Pervious Area					vious Area		
		11,012	5	.92% Impe	ervious Area	a	
	Тс	Length	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	9.4	100	0.1400	0.18		Sheet Flow, 1 to 2	
						Woods: Light underbrush n= 0.400 P2= 3.50"	
	10.2	580	0.0360	0.95		Shallow Concentrated Flow, 2 to DP-4	
						Woodland Kv= 5.0 fps	
	19.6	680	Total				

Subcatchment 4S: Subarea-4



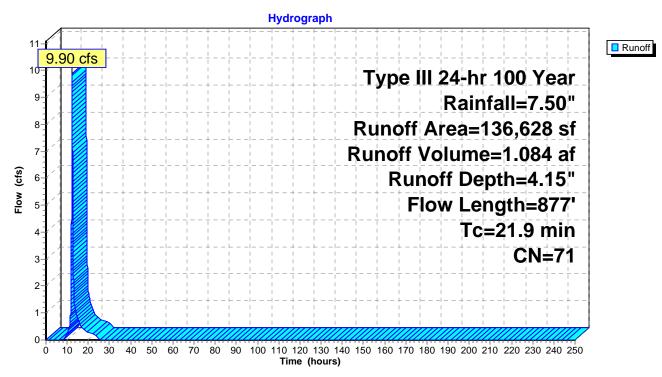
Summary for Subcatchment 5S: Subarea-5

Runoff = 9.90 cfs @ 12.29 hrs, Volume= 1.084 af, Depth= 4.15"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 100 Year Rainfall=7.50"

_	Α	Area (sf) CN Description							
		25,055			s w/curbs &				
_	1	11,573	65 V	Voods/gras	ss comb., F	air, HSG B			
	1	36,628	71 V	Veighted A	verage				
111,573 81.66% Pervious Area					vious Area				
		25,055	1	8.34% Imp	pervious Ar	ea			
	Tc	Length	Slope	Velocity	Capacity	Description			
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	13.7	100	0.0550	0.12		Sheet Flow, 1 to 2			
						Woods: Light underbrush n= 0.400 P2= 3.50"			
	8.2	777	0.0990	1.57		Shallow Concentrated Flow, 2 to DP-5			
						Woodland Kv= 5.0 fps			
	21.9	877	Total	-					

Subcatchment 5S: Subarea-5



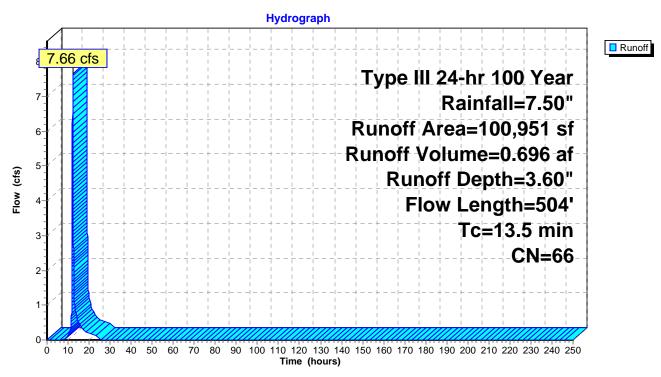
Summary for Subcatchment 6S: Subarea-6

Runoff = 7.66 cfs @ 12.19 hrs, Volume= 0.696 af, Depth= 3.60"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 100 Year Rainfall=7.50"

	Area (sf) CN Description						
	1,957 98 Paved roads w/curbs 8						
_		98,994	65 V	Voods/gras	ss comb., F	air, HSG B	
	100,951 66 Weighted Average						
98,994 98.06% Pervious Area					vious Area		
		1,957	1	.94% Impe	ervious Area	a	
	•						
	Tc	Length	Slope	Velocity	Capacity	Description	
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	9.7	100	0.1300	0.17		Sheet Flow, 1 to 2	
						Woods: Light underbrush n= 0.400 P2= 3.50"	
	3.8	404	0.1260	1.77		Shallow Concentrated Flow, 2 to DP-6	
						Woodland Kv= 5.0 fps	
	13.5	504	Total				

Subcatchment 6S: Subarea-6



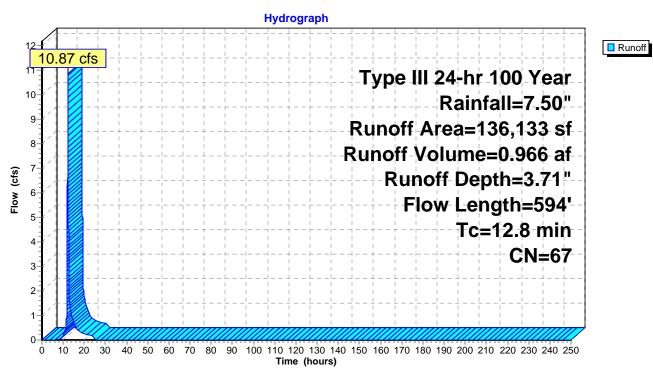
Summary for Subcatchment 7S: Subarea-7

Runoff = 10.87 cfs @ 12.18 hrs, Volume= 0.966 af, Depth= 3.71"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 100 Year Rainfall=7.50"

	Α	rea (sf)	CN E	Description					
		& sewers							
_	125,910 65 Woods/grass comb., Fair, HSG B								
136,133 67 Weighted Average					verage				
	1	25,910	9	2.49% Per	vious Area				
		10,223	7	.51% Impe	ervious Area	a			
·									
	Tc	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	8.9	100	0.1600	0.19		Sheet Flow, 1 to 2			
						Woods: Light underbrush n= 0.400 P2= 3.50"			
	3.9	494	0.1780	2.11		Shallow Concentrated Flow, 2 to DP-7			
						Woodland Kv= 5.0 fps			
_	12.8	594	Total			·			

Subcatchment 7S: Subarea-7



Summary for Link DP-1: DP-1

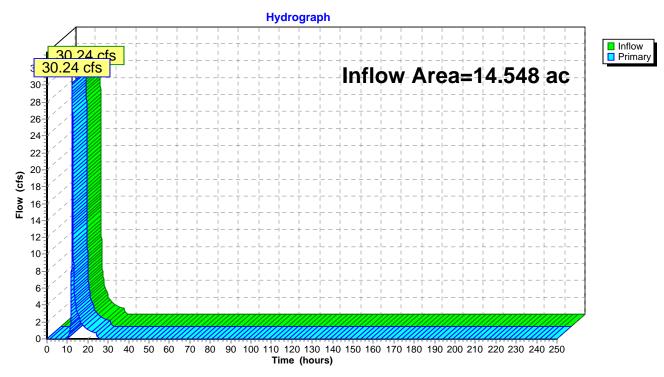
Inflow Area = 14.548 ac, 3.76% Impervious, Inflow Depth = 2.96" for 100 Year event

Inflow = 30.24 cfs @ 12.36 hrs, Volume= 3.592 af

Primary = 30.24 cfs @ 12.36 hrs, Volume= 3.592 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-1: DP-1



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Page 58

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Summary for Link DP-2: DP-2

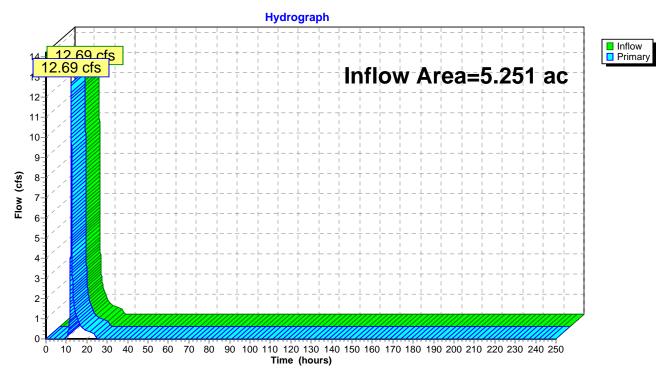
Inflow Area = 5.251 ac, 10.55% Impervious, Inflow Depth = 3.17" for 100 Year event

Inflow = 12.69 cfs @ 12.30 hrs, Volume= 1.389 af

Primary = 12.69 cfs @ 12.30 hrs, Volume= 1.389 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-2: DP-2



Page 59

Summary for Link DP-3: DP-3

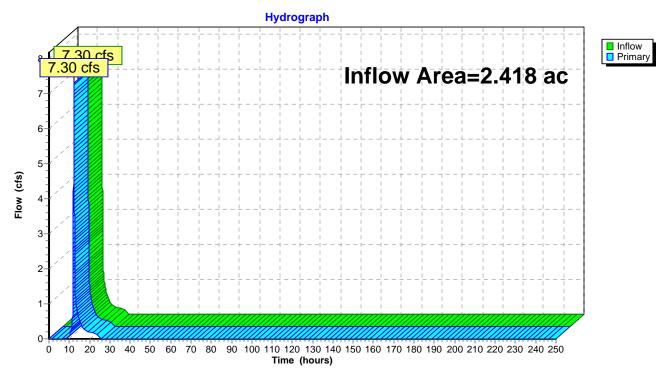
Inflow Area = 2.418 ac, 2.16% Impervious, Inflow Depth = 3.60" for 100 Year event

Inflow = 7.30 cfs @ 12.24 hrs, Volume= 0.726 af

Primary = 7.30 cfs @ 12.24 hrs, Volume= 0.726 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-3: DP-3



Summary for Link DP-4: DP-4

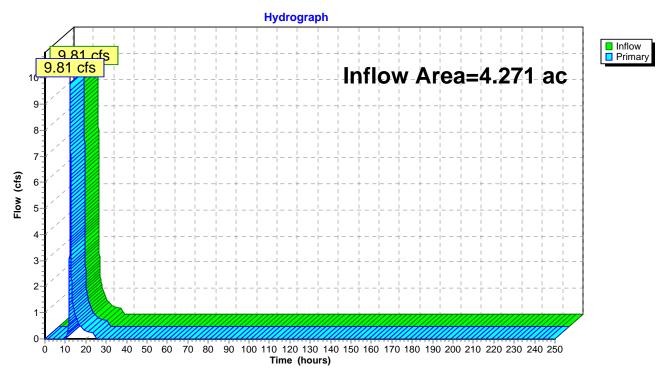
Inflow Area = 4.271 ac, 5.92% Impervious, Inflow Depth = 2.96" for 100 Year event

Inflow = 9.81 cfs @ 12.28 hrs, Volume= 1.055 af

Primary = 9.81 cfs @ 12.28 hrs, Volume= 1.055 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-4: DP-4



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Page 61

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Summary for Link DP-5: DP-5

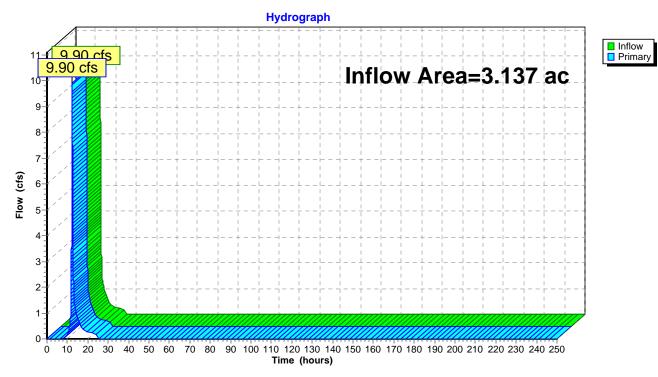
Inflow Area = 3.137 ac, 18.34% Impervious, Inflow Depth = 4.15" for 100 Year event

Inflow = 9.90 cfs @ 12.29 hrs, Volume= 1.084 af

Primary = 9.90 cfs @ 12.29 hrs, Volume= 1.084 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-5: DP-5



Summary for Link DP-6: DP-6

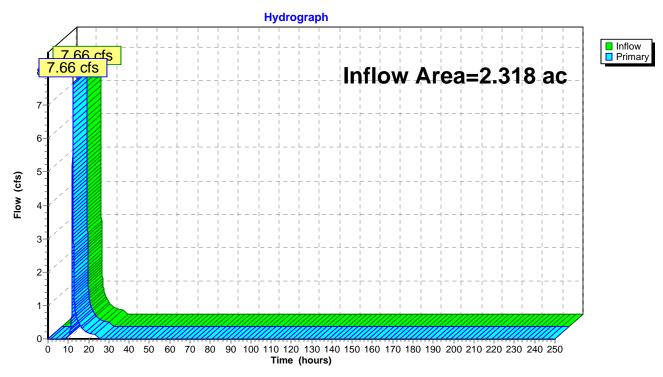
Inflow Area = 2.318 ac, 1.94% Impervious, Inflow Depth = 3.60" for 100 Year event

Inflow = 7.66 cfs @ 12.19 hrs, Volume= 0.696 af

Primary = 7.66 cfs @ 12.19 hrs, Volume= 0.696 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-6: DP-6



Page 63

Summary for Link DP-7: DP-7

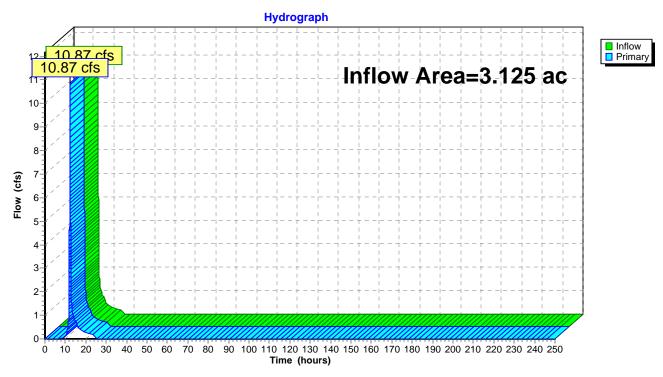
Inflow Area = 3.125 ac, 7.51% Impervious, Inflow Depth = 3.71" for 100 Year event

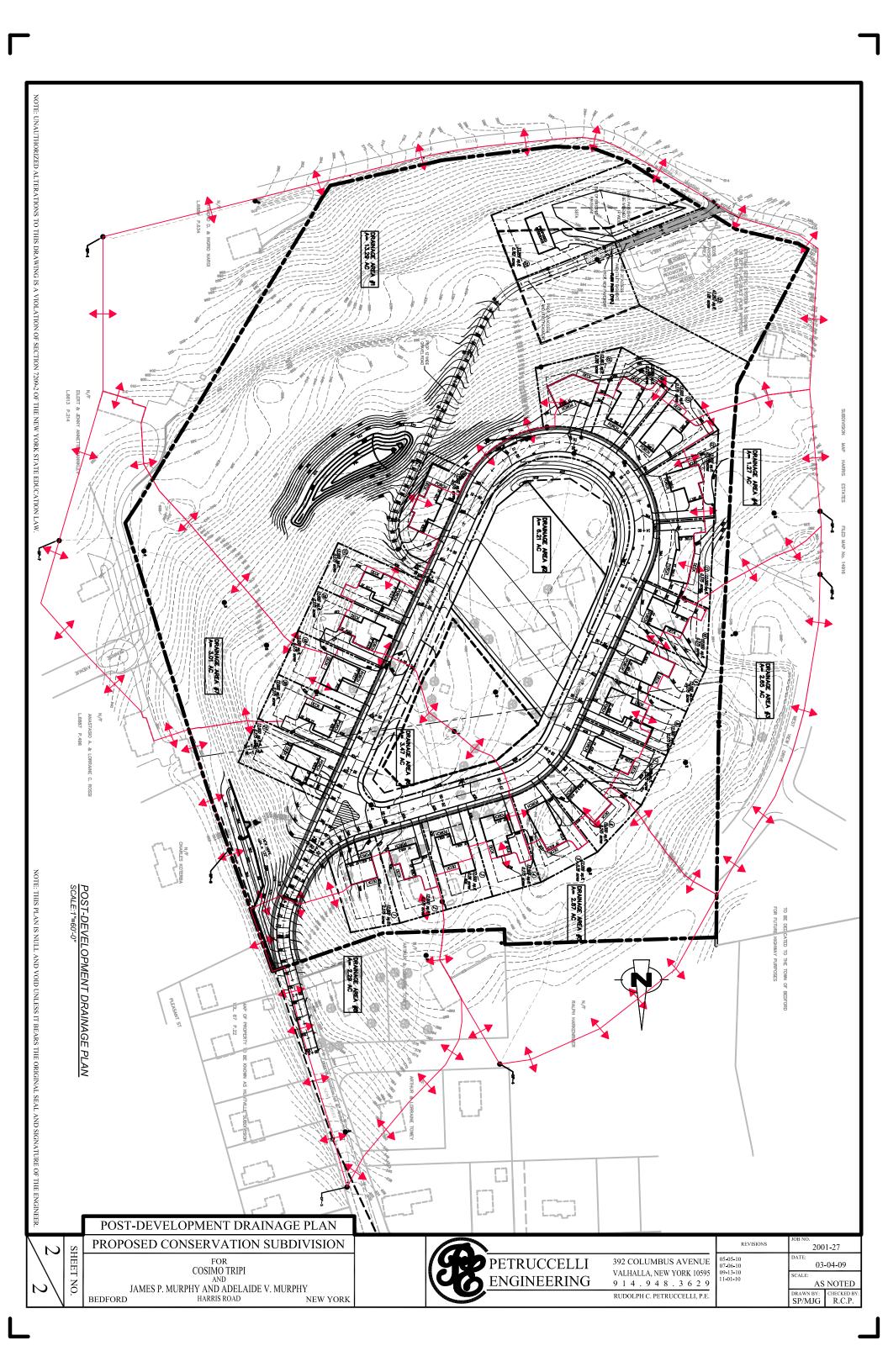
Inflow = 10.87 cfs @ 12.18 hrs, Volume= 0.966 af

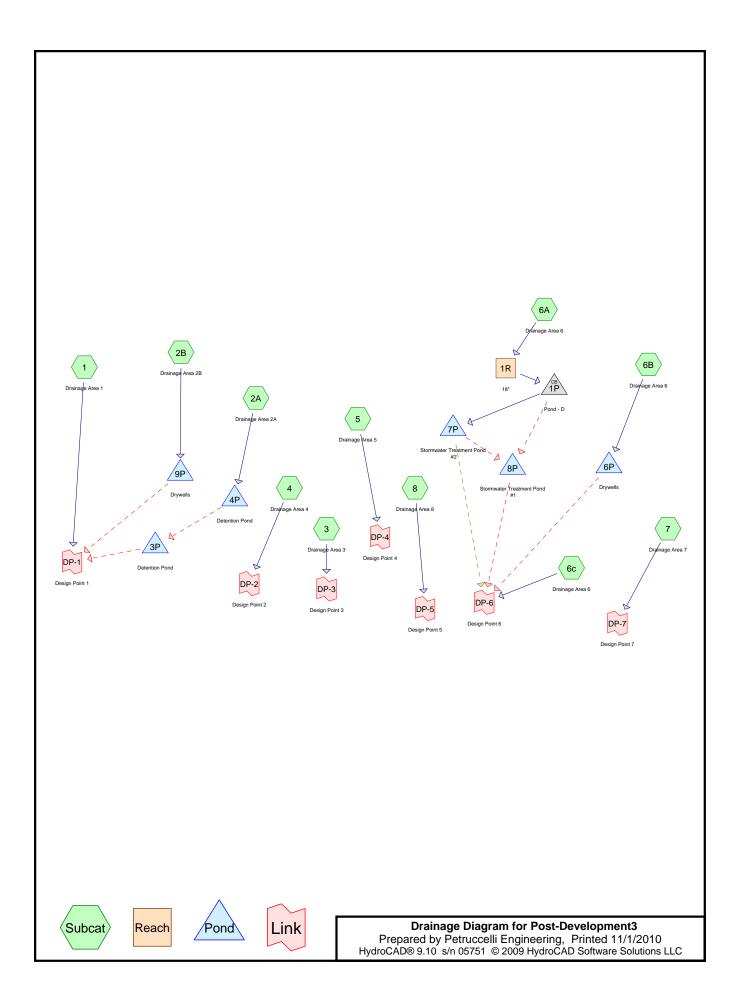
Primary = 10.87 cfs @ 12.18 hrs, Volume= 0.966 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-7: DP-7







Printed 11/1/2010

Page 2

Area Listing (all nodes)

Aı	rea CN	Description
(acre	es)	(subcatchment-numbers)
13.5	564 55	Woods, Good, HSG B (1, 2A, 5, 6A)
4.6	60	Woods, Fair, HSG B (3, 4, 6c, 7, 8)
8.7	7 46 61	>75% Grass cover, Good, HSG B (2A, 3, 4, 5, 6A, 7, 8)
2.7	770 69	50-75% Grass cover, Fair, HSG B (1)
0.2	263 82	Dirt roads, HSG B (1)
0.0)16 98	Paved parking, HSG B (5)
5.1	03 98	Paved roads w/curbs & sewers (1, 2A, 2B, 3, 4, 6A, 6B, 7, 8)
35.0	087	TOTAL AREA

Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
0.000	HSG A	
29.984	HSG B	1, 2A, 3, 4, 5, 6A, 6c, 7, 8
0.000	HSG C	
0.000	HSG D	
5.103	Other	1, 2A, 2B, 3, 4, 6A, 6B, 7, 8
35.087		TOTAL AREA

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Page 4

Pipe Listing (all nodes)

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Fill (inches)
1	2A	0.00	0.00	25.0	0.0100	0.013	12.0	0.0	0.0
2	2A	0.00	0.00	167.0	0.0120	0.013	12.0	0.0	0.0
3	2A	0.00	0.00	70.0	0.0090	0.025	15.0	0.0	0.0
4	2A	0.00	0.00	140.0	0.1510	0.025	18.0	0.0	0.0
5	2B	0.00	0.00	25.0	0.0100	0.013	12.0	0.0	0.0
6	2B	0.00	0.00	167.0	0.0120	0.013	12.0	0.0	0.0
7	2B	0.00	0.00	70.0	0.0090	0.025	15.0	0.0	0.0
8	2B	0.00	0.00	140.0	0.1510	0.025	18.0	0.0	0.0
9	6A	0.00	0.00	17.0	0.0120	0.012	15.0	0.0	0.0
10	6A	0.00	0.00	132.0	0.0077	0.013	18.0	0.0	0.0
11	6B	0.00	0.00	17.0	0.0120	0.012	15.0	0.0	0.0
12	6B	0.00	0.00	101.0	0.1730	0.013	18.0	0.0	0.0
13	6c	0.00	0.00	17.0	0.0120	0.012	15.0	0.0	0.0
14	6c	0.00	0.00	101.0	0.1730	0.013	18.0	0.0	0.0
15	1R	374.01	366.80	132.0	0.0546	0.013	18.0	0.0	0.0
16	1P	366.80	361.00	6.0	0.9667	0.025	18.0	0.0	0.0
17	8P	361.00	330.00	200.0	0.1550	0.013	24.0	0.0	0.0

Time span=0.00-250.00 hrs, dt=0.01 hrs, 25001 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

reddir redding by eter i	ma method if one routing by otor ma method
Subcatchment1: Drainage Area 1	Runoff Area=13.292 ac 3.27% Impervious Runoff Depth=0.26" Flow Length=1,455' Tc=31.8 min CN=60 Runoff=1.20 cfs 0.293 af
Subcatchment 2A: Drainage Area 2A	Runoff Area=5.751 ac 33.25% Impervious Runoff Depth=0.74" Flow Length=999' Tc=19.1 min CN=73 Runoff=3.05 cfs 0.353 af
Subcatchment 2B: Drainage Area 2B	Runoff Area=19,994 sf 100.00% Impervious Runoff Depth=2.57" Flow Length=999' Tc=19.1 min CN=98 Runoff=0.86 cfs 0.098 af
Subcatchment 3: Drainage Area 3	Runoff Area=115,560 sf 7.97% Impervious Runoff Depth=0.35" Flow Length=552' Tc=28.6 min CN=63 Runoff=0.41 cfs 0.078 af
Subcatchment 4: Drainage Area 4	Runoff Area=55,457 sf 7.29% Impervious Runoff Depth=0.35" Flow Length=284' Tc=22.5 min CN=63 Runoff=0.21 cfs 0.037 af
Subcatchment 5: Drainage Area 5	Runoff Area=125,820 sf 0.56% Impervious Runoff Depth=0.17" Flow Length=497' Tc=33.7 min CN=56 Runoff=0.10 cfs 0.040 af
Subcatchment 6A: Drainage Area 6	Runoff Area=2.937 ac 37.76% Impervious Runoff Depth=0.74" Flow Length=578' Tc=32.3 min CN=73 Runoff=1.25 cfs 0.180 af
Subcatchment 6B: Drainage Area 6	Runoff Area=10,050 sf 100.00% Impervious Runoff Depth=2.57" Flow Length=540' Tc=29.9 min CN=98 Runoff=0.36 cfs 0.049 af
Subcatchment 6c: Drainage Area 6	Runoff Area=13,208 sf 0.00% Impervious Runoff Depth=0.26" Flow Length=540' Tc=29.9 min CN=60 Runoff=0.03 cfs 0.007 af
Subcatchment 7: Drainage Area 7	Runoff Area=3.006 ac 9.68% Impervious Runoff Depth=0.38" Flow Length=527' Tc=13.4 min CN=64 Runoff=0.66 cfs 0.096 af
Subcatchment 8: Drainage Area 8	Runoff Area=99,910 sf 15.79% Impervious Runoff Depth=0.45" Flow Length=558' Tc=13.0 min CN=66 Runoff=0.68 cfs 0.086 af
Reach 1R: 18" 18.0" Round Pipe n=0.013 L	Avg. Flow Depth=0.23' Max Vel=7.28 fps Inflow=1.25 cfs 0.180 af =132.0' S=0.0546 '/' Capacity=24.55 cfs Outflow=1.25 cfs 0.180 af
Pond 1P: Pond - D Primary=1.25 c	Peak Elev=367.37' Inflow=1.25 cfs 0.180 af fs 0.180 af Secondary=0.00 cfs 0.000 af Outflow=1.25 cfs 0.180 af
Pond 3P: Detention Pond Primary=0.00 c	Peak Elev=363.00' Storage=0 cf Inflow=0.00 cfs 0.000 af fs 0.000 af Secondary=0.00 cfs 0.000 af Outflow=0.00 cfs 0.000 af
Pond 4P: Detention Pond Primary=0.44 c	Peak Elev=369.34' Storage=6,164 cf Inflow=3.05 cfs 0.353 af fs 0.353 af Secondary=0.00 cfs 0.000 af Outflow=0.44 cfs 0.353 af
Pond 6P: Drywells Primary=0.18 c	Peak Elev=386.63' Storage=309 cf Inflow=0.36 cfs 0.049 af fs 0.049 af Secondary=0.00 cfs 0.000 af Outflow=0.18 cfs 0.049 af

Post-Development3	Type III 24-hr 1 Year Rainfall=2.80"
Prepared by Petruccelli Engineering	Printed 11/1/2010
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- 1	2

Pond 7P: Stormwater Tr Primary=0.17 cfs 0.119 af			cf Inflow=1.25 cfs Outflow=1.20 cfs	
Pond 8P: Stormwater Tr	eatment Pond #1 Primary=0.21 cfs 0.		cf Inflow=1.03 cfs Outflow=0.70 cfs	
Pond 9P: Drywells	Primary=0.25 cfs 0.		cf Inflow=0.86 cfs Outflow=0.25 cfs	
Link DP-1: Design Point	1		Inflow=1.20 cfs Primary=1.20 cfs	
Link DP-2: Design Point	2		Inflow=0.21 cfs Primary=0.21 cfs	
Link DP-3: Design Point	3		Inflow=0.41 cfs Primary=0.41 cfs	
Link DP-4: Design Point	4		Inflow=0.10 cfs Primary=0.10 cfs	
Link DP-5: Design Point	5		Inflow=0.68 cfs Primary=0.68 cfs	
Link DP-6: Design Point	6		Inflow=0.51 cfs Primary=0.51 cfs	
Link DP-7: Design Point	7		Inflow=0.66 cfs	0.096 af

Total Runoff Area = 35.087 ac Runoff Volume = 1.319 af Average Runoff Depth = 0.45" 85.41% Pervious = 29.968 ac 14.59% Impervious = 5.119 ac

Primary=0.66 cfs 0.096 af

Page 7

Summary for Subcatchment 1: Drainage Area 1

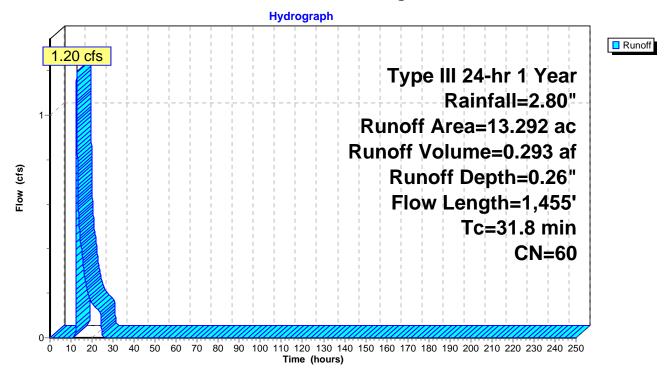
Runoff = 1.20 cfs @ 12.68 hrs, Volume= 0.293 af, Depth= 0.26"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 1 Year Rainfall=2.80"

Area	(ac) C	N Desc	cription						
2.	2.770 69 50-75% Grass cover, Fair, HSG B								
0.435 98 Paved roads w/curbs & sewers									
9.824 55 Woods, Good, HSG B									
0.263 82 Dirt roads, HSG B									
13.292 60 Weighted Average									
12.	857	96.7	3% Pervio	us Area					
0.	435	3.27	% Impervi	ous Area					
_		-							
Tc	Length	Slope	Velocity	Capacity	Description				
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)					
18.9	100	0.0260	0.09		Sheet Flow, 1 to 2				
4 7	040	0.4000	0.44		Grass: Bermuda n= 0.410 P2= 3.50"				
1.7	216	0.1830	2.14		Shallow Concentrated Flow, 2 to 3				
0.2	70	0.0450	4 24		Woodland Kv= 5.0 fps				
0.3	78	0.0450	4.31		Shallow Concentrated Flow, 3 to 4 Paved Kv= 20.3 fps				
1.2	121	0.1150	1.70		Shallow Concentrated Flow, 4 to 5				
1.2	121	0.1150	1.70		Woodland Kv= 5.0 fps				
5.8	679	0.0770	1.94		Shallow Concentrated Flow, 5 to 6				
0.0	0.0	0.0770	1.01		Short Grass Pasture Kv= 7.0 fps				
3.9	261	0.0500	1.12		Shallow Concentrated Flow, 6 to DP1				
					Woodland Kv= 5.0 fps				
31.8	1,455	Total			·				

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Subcatchment 1: Drainage Area 1



Page 9

Summary for Subcatchment 2A: Drainage Area 2A

Runoff = 3.05 cfs @ 12.29 hrs, Volume= 0.353 af, Depth= 0.74"

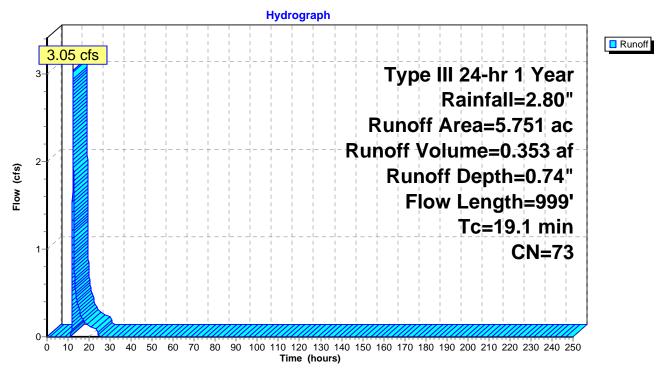
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 1 Year Rainfall=2.80"

Area	(ac) C	N Desc	cription				
	1.912 98 Paved roads w/curbs & sewers						
				over, Good,	, HSG B		
	0.520 55 Woods, Good, HSG B 5.751 73 Weighted Average						
	839		5% Pervio				
	912			ious Area			
			·				
Tc	Length	Slope	Velocity	Capacity	Description		
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)			
11.5	100	0.0900	0.15		Sheet Flow, 1 to 2		
- 0	400	0.0000	4.04		Grass: Bermuda n= 0.410 P2= 3.50"		
5.8	423	0.0300	1.21		Shallow Concentrated Flow, 2 to 3 Short Grass Pasture Kv= 7.0 fps		
0.5	74	0.0140	2.40		Shallow Concentrated Flow, 3 to 4 (Road)		
0.5	74	0.0140	2.40		Paved Kv= 20.3 fps		
0.1	25	0.0100	4.54	3.56			
			-		12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'		
					n= 0.013 Corrugated PE, smooth interior		
0.6	167	0.0120	4.97	3.90			
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'		
					n= 0.013 Corrugated PE, smooth interior		
0.4	70	0.0090	2.60	3.19			
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'		
0.0	4.40	0.4540	40.04	24.22	n= 0.025 Corrugated metal		
0.2	140	0.1510	12.01	21.23	Pipe Channel, 7 to 8 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'		
					n= 0.025 Corrugated metal		
19.1	999	Total			11- 0.020 Corrugated metal		
19.1	999	iolai					

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Page 10

Subcatchment 2A: Drainage Area 2A



Page 11

Summary for Subcatchment 2B: Drainage Area 2B

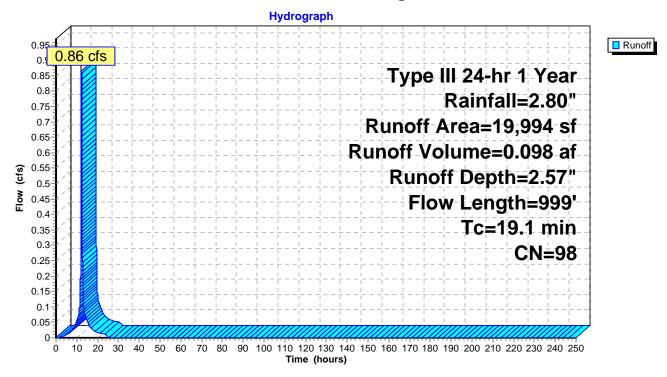
Runoff = 0.86 cfs @ 12.25 hrs, Volume= 0.098 af, Depth= 2.57"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 1 Year Rainfall=2.80"

 Α	rea (sf)	CN [Description		
	19,994	98 F	Paved road	s w/curbs 8	& sewers
	19,994	1	00.00% In	pervious A	rea
	•				
Tc	Length	Slope	Velocity	Capacity	Description
 (min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
11.5	100	0.0900	0.15		Sheet Flow, 1 to 2
					Grass: Bermuda n= 0.410 P2= 3.50"
5.8	423	0.0300	1.21		Shallow Concentrated Flow, 2 to 3
					Short Grass Pasture Kv= 7.0 fps
0.5	74	0.0140	2.40		Shallow Concentrated Flow, 3 to 4 (Road)
					Paved Kv= 20.3 fps
0.1	25	0.0100	4.54	3.56	
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
0.6	167	0.0420	4.07	2.00	n= 0.013 Corrugated PE, smooth interior
0.6	167	0.0120	4.97	3.90	Pipe Channel, 5 to 6 12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
					n= 0.013 Corrugated PE, smooth interior
0.4	70	0.0090	2.60	3.19	
0.4	70	0.0030	2.00	5.13	15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.025 Corrugated metal
0.2	140	0.1510	12.01	21.23	· · · · · · · · · · · · · · · · · · ·
· · -		01.10.10		0	18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'
					n= 0.025 Corrugated metal
19.1	999	Total			

1 age 1

Subcatchment 2B: Drainage Area 2B



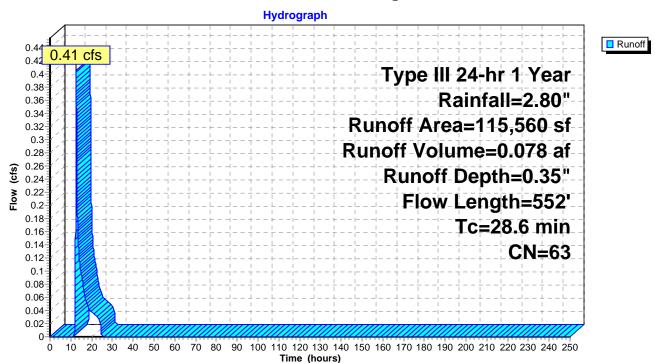
Summary for Subcatchment 3: Drainage Area 3

Runoff = 0.41 cfs @ 12.55 hrs, Volume= 0.078 af, Depth= 0.35"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 1 Year Rainfall=2.80"

	Α	rea (sf)	CN E	Description		
		49,852	61 >	75% Gras	s cover, Go	ood, HSG B
		56,494	60 V	Voods, Fai	r, HSG B	
		9,214	98 F	Paved road	s w/curbs &	& sewers
	1	15,560	63 V	Veighted A	verage	
	1	06,346	9	2.03% Per	vious Area	
		9,214	7	'.97% Impe	ervious Area	a
	Tc	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	19.4	118	0.0320	0.10		Sheet Flow, 1 to 2
						Woods: Light underbrush n= 0.400 P2= 3.50"
	2.5	87	0.0140	0.59		Shallow Concentrated Flow, 2 to 3
						Woodland Kv= 5.0 fps
	1.2	159	0.1940	2.20		Shallow Concentrated Flow, 3 to 4
						Woodland Kv= 5.0 fps
	5.5	188	0.0130	0.57		Shallow Concentrated Flow, 4 to DP 3
_						Woodland Kv= 5.0 fps
	28.6	552	Total			

Subcatchment 3: Drainage Area 3



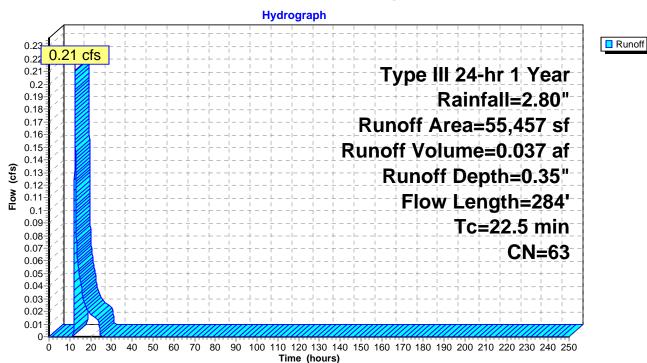
Summary for Subcatchment 4: Drainage Area 4

Runoff = 0.21 cfs @ 12.47 hrs, Volume= 0.037 af, Depth= 0.35"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 1 Year Rainfall=2.80"

_	Α	rea (sf)	CN	Description		
		31,781	60	Woods, Fai	r, HSG B	
		4,042	98	Paved road	s w/curbs &	& sewers
_		19,634	61	>75% Gras	s cover, Go	ood, HSG B
		55,457	63	Weighted A	verage	
		51,415		92.71% Per	rvious Area	
		4,042		7.29% Impe	ervious Are	a
_	Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	Description
	20.9	207	0.0860	0.16		Sheet Flow, 1 to 2
	1.6	77	0.0130	0.80		Grass: Bermuda n= 0.410 P2= 3.50" Shallow Concentrated Flow, 2 to DP 4 Short Grass Pasture Kv= 7.0 fps
	22.5	284	Total			

Subcatchment 4: Drainage Area 4



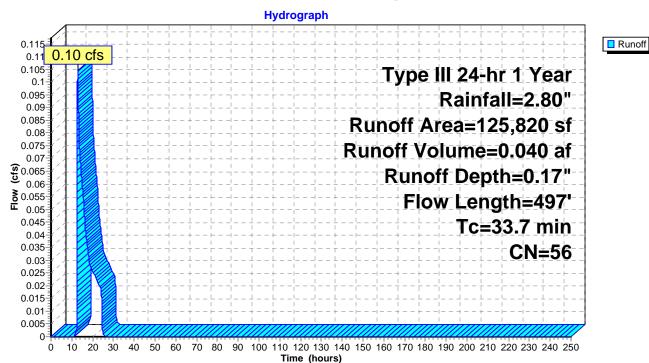
Summary for Subcatchment 5: Drainage Area 5

Runoff = 0.10 cfs @ 12.84 hrs, Volume= 0.040 af, Depth= 0.17"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 1 Year Rainfall=2.80"

_	Α	rea (sf)	CN I	Description		
		23,735	61 :	>75% Gras	s cover, Go	ood, HSG B
	1	01,385	55 \	Noods, Go	od, HSG B	
_		700	98 I	Paved park	ing, HSG B	
_	1	25,820	56 \	Neighted A	verage	
	1	25,120	(99.44% Pei	rvious Area	
		700	(0.56% Impe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description
	29.9	135	0.0150	0.08		Sheet Flow, 1 to 2
	3.8	362	0.1020	1.60		Grass: Bermuda n= 0.410 P2= 3.50" Shallow Concentrated Flow, 2 to DP 4 Woodland Kv= 5.0 fps
Ī	33.7	497	Total			

Subcatchment 5: Drainage Area 5



Page 16

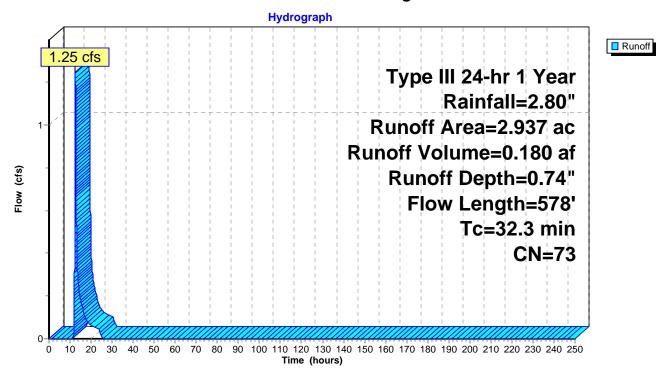
Summary for Subcatchment 6A: Drainage Area 6

Runoff = 1.25 cfs @ 12.52 hrs, Volume= 0.180 af, Depth= 0.74"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 1 Year Rainfall=2.80"

Area	(ac) C	N Desc	cription		
0.	.893 5	55 Woo	ds, Good,	HSG B	
1.	.109 9	8 Pave	ed roads w	/curbs & se	ewers
0.	.935 6	51 >75%	% Grass co	over, Good,	, HSG B
2.	.937 7	'3 Weig	hted Aver	age	
1.	.828	62.2	4% Pervio	us Area	
1.	109	37.7	6% Imperv	ious Area	
			•		
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
21.6	185	0.0600	0.14		Sheet Flow, 1 to 2
					Woods: Light underbrush n= 0.400 P2= 3.50"
0.5	214	0.1180	6.97		Shallow Concentrated Flow, 2 to 3
					Paved Kv= 20.3 fps
0.0	17	0.0120	6.25	7.67	Pipe Channel, 3 to 4 (Catchbasins)
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.012 HDPE
0.4	132	0.0077	5.22	9.22	· · · · · · · · · · · · · · · · · · ·
					18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'
					n= 0.013 HDPE
9.8	30	0.0120	0.05		Sheet Flow, 5 to DP 6
					Grass: Bermuda n= 0.410 P2= 3.50"
32.3	578	Total			

Subcatchment 6A: Drainage Area 6



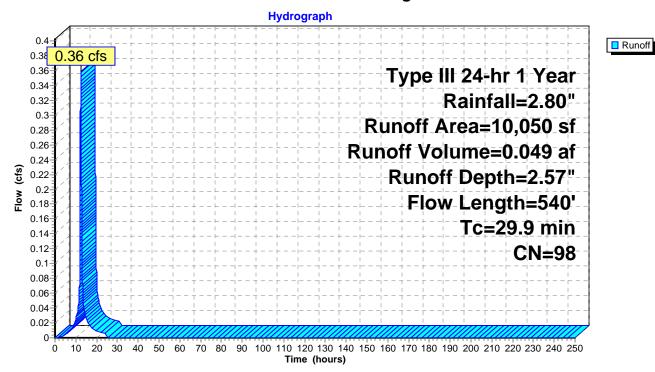
Summary for Subcatchment 6B: Drainage Area 6

Runoff = 0.36 cfs @ 12.39 hrs, Volume= 0.049 af, Depth= 2.57"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 1 Year Rainfall=2.80"

	^	"" (°t)	CN1 1			
_	A	rea (sf)	CN [Description		
		10,050	98 F	Paved road	s w/curbs 8	R sewers
		10,050	•	100.00% In	npervious A	ırea
	Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description
	21.6	185	0.0600	0.14		Sheet Flow, 1 to 2
						Woods: Light underbrush n= 0.400 P2= 3.50"
	0.5	214	0.1180	6.97		Shallow Concentrated Flow, 2 to 3
						Paved Kv= 20.3 fps
	0.0	17	0.0120	6.25	7.67	
						15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
						n= 0.012 HDPE
	0.1	101	0.1730	24.72	43.69	Pipe Channel, 4 to 5 (18" Culvert)
						18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'
						n= 0.013 HDPE
	7.7	23	0.0130	0.05		Sheet Flow, 5 to DP 6
						Grass: Bermuda n= 0.410 P2= 3.50"
	29.9	540	Total			

Subcatchment 6B: Drainage Area 6



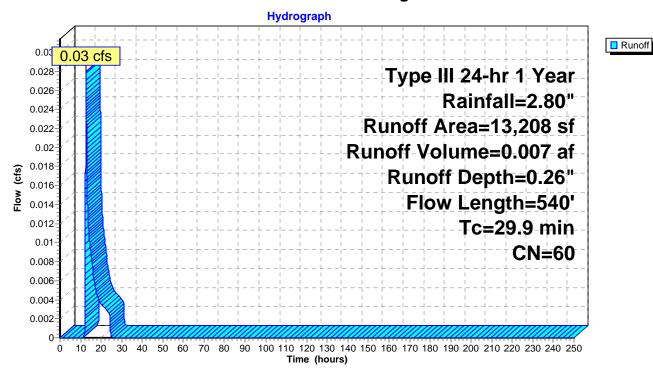
Summary for Subcatchment 6c: Drainage Area 6

Runoff = 0.03 cfs @ 12.65 hrs, Volume= 0.007 af, Depth= 0.26"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 1 Year Rainfall=2.80"

_	Α	rea (sf)	CN	Description		
Ī		13,208	60	Woods, Fai	r, HSG B	
		13,208		100.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	•	Capacity (cfs)	Description
	21.6	185	0.0600	0.14		Sheet Flow, 1 to 2 Woods: Light underbrush n= 0.400 P2= 3.50"
	0.5	214	0.1180	6.97		Shallow Concentrated Flow, 2 to 3 Paved Kv= 20.3 fps
	0.0	17	0.0120	6.25	7.67	Pipe Channel, 3 to 4 (Catchbasins) 15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
	0.1	101	0.1730	24.72	43.69	n= 0.012 HDPE Pipe Channel, 4 to 5 (18" Culvert) 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'
_	7.7	23	0.0130	0.05		n= 0.013 HDPE Sheet Flow, 5 to DP 6 Grass: Bermuda n= 0.410 P2= 3.50"
	29.9	540	Total			

Subcatchment 6c: Drainage Area 6



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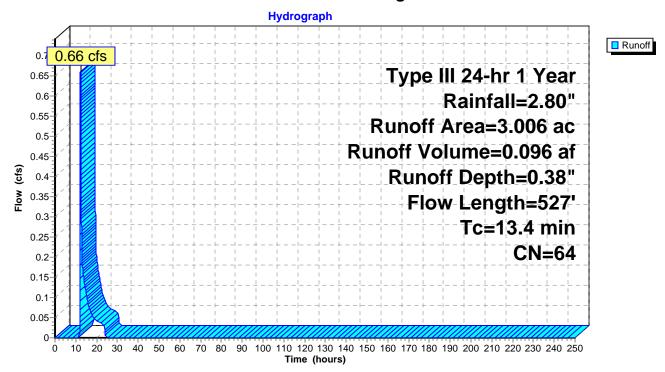
Summary for Subcatchment 7: Drainage Area 7

Runoff = 0.66 cfs @ 12.26 hrs, Volume= 0.096 af, Depth= 0.38"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 1 Year Rainfall=2.80"

Area	(ac) C	N Desc	cription		
	· /		ds, Fair, F	ISC P	
0.	291 9	98 Pave	ed roads w	/curbs & se	ewers
1.	375 6	51 >759	% Grass c	over, Good	, HSG B
3.	006 6	64 Wei	ghted Aver	age	
2	715	•	2% Pervio	•	
	291		% Impervi		
0.	231	3.00	70 IIIIpei vi	ous Alea	
Тс	Longth	Slope	Volocity	Capacity	Description
	Length	•	Velocity		Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
10.1	117	0.1620	0.19		Sheet Flow, 1 to 2
					Woods: Light underbrush n= 0.400 P2= 3.50"
0.7	100	0.2300	2.40		Shallow Concentrated Flow, 2 to 3
0.7	100	0.2300	2.40		•
4.0	404	0.0040	0.04		Woodland Kv= 5.0 fps
1.2	164	0.2010	2.24		Shallow Concentrated Flow, 3 to 4
					Woodland Kv= 5.0 fps
1.4	146	0.1230	1.75		Shallow Concentrated Flow, 3 to DP 7
					Woodland Kv= 5.0 fps
12.4	F07	Total			Troduction Itt of the
13.4	527	Total			

Subcatchment 7: Drainage Area 7



Page 21

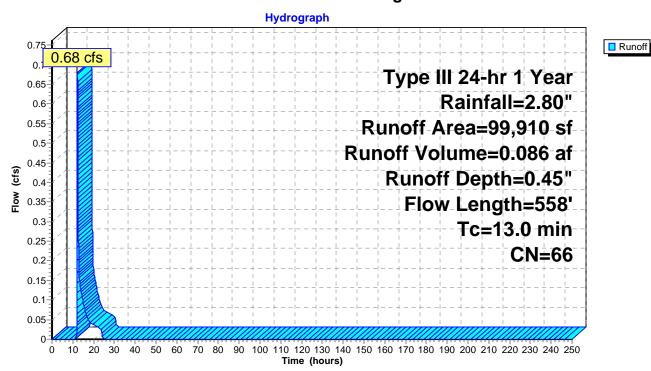
Summary for Subcatchment 8: Drainage Area 8

Runoff = 0.68 cfs @ 12.23 hrs, Volume= 0.086 af, Depth= 0.45"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 1 Year Rainfall=2.80"

	Area (sf)	CN E	escription		
	41,570	60 V	Voods, Fai	r, HSG B	
	15,772	98 F	aved road	s w/curbs &	& sewers
	42,568	61 >	75% Gras	s cover, Go	ood, HSG B
	99,910		Veighted A	•	
	84,138	_		vious Area	
	15,772	1	5.79% lmp	ervious Ar	ea
To	Longth	Slope	\/olooit\/	Capacity	Description
Tc (min)	Length (feet)	(ft/ft)	Velocity (ft/sec)	(cfs)	Description
9.7	100	0.1300	0.17	(013)	Sheet Flow, 1 to 2
3.1	100	0.1300	0.17		Woods: Light underbrush n= 0.400 P2= 3.50"
3.1	362	0.1550	1.97		Shallow Concentrated Flow, 2 to 3
0.1	002	0.1000	1.07		Woodland Kv= 5.0 fps
0.2	96	0.1150	6.88		Shallow Concentrated Flow, 3 to DP 8
					Paved Kv= 20.3 fps
13.0	558	Total			

Subcatchment 8: Drainage Area 8



Inflow
Outflow

Summary for Reach 1R: 18"

Inflow Area = 2.937 ac, 37.76% Impervious, Inflow Depth = 0.74" for 1 Year event

Inflow = 1.25 cfs @ 12.52 hrs, Volume= 0.180 af

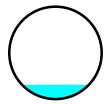
Outflow = 1.25 cfs @ 12.52 hrs, Volume= 0.180 af, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

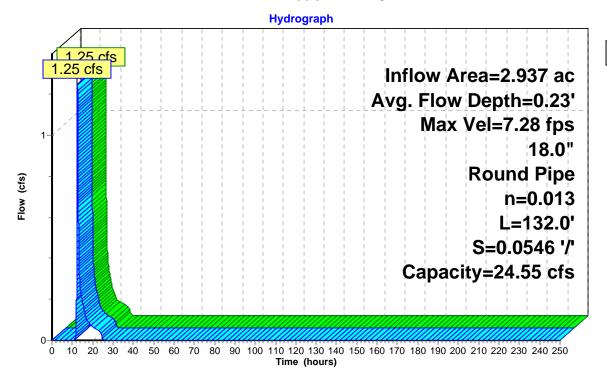
Max. Velocity= 7.28 fps, Min. Travel Time= 0.3 min Avg. Velocity = 3.38 fps, Avg. Travel Time= 0.7 min

Peak Storage= 23 cf @ 12.52 hrs Average Depth at Peak Storage= 0.23' Bank-Full Depth= 1.50', Capacity at Bank-Full= 24.55 cfs

18.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 132.0' Slope= 0.0546 '/' Inlet Invert= 374.01', Outlet Invert= 366.80'



Reach 1R: 18"



Summary for Pond 1P: Pond - D

Inflow Area =	2.937 ac, 37.76% Impervious, Inflow Do	epth = 0.74" for 1 Year event
Inflow =	1.25 cfs @ 12.52 hrs, Volume=	0.180 af
Outflow =	1.25 cfs @ 12.52 hrs, Volume=	0.180 af, Atten= 0%, Lag= 0.0 min
Primary =	1.25 cfs @ 12.52 hrs, Volume=	0.180 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

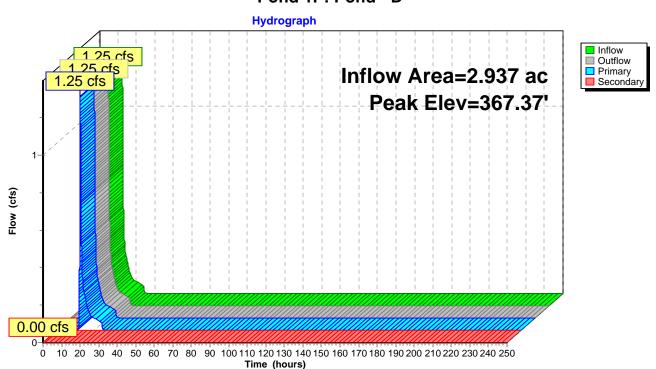
Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 367.37' @ 12.52 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	366.80'	18.0" Round Culvert
	•		L= 6.0' CMP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 366.80' / 361.00' S= 0.9667 '/' Cc= 0.900
			n= 0.025 Corrugated metal
#2	Secondary	371.19'	57.0" W x 57.0" H Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=1.24 cfs @ 12.52 hrs HW=367.37' (Free Discharge) 1=Culvert (Inlet Controls 1.24 cfs @ 2.03 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=366.80' (Free Discharge) 2=Orifice/Grate (Controls 0.00 cfs)

Pond 1P: Pond - D



Page 24

Summary for Pond 3P: Detention Pond

Inflow 0.00 cfs @ 0.00 hrs, Volume= 0.000 af 0.00 hrs. Volume= 0.000 af, Atten= 0%, Lag= 0.0 min Outflow 0.00 cfs @ = 0.00 hrs, Volume= Primary 0.00 cfs @ 0.000 af 0.00 hrs, Volume= 0.000 af Secondary = 0.00 cfs @

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 363.00' @ 0.00 hrs Surf.Area= 0 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: initial storage excedes outflow)

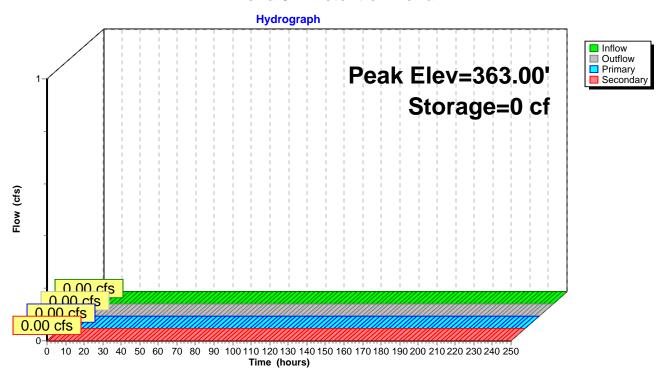
Center-of-Mass det. time= (not calculated: no inflow)

Volume	Invert	Avail.Sto	rage Storage	Description	
#1	363.00'	39,4	43 cf Custom	Stage Data (Pr	rismatic)Listed below (Recalc)
Elevation (feet		urf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
363.00)	0	0	0	
364.00)	1,137	569	569	
366.00)	2,172	3,309	3,878	
368.00)	3,441	5,613	9,491	
370.00		5,233	8,674	18,165	
372.00)	7,184	12,417	30,582	
373.00)	7,500	7,342	37,924	
373.20)	7,700	1,520	39,443	
Device	Routing	Invert	Outlet Device	S	
	Primary Secondary	363.00' 373.00'	208.0' long x Head (feet) 0	.20 0.40 0.60	r Surface area Broad-Crested Rectangular Weir 0.80 1.00 1.20 1.40 1.60 70 2.69 2.68 2.69 2.67 2.64

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=363.00' (Free Discharge) 1=Exfiltration (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=363.00' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 3P: Detention Pond



Summary for Pond 4P: Detention Pond

Inflow Area = 5.751 ac, 33.25% Impervious, Inflow Depth = 0.74" for 1 Year event Inflow 3.05 cfs @ 12.29 hrs. Volume= 0.353 af 0.44 cfs @ 14.06 hrs, Volume= Outflow 0.353 af, Atten= 86%, Lag= 106.3 min 0.44 cfs @ 14.06 hrs, Volume= Primary = 0.353 af 0.00 cfs @ 0.00 hrs, Volume= Secondary = 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 369.34' @ 14.06 hrs Surf.Area= 1,886 sf Storage= 6,164 cf

Plug-Flow detention time= 184.5 min calculated for 0.353 af (100% of inflow)

Center-of-Mass det. time= 184.5 min (1,071.3 - 886.9)

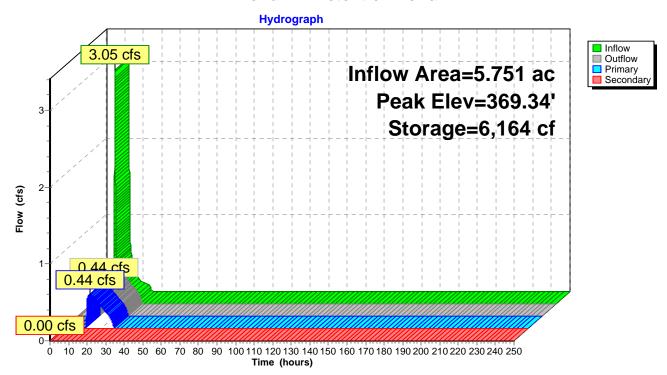
v (Recalc)
ngular Weir
0 1.60 1.80 2.00
2.92 3.07 3.07
1

Primary OutFlow Max=0.44 cfs @ 14.06 hrs HW=369.34' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.44 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=363.00' (Free Discharge)

⁻³⁼Custom Weir/Orifice (Controls 0.00 cfs)
-2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 4P: Detention Pond



Post-Development3

Type III 24-hr 1 Year Rainfall=2.80" Printed 11/1/2010

Prepared by Petruccelli Engineering

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Page 28

Summary for Pond 6P: Drywells

Inflow Area =	0.231 ac,100.00% Impervious, Inflow D	epth = 2.57" for 1 Year event
Inflow =	0.36 cfs @ 12.39 hrs, Volume=	0.049 af
Outflow =	0.18 cfs @ 12.79 hrs, Volume=	0.049 af, Atten= 49%, Lag= 23.9 min
Primary =	0.18 cfs @ 12.79 hrs, Volume=	0.049 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 386.63' @ 12.79 hrs Surf.Area= 1,218 sf Storage= 309 cf

Plug-Flow detention time= 10.6 min calculated for 0.049 af (100% of inflow) Center-of-Mass det. time= 10.6 min (792.1 - 781.5)

Volume	Invert	Avail.Storage	Storage Description
#1A	386.00'	1,291 cf	27.68'W x 44.00'L x 4.83'H Field A
			5,883 cf Overall - 2,655 cf Embedded = 3,227 cf x 40.0% Voids
#2A	387.00'	2,058 cf	Dry_Well 1000 Gallon x 16 Inside #1
			Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf
			Outside= 68.0"W x 34.0"H => 15.81 sf x 10.50'L = 166.0 cf
		3,349 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	386.00'	6.000 in/hr Exfiltration over Wetted area
#2	Secondary	393.00'	24.0" Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=0.18 cfs @ 12.79 hrs HW=386.63' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.18 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=386.00' (Free Discharge) 2=Orifice/Grate (Controls 0.00 cfs)

Pond 6P: Drywells - Chamber Wizard Field A

Chamber Model = Dry Well 1000 Gallon

Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf Outside= 68.0"W x 34.0"H => 15.81 sf x 10.50'L = 166.0 cf

68.0" Wide + 12.0" Spacing = 80.0" C-C

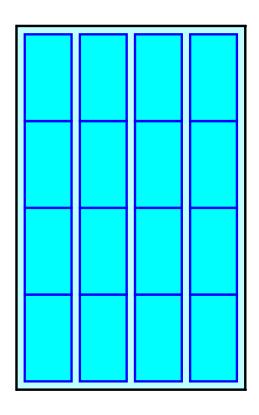
4 Chambers/Row x 10.50' Long = 42.00' + 12.0" End Stone x 2 = 44.00' Base Length 4 Rows x 68.0" Wide + 12.0" Spacing x 3 + 12.0" Side Stone x 2 = 27.68' Base Width 12.0" Base + 34.0" Chamber Height + 12.0" Cover = 4.83' Field Height

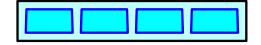
16 Chambers x 128.6 cf = 2,058.4 cf Chamber Storage 16 Chambers x 166.0 cf = 2,655.4 cf Displacement

5,882.6 cf Field - 2,655.4 cf Chambers = 3,227.2 cf Stone x 40.0% Voids = 1,290.9 cf Stone Storage

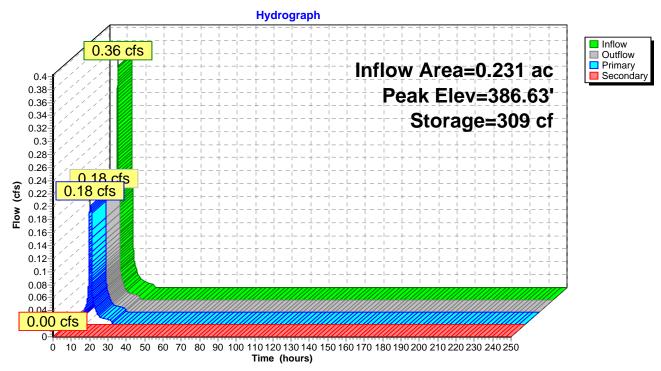
Stone + Chamber Storage = 3,349.3 cf = 0.077 af

16 Chambers 217.9 cy Field 119.5 cy Stone





Pond 6P: Drywells



Volume

Invert

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Page 31

Summary for Pond 7P: Stormwater Treatment Pond #2

Inflow Area = 2.937 ac, 37.76% Impervious, Inflow Depth = 0.74" for 1 Year event Inflow 1.25 cfs @ 12.52 hrs. Volume= 0.180 af 1.20 cfs @ 12.60 hrs, Volume= Outflow 0.180 af, Atten= 4%, Lag= 4.8 min 0.17 cfs @ 12.60 hrs, Volume= Primary 0.119 af Secondary = 1.03 cfs @ 12.60 hrs, Volume= 0.061 af 0.00 hrs, Volume= Tertiary 0.00 cfs @ 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 363.79' @ 12.60 hrs Surf.Area= 715 sf Storage= 1,109 cf

Plug-Flow detention time= 55.0 min calculated for 0.180 af (100% of inflow) Center-of-Mass det. time= 55.0 min (954.6 - 899.6)

Avail.Storage Storage Description

#1	361.00'	2,60	05 cf Custom	n Stage Data (Pi	rismatic)Listed below (Recalc)
Elevation	on Su	urf.Area	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
361.0	00	100	0	0	
362.0	00	300	200	200	
364.0	00	763	1,063	1,263	
365.0	00	1,132	948	2,211	
365.3	30	1,500	395	2,605	
Device	Routing	Invert	Outlet Device	es	
#1	Primary	361.00'	10.000 in/hr	Exfiltration over	r Surface area
#2	Secondary	363.50'	Custom Wei	r/Orifice, Cv= 2.	62 (C= 3.28)
			Head (feet) 0	0.00 1.50	
			Width (feet) 2		
#3	Tertiary	365.10'			oad-Crested Rectangular Weir
			, ,		0.80 1.00 1.20 1.40 1.60 1.80 2.00
				50 4.00 4.50	
			, ,	,	68 2.67 2.65 2.64 2.64 2.68 2.68
			2.72 2.81 2.	92 2.97 3.07 3	.32

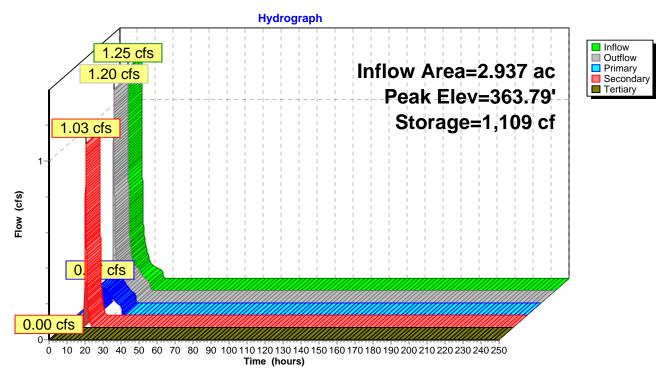
Primary OutFlow Max=0.17 cfs @ 12.60 hrs HW=363.79' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.17 cfs)

Secondary OutFlow Max=1.03 cfs @ 12.60 hrs HW=363.79' (Free Discharge) 2=Custom Weir/Orifice (Weir Controls 1.03 cfs @ 1.77 fps)

Tertiary OutFlow Max=0.00 cfs @ 0.00 hrs HW=361.00' (Free Discharge)

3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 7P: Stormwater Treatment Pond #2



Volume

Invert

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Page 33

Summary for Pond 8P: Stormwater Treatment Pond #1

Inflow 1.03 cfs @ 12.60 hrs, Volume= 0.061 af = 0.70 cfs @ 12.86 hrs. Volume= 0.061 af, Atten= 33%, Lag= 15.6 min Outflow = 0.21 cfs @ 12.86 hrs, Volume= Primary 0.032 af 0.49 cfs @ 12.86 hrs, Volume= Secondary = 0.029 af

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 362.12' @ 12.86 hrs Surf.Area= 897 sf Storage= 674 cf

Plug-Flow detention time= 17.9 min calculated for 0.061 af (100% of inflow) Center-of-Mass det. time= 17.9 min (805.2 - 787.3)

Avail.Storage Storage Description

volunie	IIIVEIL	Avaii.Siu	rage Storage	Description	
#1	361.00'	5,80	67 cf Custor	n Stage Data (P	rismatic)Listed below (Recalc)
Elevation		ırf.Area	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
361.0	00	300	0	0	
362.0	00	842	571	571	
364.0		1,772	2,614	3,185	
365.0		2,163	1,968	5,153	
365.3	30	2,600	714	5,867	
Device	Routing	Invert	Outlet Device	es	
#1	Secondary	361.00'	24.0" Round	d Culvert	
					conforming to fill, Ke= 0.500
					330.00' S= 0.1550 '/' Cc= 0.900
				rrugated PE, sm	
#2	Device 1	361.10'		ifice/Grate (0 yr	,
#3	Device 1	361.70'		ifice/Grate (1yr)	
#4	Device 1	362.30'		rifice/Grate(2yr)	
#5	Device 1	363.40'		ifice/Grate(10yr	
#6	Device 1	364.70'			Riser (100yr) C= 0.600
				eir flow at low hea	
#7	Device 1	365.00'			pad-Crested Rectangular Weir (14.5)
					0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3		04 0 00 0 00 0 70 0 77 0 00 0 00
					61 2.60 2.66 2.70 2.77 2.89 2.88
0	Davidsa 4	005 001	2.85 3.07 3		no al Occasio I Designation Mala
#8	Device 1	365.20'			road-Crested Rectangular Weir
					0.80 1.00 1.20 1.40 1.60 1.80 2.00
				.50 4.00 4.50 b) 2.44 2.58 2	60 267 265 264 264 269 269
			, •	,	68 2.67 2.65 2.64 2.64 2.68 2.68
#0	Drimon	261.00	_	.92 2.97 3.07 3	
#9	Primary	361.00'	าบ.บบบ เท/ทา	Exfiltration ove	r Surrace area

-8=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Primary OutFlow Max=0.21 cfs @ 12.86 hrs HW=362.12' (Free Discharge) 9=Exfiltration (Exfiltration Controls 0.21 cfs)

Secondary OutFlow Max=0.49 cfs @ 12.86 hrs HW=362.12' (Free Discharge)

1=Culvert (Passes 0.49 cfs of 6.51 cfs potential flow)

-2=Orifice/Grate (0 yr) (Orifice Controls 0.10 cfs @ 4.66 fps)

-3=Orifice/Grate (1yr) (Orifice Controls 0.39 cfs @ 2.20 fps)

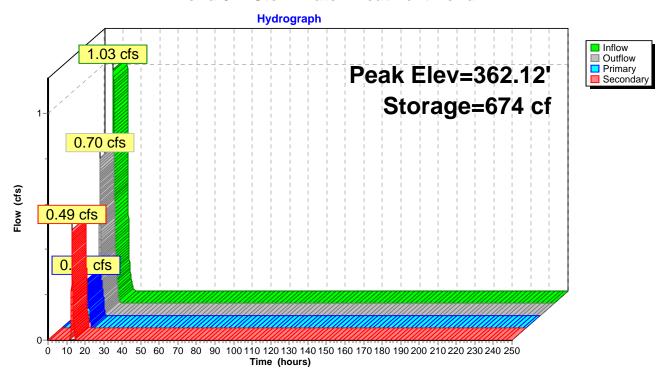
-4=Orifice/Grate(2yr) (Controls 0.00 cfs)

-5=Orifice/Grate(10yr) (Controls 0.00 cfs)

-6=Top of Riser (100yr) (Controls 0.00 cfs)

-7=Broad-Crested Rectangular Weir (14.5) (Controls 0.00 cfs)

Pond 8P: Stormwater Treatment Pond #1



Post-Development3

Type III 24-hr 1 Year Rainfall=2.80" Printed 11/1/2010

Prepared by Petruccelli Engineering
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Page 35

Summary for Pond 9P: Drywells

Inflow Area =	0.459 ac,100.00% Impervious, Inflow De	epth = 2.57" for 1 Year event
Inflow =	0.86 cfs @ 12.25 hrs, Volume=	0.098 af
Outflow =	0.25 cfs @ 11.93 hrs, Volume=	0.098 af, Atten= 71%, Lag= 0.0 min
Primary =	0.25 cfs @ 11.93 hrs, Volume=	0.098 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 387.14' @ 12.73 hrs Surf.Area= 1,834 sf Storage= 950 cf

Plug-Flow detention time= 20.9 min calculated for 0.098 af (100% of inflow) Center-of-Mass det. time= 20.9 min (792.4 - 771.4)

Volume	Invert	Avail.Storage	Storage Description
#1A	386.00'	1,685 cf	41.69'W x 44.00'L x 4.83'H Field A
			8,860 cf Overall - 4,647 cf Embedded = 4,213 cf \times 40.0% Voids
#2A	387.00'	3,602 cf	Dry_Well 1000 Gallon x 28 Inside #1
			Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf
			Outside= 68.0"W x 34.0"H => 15.81 sf x 10.50'L = 166.0 cf
		5,287 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	386.00'	6.000 in/hr Exfiltration over Surface area
#2	Secondary	392.00'	24.0" Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=0.25 cfs @ 11.93 hrs HW=386.08' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.25 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=386.00' (Free Discharge) 2=Orifice/Grate (Controls 0.00 cfs)

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Pond 9P: Drywells - Chamber Wizard Field A

Chamber Model = Dry_Well 1000 Gallon

Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf Outside= 68.0"W x 34.0"H => 15.81 sf x 10.50'L = 166.0 cf

68.0" Wide + 0.0" Spacing = 68.0" C-C

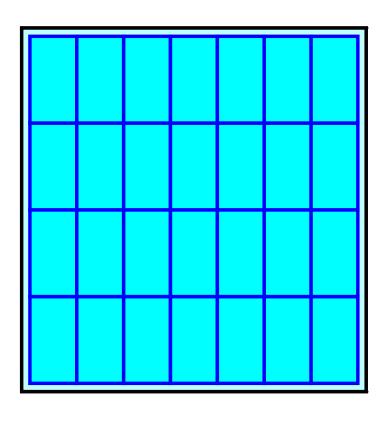
4 Chambers/Row x 10.50' Long = 42.00' + 12.0'' End Stone x 2 = 44.00' Base Length 7 Rows x 68.0'' Wide + 12.0'' Side Stone x 2 = 41.69' Base Width 12.0'' Base + 34.0'' Chamber Height + 12.0'' Cover = 4.83' Field Height

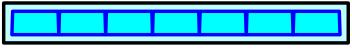
28 Chambers x 128.6 cf = 3,602.2 cf Chamber Storage 28 Chambers x 166.0 cf = 4,646.9 cf Displacement

8,860.0 cf Field - 4,646.9 cf Chambers = 4,213.1 cf Stone x 40.0% Voids = 1,685.2 cf Stone Storage

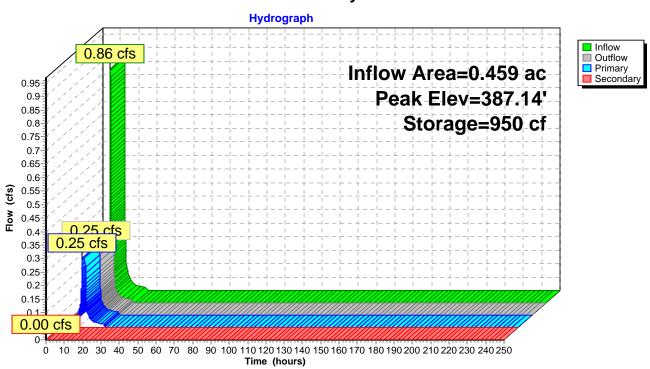
Stone + Chamber Storage = 5,287.4 cf = 0.121 af

28 Chambers 328.1 cy Field 156.0 cy Stone





Pond 9P: Drywells



Summary for Link DP-1: Design Point 1

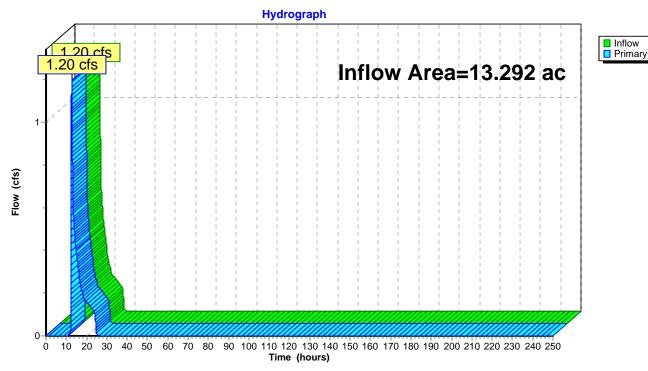
Inflow Area = 13.292 ac, 3.27% Impervious, Inflow Depth = 0.26" for 1 Year event

Inflow = 1.20 cfs @ 12.68 hrs, Volume= 0.293 af

Primary = 1.20 cfs @ 12.68 hrs, Volume= 0.293 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-1: Design Point 1



Summary for Link DP-2: Design Point 2

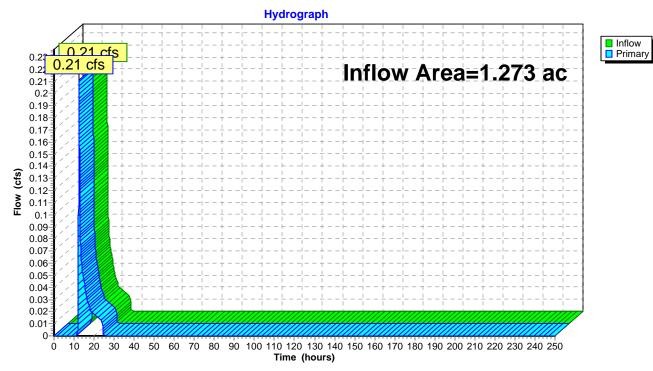
Inflow Area = 1.273 ac, 7.29% Impervious, Inflow Depth = 0.35" for 1 Year event

Inflow = 0.21 cfs @ 12.47 hrs, Volume= 0.037 af

Primary = 0.21 cfs @ 12.47 hrs, Volume= 0.037 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-2: Design Point 2



Summary for Link DP-3: Design Point 3

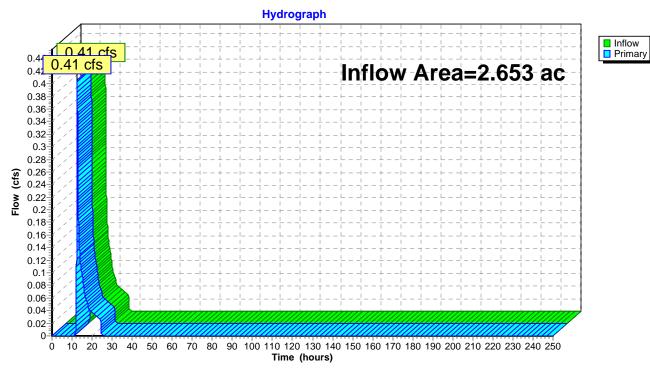
Inflow Area = 2.653 ac, 7.97% Impervious, Inflow Depth = 0.35" for 1 Year event

Inflow = 0.41 cfs @ 12.55 hrs, Volume= 0.078 af

Primary = 0.41 cfs @ 12.55 hrs, Volume= 0.078 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-3: Design Point 3



Page 41

Summary for Link DP-4: Design Point 4

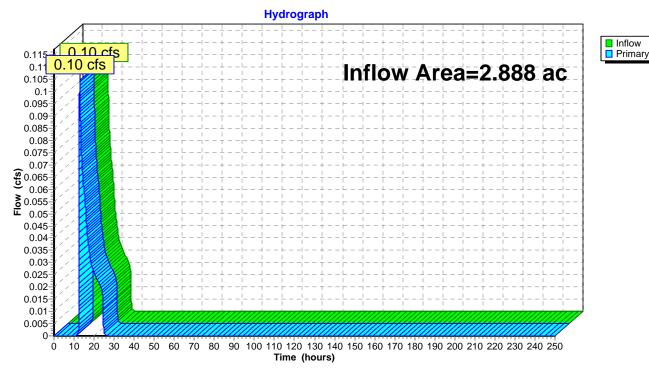
Inflow Area = 2.888 ac, 0.56% Impervious, Inflow Depth = 0.17" for 1 Year event

Inflow = 0.10 cfs @ 12.84 hrs, Volume= 0.040 af

Primary = 0.10 cfs @ 12.84 hrs, Volume= 0.040 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-4: Design Point 4



Summary for Link DP-5: Design Point 5

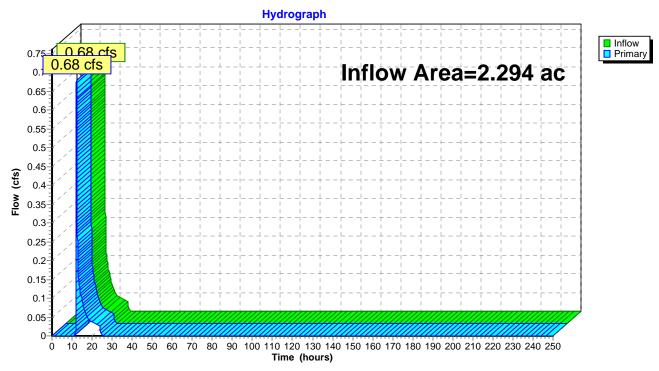
Inflow Area = 2.294 ac, 15.79% Impervious, Inflow Depth = 0.45" for 1 Year event

Inflow = 0.68 cfs @ 12.23 hrs, Volume= 0.086 af

Primary = 0.68 cfs @ 12.23 hrs, Volume= 0.086 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-5: Design Point 5



Summary for Link DP-6: Design Point 6

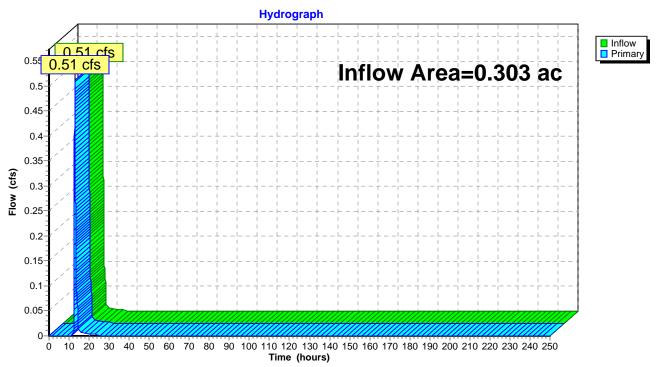
Inflow Area = 0.303 ac, 0.00% Impervious, Inflow Depth = 1.41" for 1 Year event

Inflow = 0.51 cfs @ 12.86 hrs, Volume= 0.036 af

Primary = 0.51 cfs @ 12.86 hrs, Volume= 0.036 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-6: Design Point 6



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Summary for Link DP-7: Design Point 7

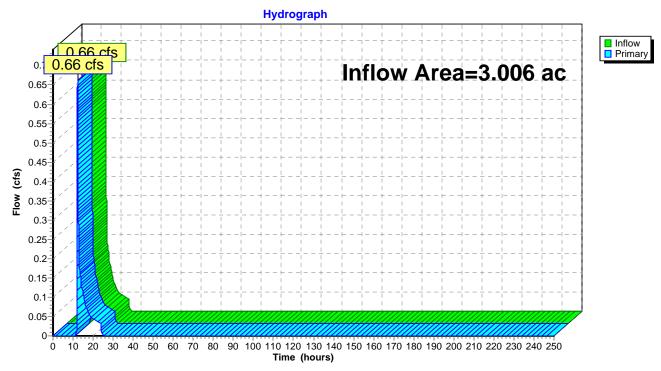
Inflow Area = 3.006 ac, 9.68% Impervious, Inflow Depth = 0.38" for 1 Year event

Inflow = 0.66 cfs @ 12.26 hrs, Volume= 0.096 af

Primary = 0.66 cfs @ 12.26 hrs, Volume= 0.096 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-7: Design Point 7



Time span=0.00-250.00 hrs, dt=0.01 hrs, 25001 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment 1: Drainage Area 1	Runoff Area=13.292 ac 3.27% Impervious Runoff Depth=0.53"
	Flow Length=1,455' Tc=31.8 min CN=60 Runoff=3.27 cfs 0.589 af
Subcatchment 2A: Drainage Area 2A	Runoff Area=5.751 ac 33.25% Impervious Runoff Depth=1.18" Flow Length=999' Tc=19.1 min CN=73 Runoff=5.18 cfs 0.565 af
Subcatchment 2B: Drainage Area 2B	Runoff Area=19,994 sf 100.00% Impervious Runoff Depth=3.27" Flow Length=999' Tc=19.1 min CN=98 Runoff=1.09 cfs 0.125 af
Subcatchment3: Drainage Area 3	Runoff Area=115,560 sf 7.97% Impervious Runoff Depth=0.66" Flow Length=552' Tc=28.6 min CN=63 Runoff=0.94 cfs 0.146 af
Subcatchment 4: Drainage Area 4	Runoff Area=55,457 sf 7.29% Impervious Runoff Depth=0.66" Flow Length=284' Tc=22.5 min CN=63 Runoff=0.50 cfs 0.070 af
Subcatchment 5: Drainage Area 5	Runoff Area=125,820 sf 0.56% Impervious Runoff Depth=0.38" Flow Length=497' Tc=33.7 min CN=56 Runoff=0.40 cfs 0.091 af
Subcatchment 6A: Drainage Area 6	Runoff Area=2.937 ac 37.76% Impervious Runoff Depth=1.18" Flow Length=578' Tc=32.3 min CN=73 Runoff=2.11 cfs 0.289 af
Subcatchment 6B: Drainage Area 6	Runoff Area=10,050 sf 100.00% Impervious Runoff Depth=3.27" Flow Length=540' Tc=29.9 min CN=98 Runoff=0.45 cfs 0.063 af
Subcatchment6c: Drainage Area 6	Runoff Area=13,208 sf 0.00% Impervious Runoff Depth=0.53" Flow Length=540' Tc=29.9 min CN=60 Runoff=0.08 cfs 0.013 af
Subcatchment 7: Drainage Area 7	Runoff Area=3.006 ac 9.68% Impervious Runoff Depth=0.71" Flow Length=527' Tc=13.4 min CN=64 Runoff=1.57 cfs 0.177 af
Subcatchment8: Drainage Area 8	Runoff Area=99,910 sf 15.79% Impervious Runoff Depth=0.80" Flow Length=558' Tc=13.0 min CN=66 Runoff=1.45 cfs 0.153 af
Reach 1R: 18" 18.0" Round Pipe n=0.013	Avg. Flow Depth=0.30' Max Vel=8.50 fps Inflow=2.11 cfs 0.289 af L=132.0' S=0.0546 '/' Capacity=24.55 cfs Outflow=2.11 cfs 0.289 af
Pond 1P: Pond - D Primary=2.11	Peak Elev=367.56' Inflow=2.11 cfs 0.289 af cfs 0.289 af Secondary=0.00 cfs 0.000 af Outflow=2.11 cfs 0.289 af
Pond 3P: Detention Pond Primary=0.00	Peak Elev=363.00' Storage=0 cf Inflow=0.00 cfs 0.000 af cfs 0.000 af Secondary=0.00 cfs 0.000 af Outflow=0.00 cfs 0.000 af
Pond 4P: Detention Pond Primary=0.60	Peak Elev=371.59' Storage=11,206 cf Inflow=5.18 cfs 0.565 af cfs 0.565 af Secondary=0.00 cfs 0.000 af Outflow=0.60 cfs 0.565 af
Pond 6P: Drywells Primary=0.19	Peak Elev=387.01' Storage=496 cf Inflow=0.45 cfs 0.063 af cfs 0.063 af Secondary=0.00 cfs 0.000 af Outflow=0.19 cfs 0.063 af

Post-Deve	lopment3
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Type III 24-hr 2 Year Rainfall=3.50"

Prepared by Petruccelli Engineering
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Page 46

Pond 7P: Stormwater Treatment Pond #2 Peak Elev=363.94' Storage=1,219 cf Inflow=2.11 cfs 0.289 af Primary=0.17 cfs 0.144 af Secondary=1.93 cfs 0.144 af Tertiary=0.00 cfs 0.000 af Outflow=2.10 cfs 0.289 af

Pond 8P: Stormwater Treatment Pond #1 Peak Elev=362.66' Storage=1,231 cf Inflow=1.93 cfs 0.144 af Primary=0.27 cfs 0.054 af Secondary=1.28 cfs 0.090 af Outflow=1.55 cfs 0.144 af

Pond 9P: Drywells Peak Elev=387.44' Storage=1,403 cf Inflow=1.09 cfs 0.125 af

Primary=0.25 cfs 0.125 af Secondary=0.00 cfs 0.000 af Outflow=0.25 cfs 0.125 af

Link DP-1: Design Point 1 Inflow=3.27 cfs 0.589 af Primary=3.27 cfs 0.589 af

Link DP-2: Design Point 2 Inflow=0.50 cfs 0.070 af

Primary=0.50 cfs 0.070 af

Link DP-3: Design Point 3 Inflow=0.94 cfs 0.146 af

Primary=0.94 cfs 0.146 af

Link DP-4: Design Point 4 Inflow=0.40 cfs 0.091 af

Primary=0.40 cfs 0.091 af

Link DP-5: Design Point 5 Inflow=1.45 cfs 0.153 af

Primary=1.45 cfs 0.153 af

Link DP-6: Design Point 6 Inflow=1.35 cfs 0.103 af

Primary=1.35 cfs 0.103 af

Link DP-7: Design Point 7 Inflow=1.57 cfs 0.177 af

Primary=1.57 cfs 0.177 af

Total Runoff Area = 35.087 ac Runoff Volume = 2.281 af Average Runoff Depth = 0.78" 85.41% Pervious = 29.968 ac 14.59% Impervious = 5.119 ac

Page 47

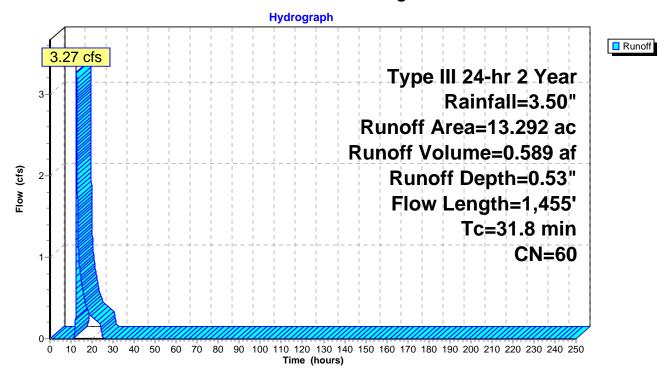
Summary for Subcatchment 1: Drainage Area 1

Runoff = 3.27 cfs @ 12.58 hrs, Volume= 0.589 af, Depth= 0.53"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 2 Year Rainfall=3.50"

Area	(ac) C	N Desc	cription					
2	2.770 69 50-75% Grass cover, Fair, HSG B							
0	0.435 98 Paved roads w/curbs & sewers							
			ds, Good,	HSG B				
			oads, HS					
			hted Aver					
	.857		3% Pervio	•				
	.435		% Impervi					
Ū	. 100	0.27	70 IIIIpoi VI	00071100				
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	2000.19.1011			
18.9	100	0.0260	0.09	(0.0)	Sheet Flow, 1 to 2			
10.5	100	0.0200	0.00		Grass: Bermuda n= 0.410 P2= 3.50"			
1.7	216	0.1830	2.14		Shallow Concentrated Flow, 2 to 3			
1.7	210	0.1000	2.17		Woodland Kv= 5.0 fps			
0.3	78	0.0450	4.31		Shallow Concentrated Flow, 3 to 4			
0.0	70	0.0400	4.01		Paved Kv= 20.3 fps			
1.2	121	0.1150	1.70		Shallow Concentrated Flow, 4 to 5			
		0.1100	1.70		Woodland Kv= 5.0 fps			
5.8	679	0.0770	1.94		Shallow Concentrated Flow, 5 to 6			
0.0	0.0	0.0770	1.01		Short Grass Pasture Kv= 7.0 fps			
3.9	261	0.0500	1.12		Shallow Concentrated Flow, 6 to DP1			
0.0		2.0000			Woodland Kv= 5.0 fps			
31.8	1,455	Total						

Subcatchment 1: Drainage Area 1



Page 49

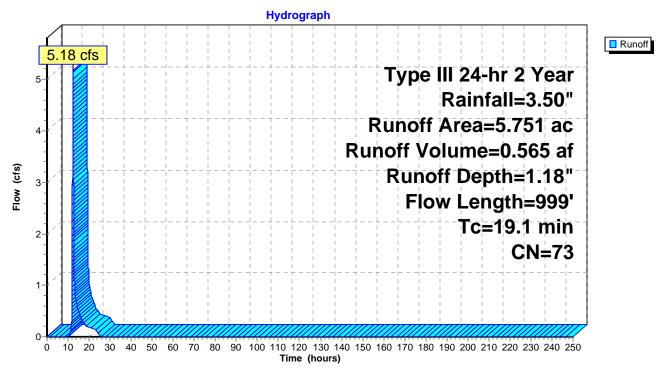
Summary for Subcatchment 2A: Drainage Area 2A

Runoff = 5.18 cfs @ 12.29 hrs, Volume= 0.565 af, Depth= 1.18"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 2 Year Rainfall=3.50"

Area	ı (ac) C	N Desc	cription						
1	1.912	98 Pave	ed roads w	/curbs & se	ewers				
				over, Good	, HSG B				
0.520 55 Woods, Good, HSG B									
5.751 73 Weighted Average									
3.839 66.75% Pervious Area									
1.912 33.25% Impervious Area									
То	Longth	Clone	\/olooity	Congoity	Description				
Tc (min)	-	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
11.5		0.0900	0.15	(613)	Shoot Flow 1 to 2				
11.5	100	0.0900	0.13		Sheet Flow, 1 to 2 Grass: Bermuda n= 0.410 P2= 3.50"				
5.8	423	0.0300	1.21		Shallow Concentrated Flow, 2 to 3				
0.0	720	0.0000	1.21		Short Grass Pasture Kv= 7.0 fps				
0.5	74	0.0140	2.40		Shallow Concentrated Flow, 3 to 4 (Road)				
			-		Paved Kv= 20.3 fps				
0.1	25	0.0100	4.54	3.56	•				
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'				
					n= 0.013 Corrugated PE, smooth interior				
0.6	167	0.0120	4.97	3.90					
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'				
				0.40	n= 0.013 Corrugated PE, smooth interior				
0.4	70	0.0090	2.60	3.19					
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'				
0.2	140	0.1510	12.01	21.23	n= 0.025 Corrugated metal Pipe Channel, 7 to 8				
0.2	140	0.1510	12.01	21.23	18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'				
					n= 0.025 Corrugated metal				
19.1	999	Total			11- 0.020 Corrugatou motar				
19.1	999	i Ulai							

Subcatchment 2A: Drainage Area 2A



Page 51

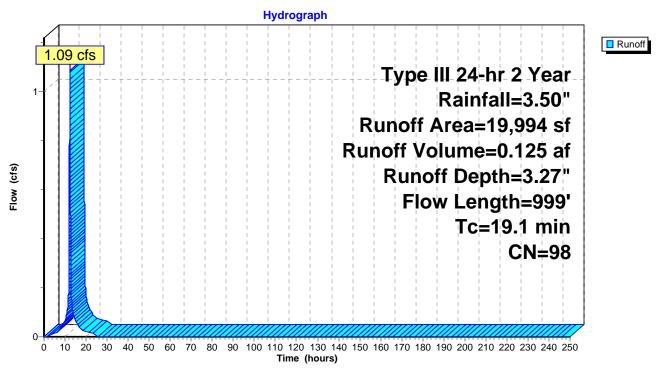
Summary for Subcatchment 2B: Drainage Area 2B

Runoff = 1.09 cfs @ 12.25 hrs, Volume= 0.125 af, Depth= 3.27"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 2 Year Rainfall=3.50"

A	rea (sf)	CN E	escription		
	19,994	98 F	aved road	s w/curbs &	& sewers
	19,994	1	00.00% lm	pervious A	rea
				•	
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
11.5	100	0.0900	0.15		Sheet Flow, 1 to 2
					Grass: Bermuda n= 0.410 P2= 3.50"
5.8	423	0.0300	1.21		Shallow Concentrated Flow, 2 to 3
					Short Grass Pasture Kv= 7.0 fps
0.5	74	0.0140	2.40		Shallow Concentrated Flow, 3 to 4 (Road)
		0.0400		0.50	Paved Kv= 20.3 fps
0.1	25	0.0100	4.54	3.56	
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
0.0	407	0.0400	4.07	2.00	n= 0.013 Corrugated PE, smooth interior
0.6	167	0.0120	4.97	3.90	Pipe Channel, 5 to 6 12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
0.4	70	0.0090	2.60	3.19	n= 0.013 Corrugated PE, smooth interior Pipe Channel, 6 to 7
0.4	70	0.0090	2.00	3.19	15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.025 Corrugated metal
0.2	140	0.1510	12.01	21.23	· · · · · · · · · · · · · · · · · · ·
0.2	170	0.1310	12.01	21.20	18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'
					n= 0.025 Corrugated metal
19.1	999	Total			II- 0.020 Corragatou motal
19.1	999	iolai			

Subcatchment 2B: Drainage Area 2B



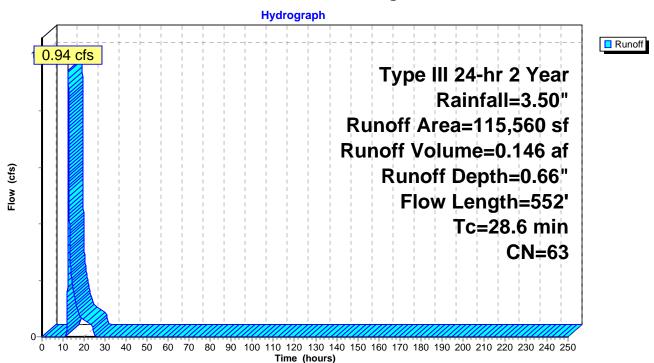
Summary for Subcatchment 3: Drainage Area 3

Runoff = 0.94 cfs @ 12.49 hrs, Volume= 0.146 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 2 Year Rainfall=3.50"

A	rea (sf)	CN E	escription		
	49,852	61 >	75% Gras	s cover, Go	ood, HSG B
	56,494	60 V	Voods, Fai	r, HSG B	
	9,214	98 F	Paved road	s w/curbs &	R sewers
1	15,560	63 V	Veighted A	verage	
1	06,346	9	2.03% Per	vious Area	
	9,214	7	".97% Impe	ervious Area	a
_					
Tc	Length	Slope	Velocity	Capacity	Description
(min)_	(feet)	(ft/ft)	(ft/sec)	(cfs)	
19.4	118	0.0320	0.10		Sheet Flow, 1 to 2
					Woods: Light underbrush n= 0.400 P2= 3.50"
2.5	87	0.0140	0.59		Shallow Concentrated Flow, 2 to 3
					Woodland Kv= 5.0 fps
1.2	159	0.1940	2.20		Shallow Concentrated Flow, 3 to 4
					Woodland Kv= 5.0 fps
5.5	188	0.0130	0.57		Shallow Concentrated Flow, 4 to DP 3
					Woodland Kv= 5.0 fps
28.6	552	Total			

Subcatchment 3: Drainage Area 3



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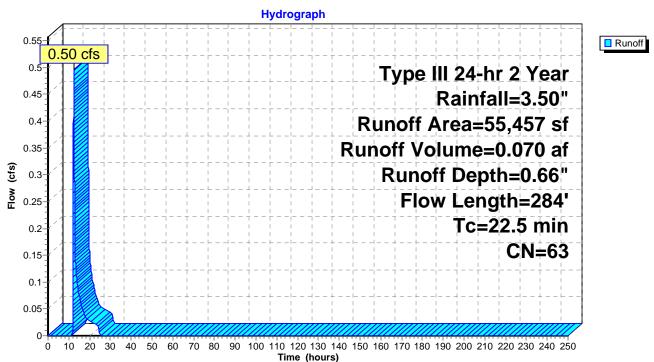
Summary for Subcatchment 4: Drainage Area 4

Runoff = 0.50 cfs @ 12.38 hrs, Volume= 0.070 af, Depth= 0.66"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 2 Year Rainfall=3.50"

	Α	rea (sf)	CN	Description		
		31,781	60	Woods, Fai	r, HSG B	
		4,042	98	Paved road	s w/curbs &	& sewers
_		19,634	61	>75% Gras	s cover, Go	ood, HSG B
		55,457	63	Weighted A	verage	
		51,415		92.71% Pei	rvious Area	
		4,042		7.29% Impe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description
_	20.9	207	0.0860	0.16	• ,	Sheet Flow, 1 to 2
_	1.6	77	0.0130	0.80		Grass: Bermuda n= 0.410 P2= 3.50" Shallow Concentrated Flow, 2 to DP 4 Short Grass Pasture Kv= 7.0 fps
	22.5	284	Total			

Subcatchment 4: Drainage Area 4



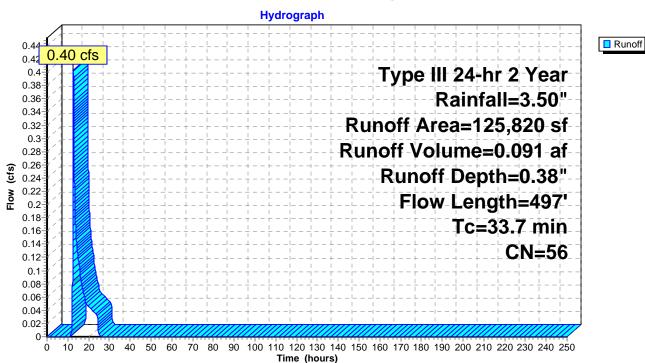
Summary for Subcatchment 5: Drainage Area 5

Runoff = 0.40 cfs @ 12.66 hrs, Volume= 0.091 af, Depth= 0.38"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 2 Year Rainfall=3.50"

_	Α	rea (sf)	CN I							
		23,735	61 :	>75% Gras	s cover, Go	ood, HSG B				
	1	01,385	55 \	Noods, Go	od, HSG B					
_		700	98 I	Paved park	ing, HSG B					
_	1	25,820	56 \	Neighted A	verage					
	1	25,120	(99.44% Pei	rvious Area					
		700	(0.56% Impe	ervious Are	a				
	Tc (min)	Length (feet)	Slope (ft/ft)	,	Capacity (cfs)	Description				
	29.9	135	0.0150	0.08		Sheet Flow, 1 to 2				
	3.8	362	0.1020	1.60		Grass: Bermuda n= 0.410 P2= 3.50" Shallow Concentrated Flow, 2 to DP 4 Woodland Kv= 5.0 fps				
Ī	33.7	497	Total							

Subcatchment 5: Drainage Area 5



Summary for Subcatchment 6A: Drainage Area 6

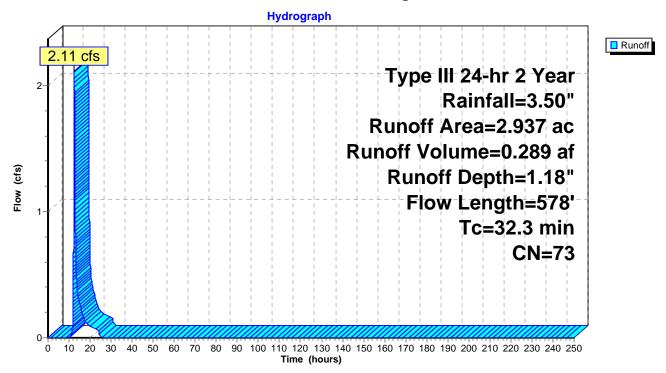
Runoff = 2.11 cfs @ 12.49 hrs, Volume= 0.289 af, Depth= 1.18"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 2 Year Rainfall=3.50"

Area	(ac) C	N Desc	cription						
0.	0.893 55 Woods, Good, HSG B								
1.	.109 9	8 Pave	ed roads w	/curbs & se	ewers				
0	.935 6	31 >75°	% Grass co	over, Good	, HSG B				
2.	.937 7	'3 Weig	ghted Aver	age					
1.	.828	62.2	4% Pervio	us Area					
1.	.109	37.7	6% Imperv	∕ious Area					
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
21.6	185	0.0600	0.14		Sheet Flow, 1 to 2				
					Woods: Light underbrush n= 0.400 P2= 3.50"				
0.5	214	0.1180	6.97		Shallow Concentrated Flow, 2 to 3				
					Paved Kv= 20.3 fps				
0.0	17	0.0120	6.25	7.67	Pipe Channel, 3 to 4 (Catchbasins)				
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'				
0.4	400	0.0077	5.00	0.00	n= 0.012 HDPE				
0.4	132	0.0077	5.22	9.22	Pipe Channel, 4 to 5 (18" Culvert)				
					18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'				
0.0	20	0.0400	0.05		n= 0.013 HDPE				
9.8	30	0.0120	0.05		Sheet Flow, 5 to DP 6				
		T			Grass: Bermuda n= 0.410 P2= 3.50"				
32.3	578	Total							

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Subcatchment 6A: Drainage Area 6



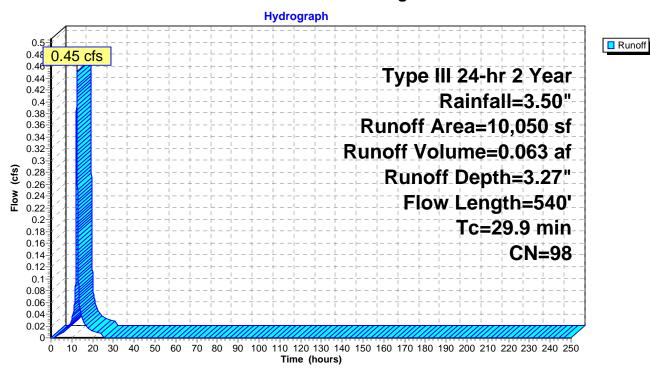
Summary for Subcatchment 6B: Drainage Area 6

Runoff = 0.45 cfs @ 12.39 hrs, Volume= 0.063 af, Depth= 3.27"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 2 Year Rainfall=3.50"

	^	"" (°t)	CN1 1			
_	A	rea (sf)	CN [Description		
		10,050	98 F	Paved road	s w/curbs 8	R sewers
		10,050	•	100.00% In	npervious A	ırea
	Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description
	21.6	185	0.0600	0.14		Sheet Flow, 1 to 2
						Woods: Light underbrush n= 0.400 P2= 3.50"
	0.5	214	0.1180	6.97		Shallow Concentrated Flow, 2 to 3
						Paved Kv= 20.3 fps
	0.0	17	0.0120	6.25	7.67	
						15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
						n= 0.012 HDPE
	0.1	101	0.1730	24.72	43.69	Pipe Channel, 4 to 5 (18" Culvert)
						18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'
						n= 0.013 HDPE
	7.7	23	0.0130	0.05		Sheet Flow, 5 to DP 6
						Grass: Bermuda n= 0.410 P2= 3.50"
	29.9	540	Total			

Subcatchment 6B: Drainage Area 6



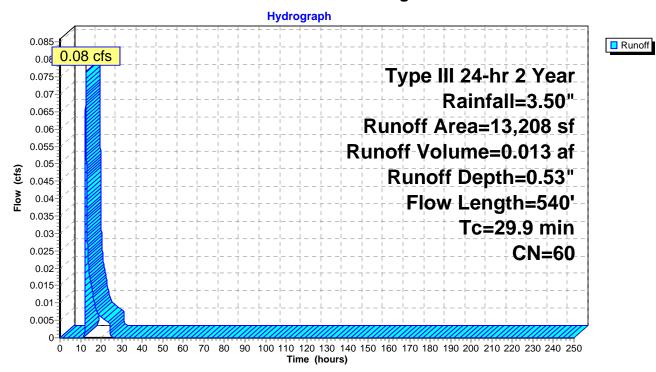
Summary for Subcatchment 6c: Drainage Area 6

Runoff = 0.08 cfs @ 12.53 hrs, Volume= 0.013 af, Depth= 0.53"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 2 Year Rainfall=3.50"

_	Α	rea (sf)	CN	Description		
		13,208	60	Woods, Fai	r, HSG B	
		13,208		100.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	•	Capacity (cfs)	Description
	21.6	185	0.0600	0.14		Sheet Flow, 1 to 2 Woods: Light underbrush n= 0.400 P2= 3.50"
	0.5	214	0.1180	6.97		Shallow Concentrated Flow, 2 to 3 Paved Kv= 20.3 fps
	0.0	17	0.0120	6.25	7.67	Pipe Channel, 3 to 4 (Catchbasins) 15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
	0.1	101	0.1730	24.72	43.69	n= 0.012 HDPE Pipe Channel, 4 to 5 (18" Culvert) 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'
_	7.7	23	0.0130	0.05		n= 0.013 HDPE Sheet Flow, 5 to DP 6 Grass: Bermuda n= 0.410 P2= 3.50"
	29.9	540	Total			

Subcatchment 6c: Drainage Area 6



Page 60

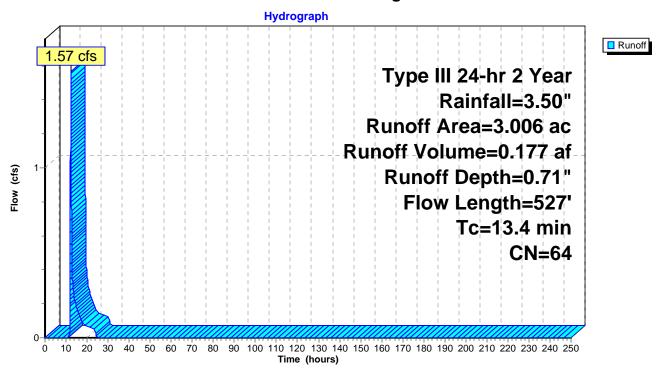
Summary for Subcatchment 7: Drainage Area 7

Runoff = 1.57 cfs @ 12.22 hrs, Volume= 0.177 af, Depth= 0.71"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 2 Year Rainfall=3.50"

Area	(ac) C	N Desc	cription					
				100 D				
	1.340 60 Woods, Fair, HSG B							
0.	0.291 98 Paved roads w/curbs & sewers							
1.	375 6	51 >759	% Grass co	over, Good	, HSG B			
3	006 6	64 Weid	ghted Aver	ane	<u> </u>			
	715	•	2% Pervio	•				
	_							
0.	291	9.68	% Impervi	ous Area				
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·			
10.1	117	0.1620	0.19		Sheet Flow, 1 to 2			
					Woods: Light underbrush n= 0.400 P2= 3.50"			
0.7	100	0.2300	2.40		Shallow Concentrated Flow, 2 to 3			
0.7	100	0.2300	2.40		•			
					Woodland Kv= 5.0 fps			
1.2	164	0.2010	2.24		Shallow Concentrated Flow, 3 to 4			
					Woodland Kv= 5.0 fps			
1.4	146	0.1230	1.75		Shallow Concentrated Flow, 3 to DP 7			
					Woodland Kv= 5.0 fps			
40.4		Tatal			1100 didira 111 – 0.0 ipo			
13.4	527	Total						

Subcatchment 7: Drainage Area 7



Page 61

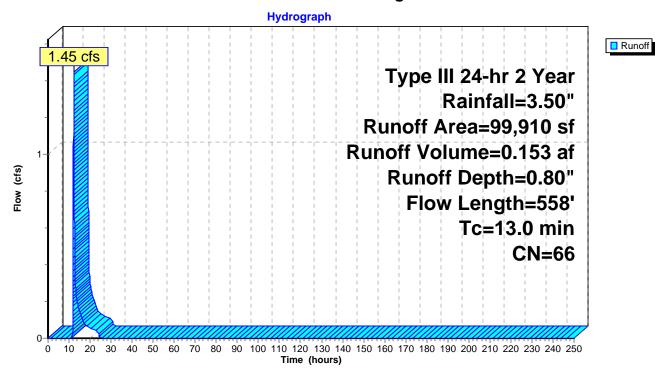
Summary for Subcatchment 8: Drainage Area 8

Runoff = 1.45 cfs @ 12.21 hrs, Volume= 0.153 af, Depth= 0.80"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 2 Year Rainfall=3.50"

A	rea (sf)	CN D	escription		
	41,570	60 V	Voods, Fai	r, HSG B	
	15,772	98 P	aved road	s w/curbs &	& sewers
	42,568	61 >	75% Gras	s cover, Go	ood, HSG B
	99,910	66 V	Veighted A	verage	
	84,138	8	4.21% Per	vious Area	
	15,772	1	5.79% lmp	ervious Ar	ea
_					
Tc	Length	Slope	Velocity	Capacity	Description
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	
9.7	100	0.1300	0.17		Sheet Flow, 1 to 2
					Woods: Light underbrush n= 0.400 P2= 3.50"
3.1	362	0.1550	1.97		Shallow Concentrated Flow, 2 to 3
					Woodland Kv= 5.0 fps
0.2	96	0.1150	6.88		Shallow Concentrated Flow, 3 to DP 8
					Paved Kv= 20.3 fps
13.0	558	Total			

Subcatchment 8: Drainage Area 8



Summary for Reach 1R: 18"

Inflow Area = 2.937 ac, 37.76% Impervious, Inflow Depth = 1.18" for 2 Year event

Inflow = 2.11 cfs @ 12.49 hrs, Volume= 0.289 af

Outflow = 2.11 cfs @ 12.49 hrs, Volume= 0.289 af, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

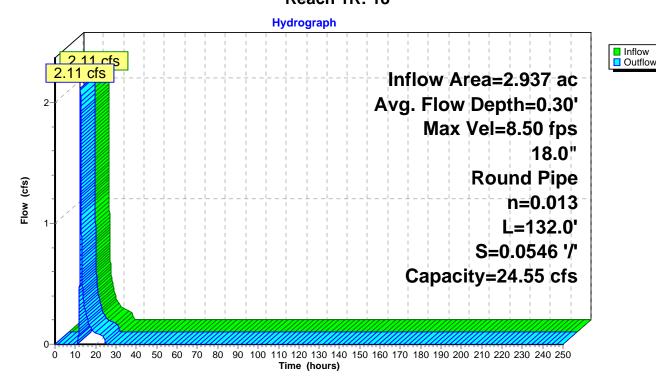
Max. Velocity= 8.50 fps, Min. Travel Time= 0.3 min Avg. Velocity = 3.75 fps, Avg. Travel Time= 0.6 min

Peak Storage= 33 cf @ 12.49 hrs Average Depth at Peak Storage= 0.30' Bank-Full Depth= 1.50', Capacity at Bank-Full= 24.55 cfs

18.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 132.0' Slope= 0.0546 '/' Inlet Invert= 374.01', Outlet Invert= 366.80'



Reach 1R: 18"



Page 63

Summary for Pond 1P: Pond - D

Inflow Area =	2.937 ac, 37.76% In	npervious, Inflow	Depth = 1.18"	for 2 Year event
Inflow =	2.11 cfs @ 12.49 hr	s, Volume=	0.289 af	
Outflow =	2.11 cfs @ 12.49 hr	s, Volume=	0.289 af, Att	en= 0%, Lag= 0.0 min
Primary =	2.11 cfs @ 12.49 hr	s, Volume=	0.289 af	_
Secondary =	0.00 cfs @ 0.00 hr	s, Volume=	0.000 af	

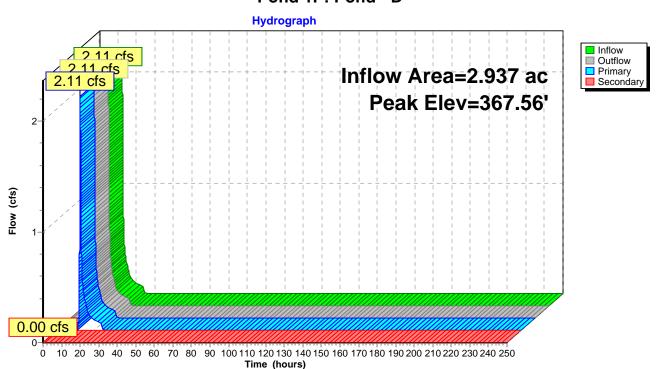
Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 367.56' @ 12.49 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	366.80'	18.0" Round Culvert
	•		L= 6.0' CMP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 366.80' / 361.00' S= 0.9667 '/' Cc= 0.900
			n= 0.025 Corrugated metal
#2	Secondary	371.19'	57.0" W x 57.0" H Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=2.11 cfs @ 12.49 hrs HW=367.56' (Free Discharge) 1=Culvert (Inlet Controls 2.11 cfs @ 2.34 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=366.80' (Free Discharge) 2=Orifice/Grate (Controls 0.00 cfs)

Pond 1P: Pond - D



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Page 64

Summary for Pond 3P: Detention Pond

Inflow 0.00 cfs @ 0.00 hrs, Volume= 0.000 af 0.00 hrs. Volume= 0.000 af, Atten= 0%, Lag= 0.0 min Outflow = 0.00 cfs @ 0.00 hrs, Volume= Primary 0.00 cfs @ 0.000 af 0.00 hrs, Volume= 0.000 af Secondary = 0.00 cfs @

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 363.00' @ 0.00 hrs Surf.Area= 0 sf Storage= 0 cf

Plug-Flow detention time= (not calculated: initial storage excedes outflow)

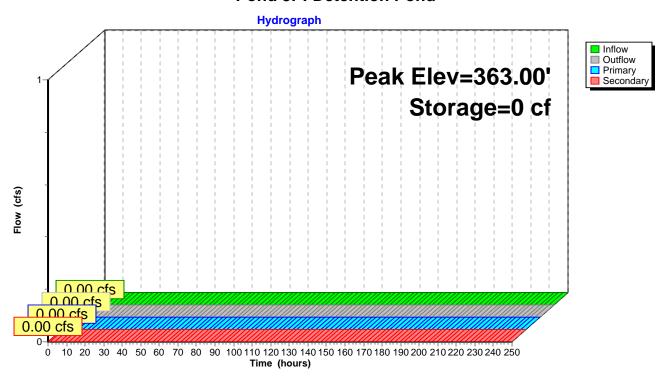
Center-of-Mass det. time= (not calculated: no inflow)

Volume	Inver	t Avail.Sto	rage Storage	Description	
#1	363.00	' 39,44	43 cf Custom	Stage Data (Pr	rismatic)Listed below (Recalc)
Elevatio		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
363.0	0	0	0	0	
364.0	0	1,137	569	569	
366.0	0	2,172	3,309	3,878	
368.0	0	3,441	5,613	9,491	
370.0		5,233	8,674	18,165	
372.0	0	7,184	12,417	30,582	
373.0		7,500	7,342	37,924	
373.2	.0	7,700	1,520	39,443	
Device	Routing	Invert	Outlet Device	S	
#1 #2	Primary Secondary	363.00' 373.00'	208.0' long X Head (feet) 0	0.20 0.40 0.60	r Surface area Broad-Crested Rectangular Weir 0.80 1.00 1.20 1.40 1.60 70 2.69 2.68 2.69 2.67 2.64

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=363.00' (Free Discharge) 1=Exfiltration (Controls 0.00 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=363.00' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 3P: Detention Pond



Printed 11/1/2010 Page 66

Summary for Pond 4P: Detention Pond

Inflow Area = 5.751 ac, 33.25% Impervious, Inflow Depth = 1.18" for 2 Year event Inflow 5.18 cfs @ 12.29 hrs. Volume= 0.565 af 0.60 cfs @ 14.26 hrs, Volume= Outflow 0.565 af, Atten= 88%, Lag= 118.5 min 0.60 cfs @ 14.26 hrs, Volume= Primary 0.565 af 0.00 cfs @ 0.00 hrs, Volume= Secondary = 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 371.59' @ 14.26 hrs Surf.Area= 2,607 sf Storage= 11,206 cf

Plug-Flow detention time= 243.3 min calculated for 0.565 af (100% of inflow)

Center-of-Mass det. time= 243.3 min (1,115.1 - 871.8)

Volume	Invert	Avail.Sto	rage	Storage	e Description	
#1	363.00'	12,5	57 cf	Custor	n Stage Data (Pi	rismatic)Listed below (Recalc)
				- .		
Elevation		urf.Area		Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic	:-feet)	(cubic-feet)	
363.0	00	0		0	0	
364.0	00	448		224	224	
366.0	00	893		1,341	1,565	
368.0	00	1,459		2,352	3,917	
370.0	00	2,095		3,554	7,471	
372.0	00	2,739		4,834	12,305	
372.0)9	2,850		252	12,557	
Device	Routing	Invert	Outle	et Device	es	
#1	Primary	363.00'	10.00	00 in/hr	Exfiltration ove	r Surface area
#2	Device 3	372.00'	54.0'	long x	1.5' breadth Bre	oad-Crested Rectangular Weir
			Head	(feet)	0.20 0.40 0.60	0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50	3.00		
			Coef	. (Englis	h) 2.62 2.64 2.	64 2.68 2.75 2.86 2.92 3.07 3.07
			3.03	3.28 3	.32	
#3	Secondary	371.00'	Cust	om Wei	r/Orifice, Cv= 2.	.62 (C= 3.28)
	-		Head	d (feet)	0.00 1.00	
			Width	n (feet)	2.00 2.00	

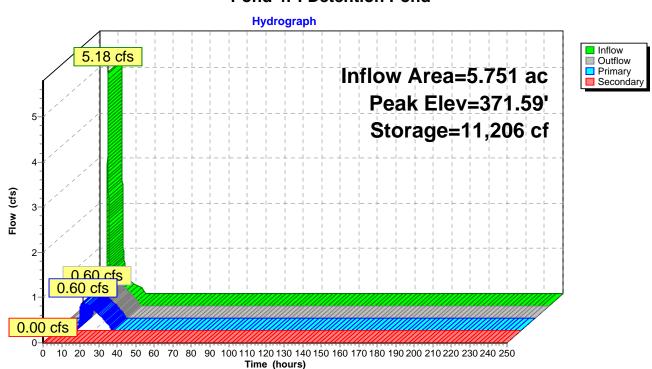
Primary OutFlow Max=0.60 cfs @ 14.26 hrs HW=371.59' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.60 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=363.00' (Free Discharge)

⁻³⁼Custom Weir/Orifice (Controls 0.00 cfs)

-2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 4P: Detention Pond



Post-Development3

Type III 24-hr 2 Year Rainfall=3.50" Printed 11/1/2010

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Page 68

Summary for Pond 6P: Drywells

Inflow Area =	0.231 ac,100.00% Impervious, Inflow De	epth = 3.27" for 2 Year event
Inflow =	0.45 cfs @ 12.39 hrs, Volume=	0.063 af
Outflow =	0.19 cfs @ 12.86 hrs, Volume=	0.063 af, Atten= 58%, Lag= 28.4 min
Primary =	0.19 cfs @ 12.86 hrs, Volume=	0.063 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 387.01' @ 12.86 hrs Surf.Area= 1,218 sf Storage= 496 cf

Plug-Flow detention time= 15.7 min calculated for 0.063 af (100% of inflow) Center-of-Mass det. time= 15.7 min (792.4 - 776.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	386.00'	1,291 cf	27.68'W x 44.00'L x 4.83'H Field A
			5,883 cf Overall - 2,655 cf Embedded = 3,227 cf x 40.0% Voids
#2A	387.00'	2,058 cf	Dry_Well 1000 Gallon x 16 Inside #1
			Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf
			Outside= 68.0"W x 34.0"H => 15.81 sf x 10.50'L = 166.0 cf
		3,349 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	386.00'	6.000 in/hr Exfiltration over Wetted area
#2	Secondary	393.00'	24.0" Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=0.19 cfs @ 12.86 hrs HW=387.01' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.19 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=386.00' (Free Discharge) 2=Orifice/Grate (Controls 0.00 cfs)

Pond 6P: Drywells - Chamber Wizard Field A

Chamber Model = Dry_Well 1000 Gallon

Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf Outside= 68.0"W x 34.0"H => 15.81 sf x 10.50'L = 166.0 cf

68.0" Wide + 12.0" Spacing = 80.0" C-C

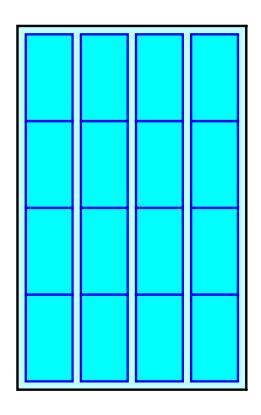
4 Chambers/Row x 10.50' Long = 42.00' + 12.0" End Stone x 2 = 44.00' Base Length 4 Rows x 68.0" Wide + 12.0" Spacing x 3 + 12.0" Side Stone x 2 = 27.68' Base Width 12.0" Base + 34.0" Chamber Height + 12.0" Cover = 4.83' Field Height

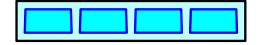
16 Chambers x 128.6 cf = 2,058.4 cf Chamber Storage 16 Chambers x 166.0 cf = 2,655.4 cf Displacement

5,882.6 cf Field - 2,655.4 cf Chambers = 3,227.2 cf Stone x 40.0% Voids = 1,290.9 cf Stone Storage

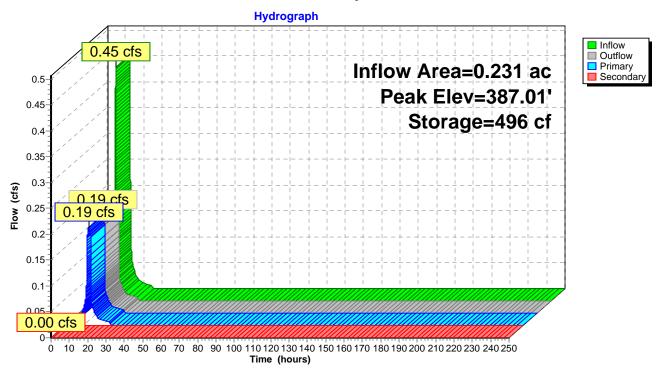
Stone + Chamber Storage = 3,349.3 cf = 0.077 af

16 Chambers 217.9 cy Field 119.5 cy Stone





Pond 6P: Drywells



Volume

Invert

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Page 71

Summary for Pond 7P: Stormwater Treatment Pond #2

Inflow Area =	2.937 ac, 37.76% Impervious, Inflow D	Depth = 1.18" for 2 Year event
Inflow =	2.11 cfs @ 12.49 hrs, Volume=	0.289 af
Outflow =	2.10 cfs @ 12.53 hrs, Volume=	0.289 af, Atten= 1%, Lag= 2.1 min
Primary =	0.17 cfs @ 12.53 hrs, Volume=	0.144 af
Secondary =	1.93 cfs @ 12.53 hrs, Volume=	0.144 af
Tertiary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 363.94' @ 12.53 hrs Surf.Area= 750 sf Storage= 1,219 cf

Plug-Flow detention time= 44.4 min calculated for 0.289 af (100% of inflow) Center-of-Mass det. time= 44.4 min (929.0 - 884.5)

Avail.Storage Storage Description

#1	361.00'	2,60	05 cf Custom	Stage Data (Pr	ismatic)Listed below (Recalc)
Elevation	on Su	ırf.Area	Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
361.0	00	100	0	0	
362.0	00	300	200	200	
364.0	00	763	1,063	1,263	
365.0	00	1,132	948	2,211	
365.3	30	1,500	395	2,605	
D	D. C.	Language	O that Day have		
Device	Routing	Invert	Outlet Devices	3	
#1	Primary	361.00'		xfiltration over	
#2	Secondary	363.50'		Orifice, Cv= 2.	62 (C= 3.28)
			Head (feet) 0.		
			Width (feet) 2		
#3	Tertiary	365.10'			oad-Crested Rectangular Weir
			` '		0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.5		
			Coef. (English)) 2.44 2.58 2.	68 2.67 2.65 2.64 2.64 2.68 2.68
			2.72 2.81 2.9	2 2.97 3.07 3	.32

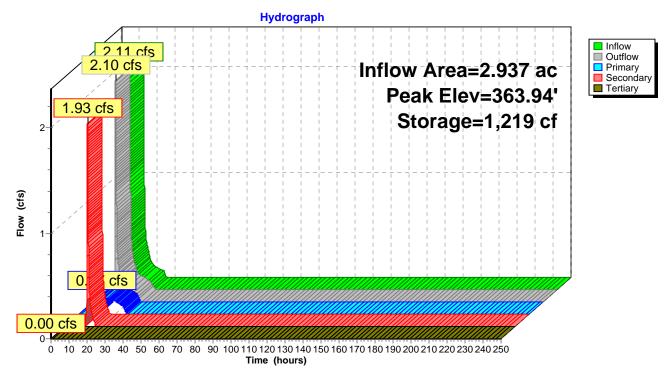
Primary OutFlow Max=0.17 cfs @ 12.53 hrs HW=363.94' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.17 cfs)

Secondary OutFlow Max=1.92 cfs @ 12.53 hrs HW=363.94' (Free Discharge) 2=Custom Weir/Orifice (Weir Controls 1.92 cfs @ 2.18 fps)

Tertiary OutFlow Max=0.00 cfs @ 0.00 hrs HW=361.00' (Free Discharge)

3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 7P: Stormwater Treatment Pond #2



Volume

Invert

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Page 73

Summary for Pond 8P: Stormwater Treatment Pond #1

Inflow 1.93 cfs @ 12.53 hrs, Volume= 0.144 af 1.55 cfs @ 12.73 hrs. Volume= 0.144 af, Atten= 20%, Lag= 12.2 min Outflow = 0.27 cfs @ 12.73 hrs, Volume= Primary 0.054 af 1.28 cfs @ 12.73 hrs, Volume= Secondary = 0.090 af

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 362.66' @ 12.73 hrs Surf.Area= 1,150 sf Storage= 1,231 cf

Plug-Flow detention time= 16.2 min calculated for 0.144 af (100% of inflow) Center-of-Mass det. time= 16.2 min (807.9 - 791.7)

Avail.Storage Storage Description

volullie	IIIVEIL	Avaii.Siu	lorage Storage Description			
#1	361.00'	5,86	67 cf Custon	n Stage Data (P	rismatic)Listed below (Recalc)	
Elevation		rf.Area	Inc.Store	Cum.Store		
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)		
361.0	00	300	0	0		
362.0	00	842	571	571		
364.0	00	1,772	2,614	3,185		
365.0	00	2,163	1,968	5,153		
365.3	30	2,600	714	5,867		
Device	Routing	Invert	Outlet Device	es		
#1	Secondary	361.00'	24.0" Round	d Culvert		
	•		L= 200.0' C	MP, end-section	conforming to fill, Ke= 0.500	
			Inlet / Outlet	Invert= 361.00' /	330.00' S= 0.1550 '/' Cc= 0.900	
			n= 0.013 Co	rrugated PE, sm	ooth interior	
#2	Device 1	361.10'		rifice/Grate (0 yr		
#3	Device 1	361.70'	6.0" Vert. Or	rifice/Grate (1yr)	C= 0.600	
#4	Device 1	362.30'		ifice/Grate(2yr)		
#5	Device 1	363.40'	9.0" Vert. Orifice/Grate(10yr) C= 0.600			
#6	Device 1	364.70'			Riser (100yr) C= 0.600	
				eir flow at low hea		
#7	Device 1	365.00'			oad-Crested Rectangular Weir (14.5)	
			` '		0.80 1.00 1.20 1.40 1.60 1.80 2.00	
			2.50 3.00 3			
					61 2.60 2.66 2.70 2.77 2.89 2.88	
			2.85 3.07 3			
#8	Device 1	365.20'			road-Crested Rectangular Weir	
					0.80 1.00 1.20 1.40 1.60 1.80 2.00	
				.50 4.00 4.50		
					68 2.67 2.65 2.64 2.64 2.68 2.68	
"0	Delesan	004.00		.92 2.97 3.07 3		
#9	Primary	361.00'	10.000 in/hr	Exfiltration ove	r Surface area	

Primary OutFlow Max=0.27 cfs @ 12.73 hrs HW=362.66' (Free Discharge) 9=Exfiltration (Exfiltration Controls 0.27 cfs)

Secondary OutFlow Max=1.28 cfs @ 12.73 hrs HW=362.66' (Free Discharge)

1=Culvert (Passes 1.28 cfs of 12.25 cfs potential flow)

2=Orifice/Grate (0 yr) (Orifice Controls 0.13 cfs @ 5.86 fps)

3=Orifice/Grate (1yr) (Orifice Controls 0.80 cfs @ 4.06 fps)

4=Orifice/Grate(2yr) (Orifice Controls 0.36 cfs @ 2.05 fps)

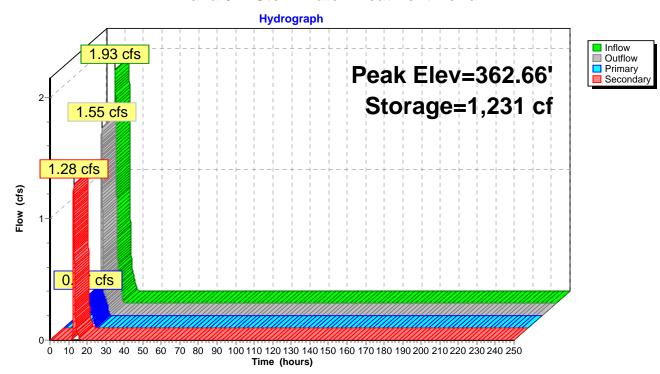
5=Orifice/Grate(10yr) (Controls 0.00 cfs)

6=Top of Riser (100yr) (Controls 0.00 cfs)

7=Broad-Crested Rectangular Weir (14.5) (Controls 0.00 cfs)

8=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 8P: Stormwater Treatment Pond #1



Post-Development3

Type III 24-hr 2 Year Rainfall=3.50" Printed 11/1/2010

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Page 75

Summary for Pond 9P: Drywells

Inflow Area =	0.459 ac,100.00% Impervious, Inflow De	epth = 3.27" for 2 Year event
Inflow =	1.09 cfs @ 12.25 hrs, Volume=	0.125 af
Outflow =	0.25 cfs @ 11.86 hrs, Volume=	0.125 af, Atten= 77%, Lag= 0.0 min
Primary =	0.25 cfs @ 11.86 hrs, Volume=	0.125 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 387.44' @ 12.81 hrs Surf.Area= 1,834 sf Storage= 1,403 cf

Plug-Flow detention time= 32.1 min calculated for 0.125 af (100% of inflow) Center-of-Mass det. time= 32.1 min (798.8 - 766.7)

Volume	Invert	Avail.Storage	Storage Description
#1A	386.00'	1,685 cf	41.69'W x 44.00'L x 4.83'H Field A
			8,860 cf Overall - 4,647 cf Embedded = 4,213 cf \times 40.0% Voids
#2A	387.00'	3,602 cf	Dry_Well 1000 Gallon x 28 Inside #1
			Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf
			Outside= 68.0"W x 34.0"H => 15.81 sf x 10.50'L = 166.0 cf
		5 007 <i>(</i>	T

5,287 cf Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	386.00'	6.000 in/hr Exfiltration over Surface area
#2	Secondary	392.00'	24.0" Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=0.25 cfs @ 11.86 hrs HW=386.08' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.25 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=386.00' (Free Discharge) 2=Orifice/Grate (Controls 0.00 cfs)

Page 76

Pond 9P: Drywells - Chamber Wizard Field A

Chamber Model = Dry Well 1000 Gallon

Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf Outside= 68.0"W x 34.0"H => 15.81 sf x 10.50'L = 166.0 cf

68.0" Wide + 0.0" Spacing = 68.0" C-C

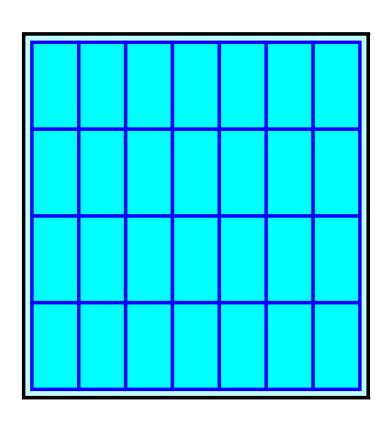
4 Chambers/Row x 10.50' Long = 42.00' + 12.0'' End Stone x 2 = 44.00' Base Length 7 Rows x 68.0'' Wide + 12.0'' Side Stone x 2 = 41.69' Base Width 12.0'' Base + 34.0'' Chamber Height + 12.0'' Cover = 4.83' Field Height

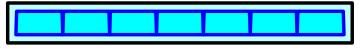
28 Chambers x 128.6 cf = 3,602.2 cf Chamber Storage 28 Chambers x 166.0 cf = 4,646.9 cf Displacement

8,860.0 cf Field - 4,646.9 cf Chambers = 4,213.1 cf Stone x 40.0% Voids = 1,685.2 cf Stone Storage

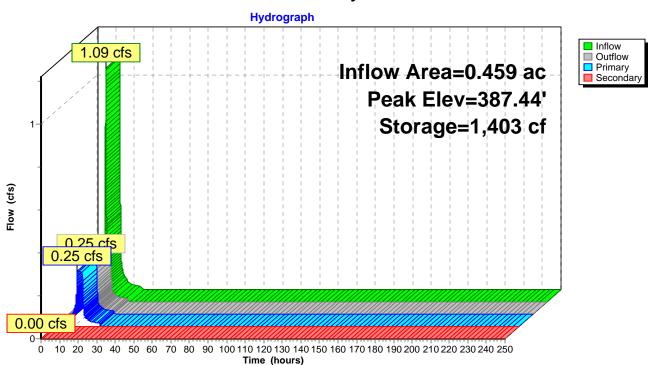
Stone + Chamber Storage = 5,287.4 cf = 0.121 af

28 Chambers 328.1 cy Field 156.0 cy Stone





Pond 9P: Drywells



Summary for Link DP-1: Design Point 1

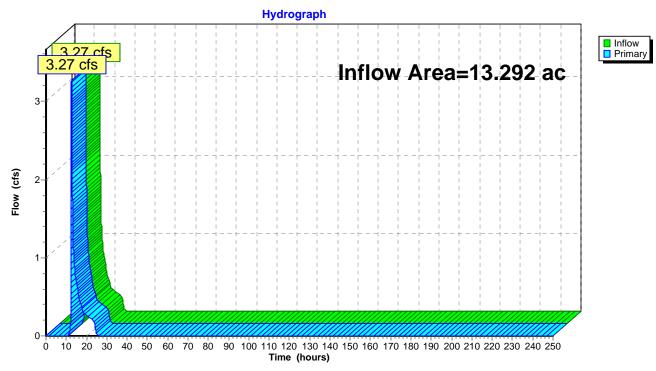
Inflow Area = 13.292 ac, 3.27% Impervious, Inflow Depth = 0.53" for 2 Year event

Inflow = 3.27 cfs @ 12.58 hrs, Volume= 0.589 af

Primary = 3.27 cfs @ 12.58 hrs, Volume= 0.589 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-1: Design Point 1



Summary for Link DP-2: Design Point 2

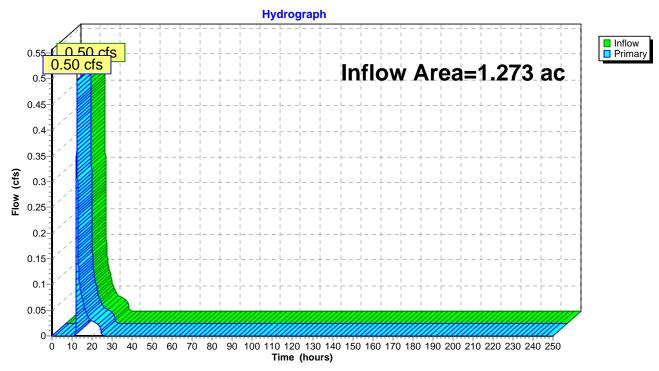
Inflow Area = 1.273 ac, 7.29% Impervious, Inflow Depth = 0.66" for 2 Year event

Inflow = 0.50 cfs @ 12.38 hrs, Volume= 0.070 af

Primary = 0.50 cfs @ 12.38 hrs, Volume= 0.070 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-2: Design Point 2



Summary for Link DP-3: Design Point 3

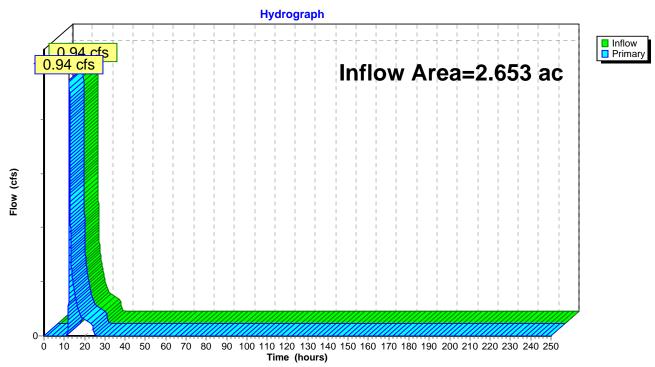
Inflow Area = 2.653 ac, 7.97% Impervious, Inflow Depth = 0.66" for 2 Year event

Inflow = 0.94 cfs @ 12.49 hrs, Volume= 0.146 af

Primary = 0.94 cfs @ 12.49 hrs, Volume= 0.146 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-3: Design Point 3



Summary for Link DP-4: Design Point 4

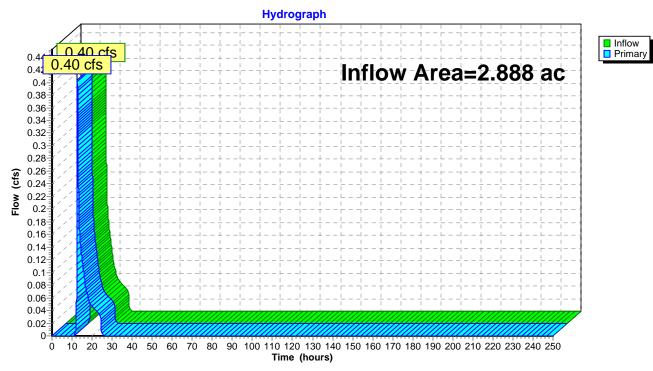
0.56% Impervious, Inflow Depth = 0.38" for 2 Year event Inflow Area = 2.888 ac,

0.091 af Inflow 0.40 cfs @ 12.66 hrs. Volume=

0.40 cfs @ 12.66 hrs, Volume= Primary 0.091 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-4: Design Point 4



Summary for Link DP-5: Design Point 5

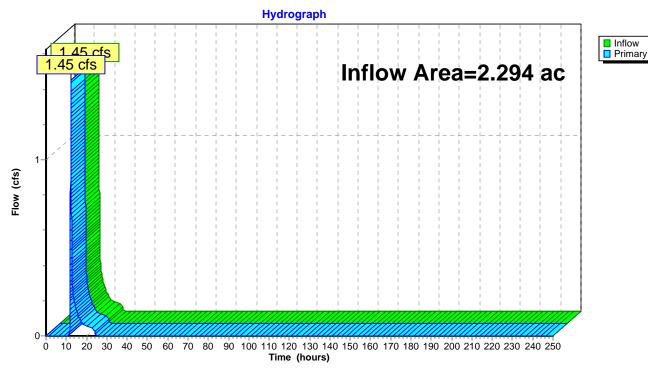
Inflow Area = 2.294 ac, 15.79% Impervious, Inflow Depth = 0.80" for 2 Year event

Inflow = 1.45 cfs @ 12.21 hrs, Volume= 0.153 af

Primary = 1.45 cfs @ 12.21 hrs, Volume= 0.153 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-5: Design Point 5



Summary for Link DP-6: Design Point 6

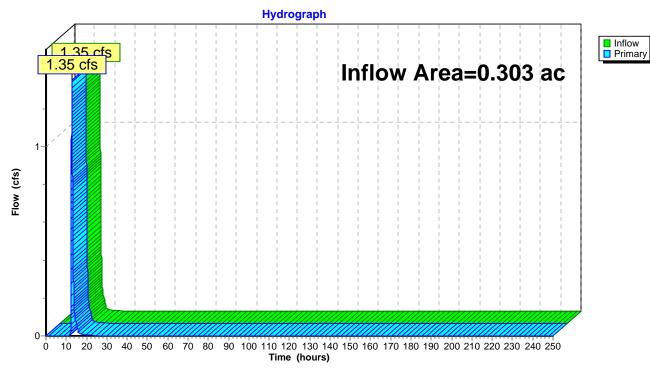
Inflow Area = 0.303 ac, 0.00% Impervious, Inflow Depth = 4.09" for 2 Year event

Inflow = 1.35 cfs @ 12.72 hrs, Volume= 0.103 af

Primary = 1.35 cfs @ 12.72 hrs, Volume= 0.103 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-6: Design Point 6



Summary for Link DP-7: Design Point 7

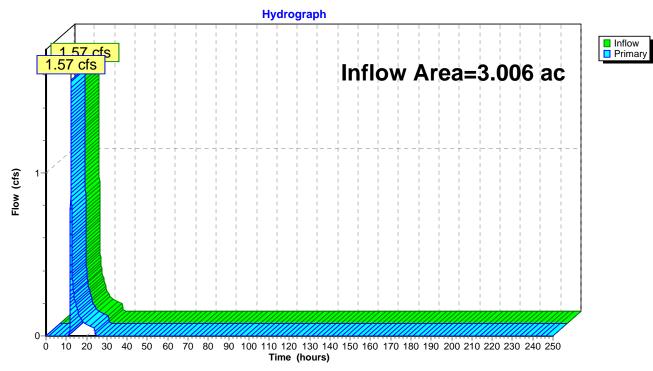
9.68% Impervious, Inflow Depth = 0.71" for 2 Year event Inflow Area = 3.006 ac,

1.57 cfs @ 12.22 hrs, Volume= Inflow 0.177 af

1.57 cfs @ 12.22 hrs, Volume= 0.177 af, Atten= 0%, Lag= 0.0 min Primary

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-7: Design Point 7



Printed 11/1/2010 Page 85

Time span=0.00-250.00 hrs, dt=0.01 hrs, 25001 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

Subcatchment 1: Drainage Area 1	Runoff Area=13.292 ac 3.27% Impervious Runoff Depth=1.30" Flow Length=1,455' Tc=31.8 min CN=60 Runoff=10.00 cfs 1.441 af
Subcatchment 2A: Drainage Area 2A	Runoff Area=5.751 ac 33.25% Impervious Runoff Depth=2.28" Flow Length=999' Tc=19.1 min CN=73 Runoff=10.45 cfs 1.093 af
Subcatchment 2B: Drainage Area 2B	Runoff Area=19,994 sf 100.00% Impervious Runoff Depth=4.76" Flow Length=999' Tc=19.1 min CN=98 Runoff=1.56 cfs 0.182 af
Subcatchment3: Drainage Area 3	Runoff Area=115,560 sf 7.97% Impervious Runoff Depth=1.51" Flow Length=552' Tc=28.6 min CN=63 Runoff=2.52 cfs 0.334 af
Subcatchment 4: Drainage Area 4	Runoff Area=55,457 sf 7.29% Impervious Runoff Depth=1.51" Flow Length=284' Tc=22.5 min CN=63 Runoff=1.34 cfs 0.160 af
Subcatchment 5: Drainage Area 5	Runoff Area=125,820 sf 0.56% Impervious Runoff Depth=1.04" Flow Length=497' Tc=33.7 min CN=56 Runoff=1.58 cfs 0.251 af
Subcatchment 6A: Drainage Area 6	Runoff Area=2.937 ac 37.76% Impervious Runoff Depth=2.28" Flow Length=578' Tc=32.3 min CN=73 Runoff=4.26 cfs 0.558 af
Subcatchment 6B: Drainage Area 6	Runoff Area=10,050 sf 100.00% Impervious Runoff Depth=4.76" Flow Length=540' Tc=29.9 min CN=98 Runoff=0.65 cfs 0.092 af
Subcatchment 6c: Drainage Area 6	Runoff Area=13,208 sf 0.00% Impervious Runoff Depth=1.30" Flow Length=540' Tc=29.9 min CN=60 Runoff=0.23 cfs 0.033 af
Subcatchment 7: Drainage Area 7	Runoff Area=3.006 ac 9.68% Impervious Runoff Depth=1.58" Flow Length=527' Tc=13.4 min CN=64 Runoff=4.11 cfs 0.396 af
Subcatchment 8: Drainage Area 8	Runoff Area=99,910 sf 15.79% Impervious Runoff Depth=1.73" Flow Length=558' Tc=13.0 min CN=66 Runoff=3.53 cfs 0.330 af
Reach 1R: 18" 18.0" Round Pipe n=0.013	Avg. Flow Depth=0.42' Max Vel=10.42 fps Inflow=4.26 cfs 0.558 af L=132.0' S=0.0546 '/' Capacity=24.55 cfs Outflow=4.26 cfs 0.558 af
Pond 1P: Pond - D Primary=4.26	Peak Elev=367.96' Inflow=4.26 cfs 0.558 af cfs 0.558 af Secondary=0.00 cfs 0.000 af Outflow=4.26 cfs 0.558 af
Pond 3P: Detention Pond Primary=0.80	Peak Elev=368.03' Storage=9,596 cf Inflow=13.96 cfs 0.355 af cfs 0.355 af Secondary=0.00 cfs 0.000 af Outflow=0.80 cfs 0.355 af
Pond 4P: Detention Pond Primary=0.66 cf	Peak Elev=373.53' Storage=12,557 cf Inflow=10.45 cfs 1.093 af s 0.738 af Secondary=13.96 cfs 0.355 af Outflow=14.62 cfs 1.093 af
Pond 6P: Drywells Primary=0.20	Peak Elev=387.49' Storage=951 cf Inflow=0.65 cfs 0.092 af cfs 0.092 af Secondary=0.00 cfs 0.000 af Outflow=0.20 cfs 0.092 af

Type III 24-hr 10 Year Rainfall=5.00"

Prepared by Petruccelli Engineering
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Page 86

Pond 7P: Stormwater Treatment Pond #2 Peak Elev=364.22' Storage=1,443 cf Inflow=4.26 cfs 0.558 af Primary=0.20 cfs 0.185 af Secondary=4.03 cfs 0.373 af Tertiary=0.00 cfs 0.000 af Outflow=4.23 cfs 0.558 af

Pond 8P: Stormwater Treatment Pond #1 Peak Elev=363.67' Storage=2,623 cf Inflow=4.03 cfs 0.373 af Primary=0.37 cfs 0.100 af Secondary=2.99 cfs 0.273 af Outflow=3.37 cfs 0.373 af

Pond 9P: Drywells Peak Elev=388.13' Storage=2,457 cf Inflow=1.56 cfs 0.182 af

Primary=0.25 cfs 0.182 af Secondary=0.00 cfs 0.000 af Outflow=0.25 cfs 0.182 af

Link DP-1: Design Point 1 Inflow=10.00 cfs 1.441 af Primary=10.00 cfs 1.441 af

Link DP-2: Design Point 2 Inflow=1.34 cfs 0.160 af Primary=1.34 cfs 0.160 af

Link DP-3: Design Point 3 Inflow=2.52 cfs 0.334 af

Link DP-4: Design Point 4 Inflow=1.58 cfs 0.251 af

Primary=1.58 cfs 0.251 af

Link DP-5: Design Point 5 Inflow=3.53 cfs 0.330 af Primary=3.53 cfs 0.330 af

1 mary=3.55 dis 0.550 di

Link DP-6: Design Point 6 Inflow=3.18 cfs 0.306 af Primary=3.18 cfs 0.306 af

Link DP-7: Design Point 7 Inflow=4.11 cfs 0.396 af

Primary=4.11 cfs 0.396 af

Primary=2.52 cfs 0.334 af

Total Runoff Area = 35.087 ac Runoff Volume = 4.869 af Average Runoff Depth = 1.67" 85.41% Pervious = 29.968 ac 14.59% Impervious = 5.119 ac

Page 87

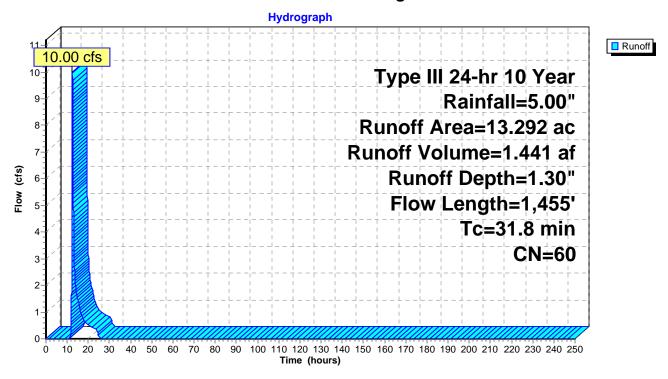
Summary for Subcatchment 1: Drainage Area 1

Runoff = 10.00 cfs @ 12.50 hrs, Volume= 1.441 af, Depth= 1.30"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.00"

Area	(ac) C	N Desc	cription					
2.770 69 50-75% Grass cover, Fair, HSG B								
0.435 98 Paved roads w/curbs & sewers								
9.824 55 Woods, Good, HSG B								
0.	0.263 82 Dirt roads, HSG B							
13.292 60 Weighted Average								
12.	857	96.7	3% Pervio	us Area				
0.	435	3.27	% Impervi	ous Area				
_								
Tc	Length	Slope	Velocity	Capacity	Description			
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)				
18.9	100	0.0260	0.09		Sheet Flow, 1 to 2			
4 7	040	0.4000	0.44		Grass: Bermuda n= 0.410 P2= 3.50"			
1.7	216	0.1830	2.14		Shallow Concentrated Flow, 2 to 3			
0.2	70	0.0450	4 24		Woodland Kv= 5.0 fps			
0.3	78	0.0450	4.31		Shallow Concentrated Flow, 3 to 4 Paved Kv= 20.3 fps			
1.2	121	0.1150	1.70		Shallow Concentrated Flow, 4 to 5			
1.2	121	0.1150	1.70		Woodland Kv= 5.0 fps			
5.8	679	0.0770	1.94		Shallow Concentrated Flow, 5 to 6			
0.0	0.0	0.0770	1.01		Short Grass Pasture Kv= 7.0 fps			
3.9	261	0.0500	1.12		Shallow Concentrated Flow, 6 to DP1			
					Woodland Kv= 5.0 fps			
31.8	1,455	Total			·			

Subcatchment 1: Drainage Area 1



Page 89

Summary for Subcatchment 2A: Drainage Area 2A

Runoff = 10.45 cfs @ 12.27 hrs, Volume= 1.093 af, Depth= 2.28"

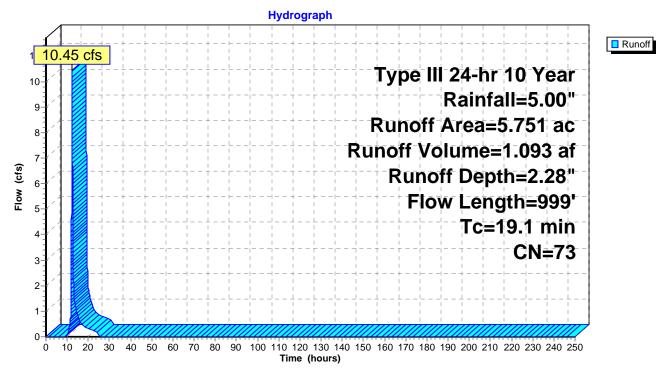
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.00"

Area	(ac) C	N Desc	cription		
1.	.912	98 Pave	ed roads w	/curbs & se	ewers
				over, Good,	, HSG B
0	.520 5	55 Woo	ds, Good,	HSG B	
_	-	•	ghted Aver	•	
	.839		5% Pervio		
1.	.912	33.2	5% Imper	ious Area	
Тс	Longth	Slope	Velocity	Capacity	Description
(min)	Length (feet)	Slope (ft/ft)	(ft/sec)	(cfs)	Description
11.5	100	0.0900	0.15	(013)	Sheet Flow, 1 to 2
11.5	100	0.0900	0.13		Grass: Bermuda n= 0.410 P2= 3.50"
5.8	423	0.0300	1.21		Shallow Concentrated Flow, 2 to 3
0.0	120	0.0000	1.21		Short Grass Pasture Kv= 7.0 fps
0.5	74	0.0140	2.40		Shallow Concentrated Flow, 3 to 4 (Road)
					Paved Kv= 20.3 fps
0.1	25	0.0100	4.54	3.56	•
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
					n= 0.013 Corrugated PE, smooth interior
0.6	167	0.0120	4.97	3.90	
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
					n= 0.013 Corrugated PE, smooth interior
0.4	70	0.0090	2.60	3.19	
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
0.0	1.10	0.4540	12.01	24.22	n= 0.025 Corrugated metal
0.2	140	0.1510	12.01	21.23	Pipe Channel, 7 to 8 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'
					n= 0.025 Corrugated metal
19.1	000	Total			11- 0.020 Confugated metal
19.1	999	iolai			

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Page 90

Subcatchment 2A: Drainage Area 2A



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Page 91

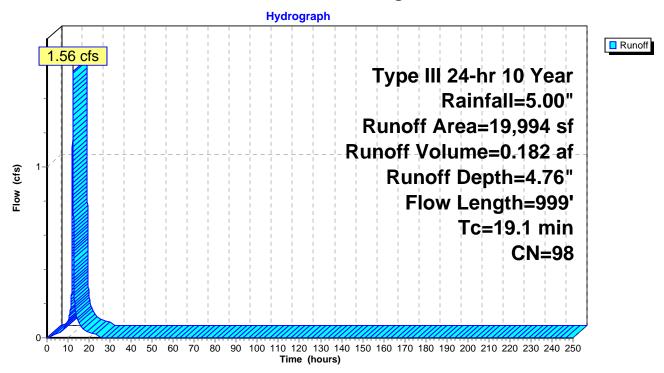
Runoff = 1.56 cfs @ 12.25 hrs, Volume= 0.182 af, Depth= 4.76"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.00"

	Α	rea (sf)	CN [Description		
		19,994	98 F	Paved road	s w/curbs &	& sewers
		19,994	1	00.00% In	pervious A	rea
		•			•	
	Tc	Length	Slope	Velocity	Capacity	Description
<u>(n</u>	nin)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
1	1.5	100	0.0900	0.15		Sheet Flow, 1 to 2
						Grass: Bermuda n= 0.410 P2= 3.50"
	5.8	423	0.0300	1.21		Shallow Concentrated Flow, 2 to 3
						Short Grass Pasture Kv= 7.0 fps
	0.5	74	0.0140	2.40		Shallow Concentrated Flow, 3 to 4 (Road)
	0.4	0.5	0.0400	4.5.4	0.50	Paved Kv= 20.3 fps
	0.1	25	0.0100	4.54	3.56	•
						12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
	0.6	167	0.0120	4.97	3.90	n= 0.013 Corrugated PE, smooth interior Pipe Channel, 5 to 6
	0.0	107	0.0120	4.97	3.90	12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
						n= 0.013 Corrugated PE, smooth interior
	0.4	70	0.0090	2.60	3.19	Pipe Channel, 6 to 7
	0.1	70	0.0000	2.00	0.10	15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
						n= 0.025 Corrugated metal
	0.2	140	0.1510	12.01	21.23	•
						18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'
						n= 0.025 Corrugated metal
1	9.1	999	Total			-

Summary for Subcatchment 2B: Drainage Area 2B

Subcatchment 2B: Drainage Area 2B



Page 93

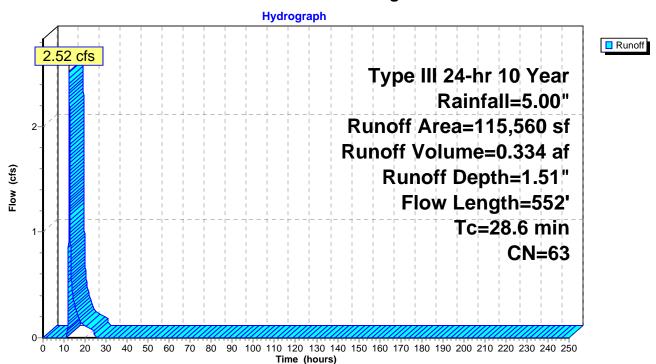
Summary for Subcatchment 3: Drainage Area 3

Runoff = 2.52 cfs @ 12.43 hrs, Volume= 0.334 af, Depth= 1.51"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.00"

A	rea (sf)	CN E	escription					
	49,852	61 >75% Grass cover, Good, HSG B						
	56,494	60 V	Voods, Fai	r, HSG B				
	9,214	98 F	aved road	s w/curbs &	R sewers			
1	15,560	63 V	Veighted A	verage				
1	06,346	9	2.03% Per	vious Area				
	9,214	7	.97% Impe	ervious Area	a			
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
19.4	118	0.0320	0.10		Sheet Flow, 1 to 2			
					Woods: Light underbrush n= 0.400 P2= 3.50"			
2.5	87	0.0140	0.59		Shallow Concentrated Flow, 2 to 3			
					Woodland Kv= 5.0 fps			
1.2	159	0.1940	2.20		Shallow Concentrated Flow, 3 to 4			
					Woodland Kv= 5.0 fps			
5.5	188	0.0130	0.57		Shallow Concentrated Flow, 4 to DP 3			
					Woodland Kv= 5.0 fps			
28.6	552	Total						

Subcatchment 3: Drainage Area 3



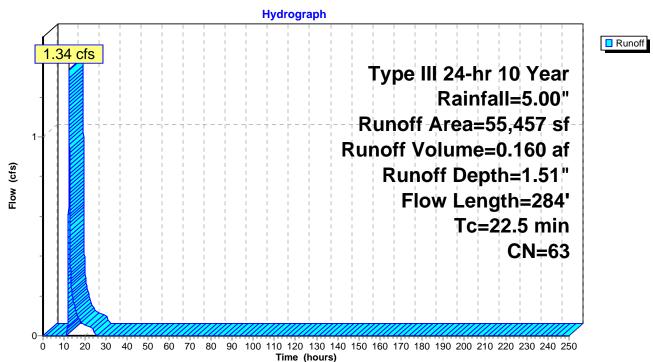
Summary for Subcatchment 4: Drainage Area 4

Runoff = 1.34 cfs @ 12.34 hrs, Volume= 0.160 af, Depth= 1.51"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.00"

_	Α	rea (sf)	CN	Description						
		31,781	60	Woods, Fair, HSG B						
		4,042	98	Paved road	s w/curbs &	& sewers				
_		19,634	61	>75% Gras	s cover, Go	ood, HSG B				
		55,457	63	Weighted A	verage					
		51,415		92.71% Per	rvious Area					
		4,042		7.29% Impe	ervious Are	a				
_	Tc (min)	Length (feet)	Slope (ft/ft	,	Capacity (cfs)	Description				
	20.9	207	0.0860	0.16		Sheet Flow, 1 to 2				
	1.6	77	0.0130	0.80		Grass: Bermuda n= 0.410 P2= 3.50" Shallow Concentrated Flow, 2 to DP 4 Short Grass Pasture Kv= 7.0 fps				
	22.5	284	Total							

Subcatchment 4: Drainage Area 4



Page 95

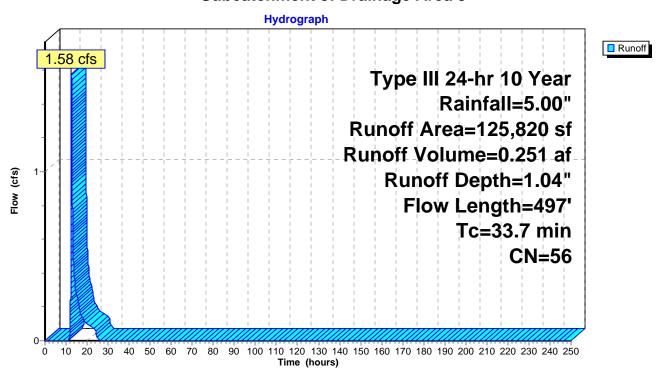
Summary for Subcatchment 5: Drainage Area 5

Runoff = 1.58 cfs @ 12.55 hrs, Volume= 0.251 af, Depth= 1.04"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.00"

A	rea (sf)	CN D	escription		
	23,735			,	ood, HSG B
1	01,385	55 V	Voods, Go	od, HSG B	
	700	98 F	aved park	ing, HSG B	
1	25,820	56 V	Veighted A	verage	
1	25,120	9	9.44% Per	vious Area	
	700	0	.56% Impe	ervious Area	a
			•		
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·
29.9	135	0.0150	0.08		Sheet Flow, 1 to 2
					Grass: Bermuda n= 0.410 P2= 3.50"
3.8	362	0.1020	1.60		Shallow Concentrated Flow, 2 to DP 4
					Woodland Kv= 5.0 fps
33.7	497	Total			

Subcatchment 5: Drainage Area 5



Page 96

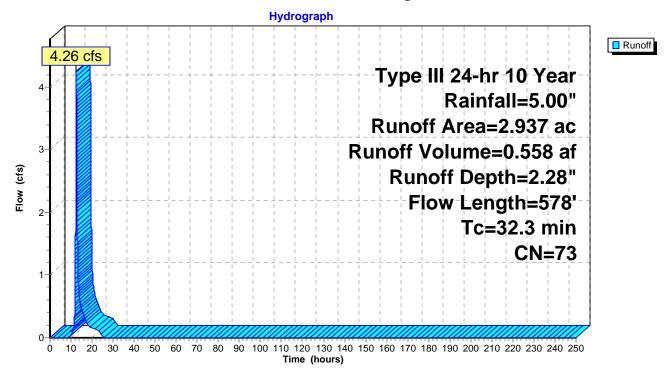
Summary for Subcatchment 6A: Drainage Area 6

Runoff = 4.26 cfs @ 12.46 hrs, Volume= 0.558 af, Depth= 2.28"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.00"

Area	(ac) C	N Desc	cription		
0.	.893 5	55 Woo	ds, Good,	HSG B	
1.	.109 9	8 Pave	ed roads w	/curbs & se	ewers
0	.935 6	§1 >75%	% Grass co	over, Good,	, HSG B
2.	.937 7	'3 Weig	hted Aver	age	
1.	.828	62.2	4% Pervio	us Area	
1.	.109	37.7	6% Imperv	ious Area	
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
21.6	185	0.0600	0.14		Sheet Flow, 1 to 2
					Woods: Light underbrush n= 0.400 P2= 3.50"
0.5	214	0.1180	6.97		Shallow Concentrated Flow, 2 to 3
					Paved Kv= 20.3 fps
0.0	17	0.0120	6.25	7.67	Pipe Channel, 3 to 4 (Catchbasins)
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.012 HDPE
0.4	132	0.0077	5.22	9.22	Pipe Channel, 4 to 5 (18" Culvert)
					18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'
					n= 0.013 HDPE
9.8	30	0.0120	0.05		Sheet Flow, 5 to DP 6
					Grass: Bermuda n= 0.410 P2= 3.50"
32.3	578	Total			

Subcatchment 6A: Drainage Area 6



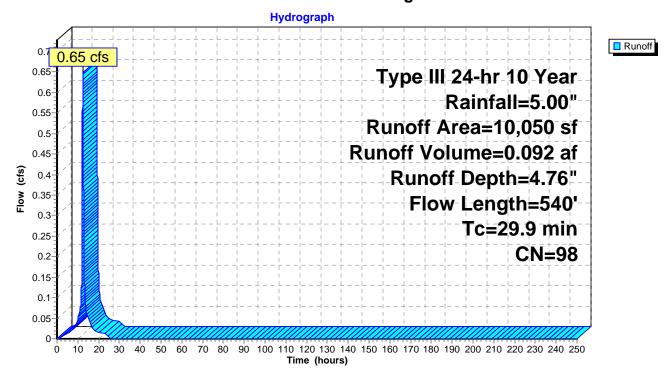
Summary for Subcatchment 6B: Drainage Area 6

Runoff = 0.65 cfs @ 12.39 hrs, Volume= 0.092 af, Depth= 4.76"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.00"

	^	"" (°t)	CN1 1			
_	A	rea (sf)	CN [Description		
		10,050	98 F	Paved road	s w/curbs 8	R sewers
		10,050	•	100.00% In	npervious A	ırea
	Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description
	21.6	185	0.0600	0.14		Sheet Flow, 1 to 2
						Woods: Light underbrush n= 0.400 P2= 3.50"
	0.5	214	0.1180	6.97		Shallow Concentrated Flow, 2 to 3
						Paved Kv= 20.3 fps
	0.0	17	0.0120	6.25	7.67	
						15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
						n= 0.012 HDPE
	0.1	101	0.1730	24.72	43.69	Pipe Channel, 4 to 5 (18" Culvert)
						18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'
						n= 0.013 HDPE
	7.7	23	0.0130	0.05		Sheet Flow, 5 to DP 6
						Grass: Bermuda n= 0.410 P2= 3.50"
	29.9	540	Total			

Subcatchment 6B: Drainage Area 6



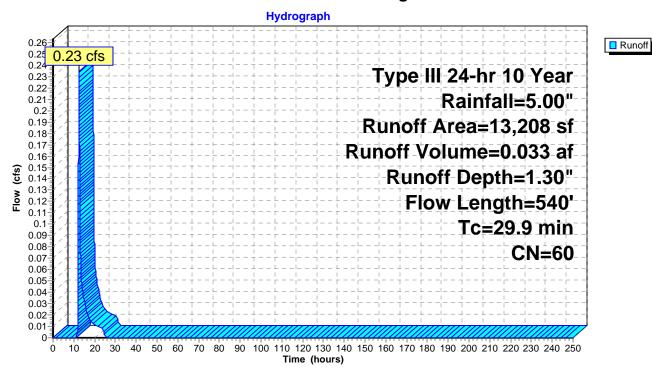
Summary for Subcatchment 6c: Drainage Area 6

Runoff = 0.23 cfs @ 12.46 hrs, Volume= 0.033 af, Depth= 1.30"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.00"

_	Α	rea (sf)	CN	Description		
Ī		13,208	60	Woods, Fai	r, HSG B	
		13,208		100.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	•	Capacity (cfs)	Description
	21.6	185	0.0600	0.14		Sheet Flow, 1 to 2 Woods: Light underbrush n= 0.400 P2= 3.50"
	0.5	214	0.1180	6.97		Shallow Concentrated Flow, 2 to 3 Paved Kv= 20.3 fps
	0.0	17	0.0120	6.25	7.67	Pipe Channel, 3 to 4 (Catchbasins) 15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
	0.1	101	0.1730	24.72	43.69	n= 0.012 HDPE Pipe Channel, 4 to 5 (18" Culvert) 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'
_	7.7	23	0.0130	0.05		n= 0.013 HDPE Sheet Flow, 5 to DP 6 Grass: Bermuda n= 0.410 P2= 3.50"
	29.9	540	Total			

Subcatchment 6c: Drainage Area 6



Page 100

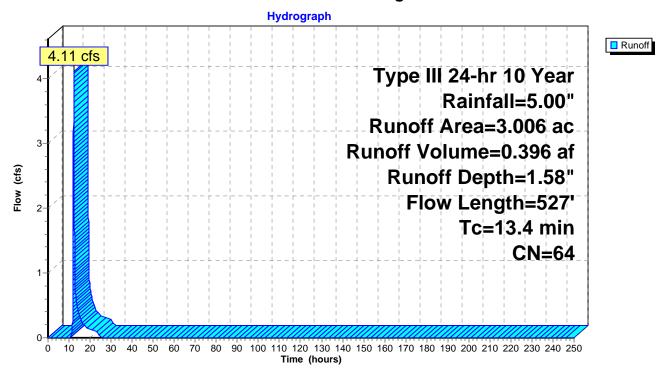
Summary for Subcatchment 7: Drainage Area 7

Runoff = 4.11 cfs @ 12.20 hrs, Volume= 0.396 af, Depth= 1.58"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.00"

Area	(ac) C	N Desc	cription						
-	` /		ds, Fair, H	ISG B					
0.291 98 Paved roads w/curbs & sewers									
1.375 61 >75% Grass cover, Good, HSG B									
	, ,								
	3.006 64 Weighted Average								
2.	715		2% Pervio						
0.	291	9.68	% Impervi	ous Area					
Tc	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·				
10.1	117	0.1620	0.19		Sheet Flow, 1 to 2				
					Woods: Light underbrush n= 0.400 P2= 3.50"				
0.7	100	0.2300	2.40		Shallow Concentrated Flow, 2 to 3				
0.7	100	0.2000	2.10		Woodland Kv= 5.0 fps				
1.2	164	0.2010	2.24		Shallow Concentrated Flow, 3 to 4				
1.2	104	0.2010	2.24		•				
4.4	4.40	0.4000	4 75		Woodland Kv= 5.0 fps				
1.4	146	0.1230	1.75		Shallow Concentrated Flow, 3 to DP 7				
					Woodland Kv= 5.0 fps				
13.4	527	Total							

Subcatchment 7: Drainage Area 7



Page 101

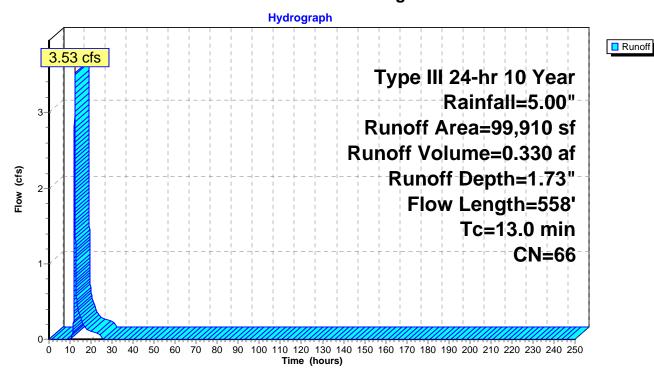
Summary for Subcatchment 8: Drainage Area 8

Runoff = 3.53 cfs @ 12.19 hrs, Volume= 0.330 af, Depth= 1.73"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 10 Year Rainfall=5.00"

	Area (sf)	CN E	escription		
	41,570	60 V	Voods, Fai	r, HSG B	
	15,772	98 F	aved road	s w/curbs &	& sewers
	42,568	61 >	75% Gras	s cover, Go	ood, HSG B
	99,910		Veighted A	•	
	84,138	_		vious Area	
	15,772	1	5.79% lmp	ervious Ar	ea
To	Longth	Slope	\/olooit\/	Capacity	Description
Tc (min)	Length (feet)	(ft/ft)	Velocity (ft/sec)	(cfs)	Description
9.7	100	0.1300	0.17	(013)	Sheet Flow, 1 to 2
3.1	100	0.1300	0.17		Woods: Light underbrush n= 0.400 P2= 3.50"
3.1	362	0.1550	1.97		Shallow Concentrated Flow, 2 to 3
0.1	002	0.1000	1.07		Woodland Kv= 5.0 fps
0.2	96	0.1150	6.88		Shallow Concentrated Flow, 3 to DP 8
					Paved Kv= 20.3 fps
13.0	558	Total			

Subcatchment 8: Drainage Area 8



Inflow
Outflow

Summary for Reach 1R: 18"

Inflow Area = 2.937 ac, 37.76% Impervious, Inflow Depth = 2.28" for 10 Year event

Inflow = 4.26 cfs @ 12.46 hrs, Volume= 0.558 af

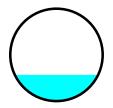
Outflow = 4.26 cfs @ 12.46 hrs, Volume= 0.558 af, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

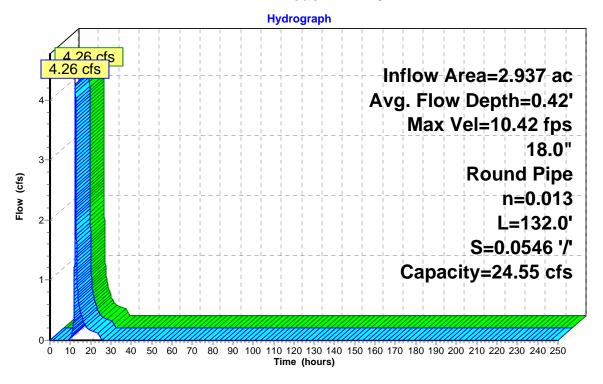
Max. Velocity= 10.42 fps, Min. Travel Time= 0.2 min Avg. Velocity = 4.36 fps, Avg. Travel Time= 0.5 min

Peak Storage= 54 cf @ 12.46 hrs Average Depth at Peak Storage= 0.42' Bank-Full Depth= 1.50', Capacity at Bank-Full= 24.55 cfs

18.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 132.0' Slope= 0.0546 '/' Inlet Invert= 374.01', Outlet Invert= 366.80'



Reach 1R: 18"



Summary for Pond 1P: Pond - D

Inflow Area =	2.937 ac, 37.76% Impervious, Inflow D	epth = 2.28" for 10 Year event
Inflow =	4.26 cfs @ 12.46 hrs, Volume=	0.558 af
Outflow =	4.26 cfs @ 12.46 hrs, Volume=	0.558 af, Atten= 0%, Lag= 0.0 min
Primary =	4.26 cfs @ 12.46 hrs, Volume=	0.558 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

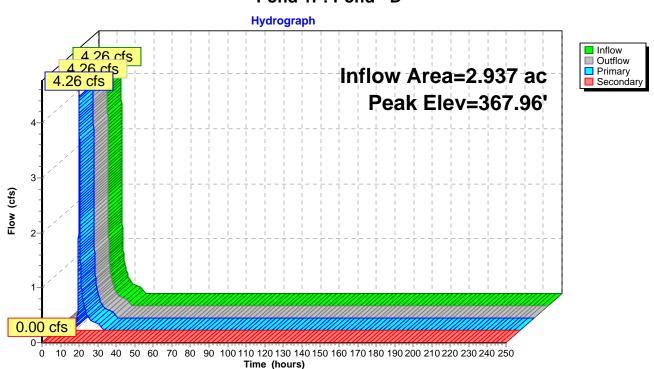
Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 367.96' @ 12.46 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	366.80'	18.0" Round Culvert
	•		L= 6.0' CMP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 366.80' / 361.00' S= 0.9667 '/' Cc= 0.900
			n= 0.025 Corrugated metal
#2	Secondary	371.19'	57.0" W x 57.0" H Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=4.26 cfs @ 12.46 hrs HW=367.96' (Free Discharge) 1=Culvert (Inlet Controls 4.26 cfs @ 2.90 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=366.80' (Free Discharge) 2=Orifice/Grate (Controls 0.00 cfs)

Pond 1P: Pond - D



Prepared by Petruccelli Engineering

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Page 104

Summary for Pond 3P: Detention Pond

Inflow = 13.96 cfs @ 12.35 hrs, Volume= 0.355 af

Outflow = 0.80 cfs @ 13.45 hrs, Volume= 0.355 af, Atten= 94%, Lag= 65.9 min

Primary = 0.80 cfs @ 13.45 hrs, Volume= 0.355 af Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 368.03' @ 13.45 hrs Surf.Area= 3,468 sf Storage= 9,596 cf

Plug-Flow detention time= 152.3 min calculated for 0.355 af (100% of inflow)

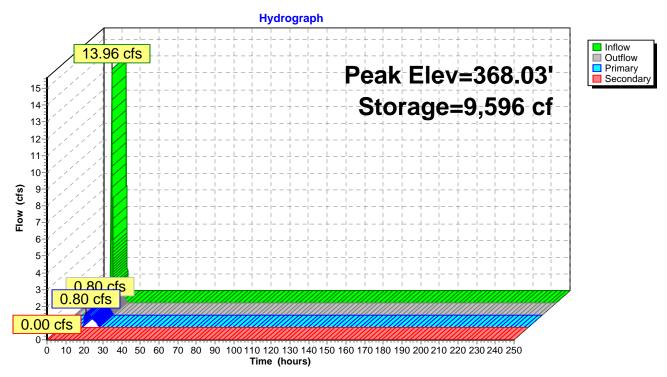
Center-of-Mass det. time= 152.3 min (931.0 - 778.6)

Volume	Invert	Avail.Sto	rage Storage	Description	
#1	363.00'	39,44	43 cf Custom	n Stage Data (Pi	rismatic)Listed below (Recalc)
Elevatio		ırf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
363.0	00	0	0	0	
364.0	00	1,137	569	569	
366.0	00	2,172	3,309	3,878	
368.0	00	3,441	5,613	9,491	
370.0	00	5,233	8,674	18,165	
372.0	00	7,184	12,417	30,582	
373.0	00	7,500	7,342	37,924	
373.2	20	7,700	1,520	39,443	
Device	Routing	Invert	Outlet Device	s	
#1	Primary	363.00'	10.000 in/hr l	Exfiltration over	r Surface area
#2	Secondary	373.00'	Head (feet)	0.20 0.40 0.60	Broad-Crested Rectangular Weir 0.80 1.00 1.20 1.40 1.60 70 2.69 2.68 2.69 2.67 2.64

Primary OutFlow Max=0.80 cfs @ 13.45 hrs HW=368.03' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.80 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=363.00' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 3P: Detention Pond



Page 106

Summary for Pond 4P: Detention Pond

Inflow Area = 5.751 ac, 33.25% Impervious, Inflow Depth = 2.28" for 10 Year event

Inflow 10.45 cfs @ 12.27 hrs. Volume= 1.093 af

14.62 cfs @ 12.35 hrs, Volume= Outflow 1.093 af, Atten= 0%, Lag= 4.7 min

0.66 cfs @ 12.35 hrs, Volume= Primary 0.738 af 13.96 cfs @ 12.35 hrs, Volume= Secondary = 0.355 af

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 373.53' @ 12.35 hrs Surf.Area= 2,850 sf Storage= 12,557 cf

Plug-Flow detention time= 179.2 min calculated for 1.093 af (100% of inflow)

Center-of-Mass det. time= 179.2 min (1,031.3 - 852.1)

Volume	Invert	Avail.Storage	Storage [Description	
#1	363.00'	12,557 cf	Custom	Stage Data (Pr	ismatic)Listed below (Recalc)
Elevation (feet)	Surf.A (sc		c.Store ic-feet)	Cum.Store (cubic-feet)	
363.00		0	0	0	
364.00	4	448	224	224	
366.00	8	393	1,341	1,565	

363.00	0	0	0
364.00	448	224	224
366.00	893	1,341	1,565
368.00	1,459	2,352	3,917
370.00	2,095	3,554	7,471
372.00	2,739	4,834	12,305
372.09	2,850	252	12,557

Device	Routing	Invert	Outlet Devices
#1	Primary	363.00'	10.000 in/hr Exfiltration over Surface area
#2	Device 3	372.00'	54.0' long x 1.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00
			Coef. (English) 2.62 2.64 2.64 2.68 2.75 2.86 2.92 3.07 3.07
			3.03 3.28 3.32
#3	Secondary	371.00'	Custom Weir/Orifice, Cv= 2.62 (C= 3.28)
	•		Head (feet) 0.00 1.00
			Width (feet) 2.00 2.00

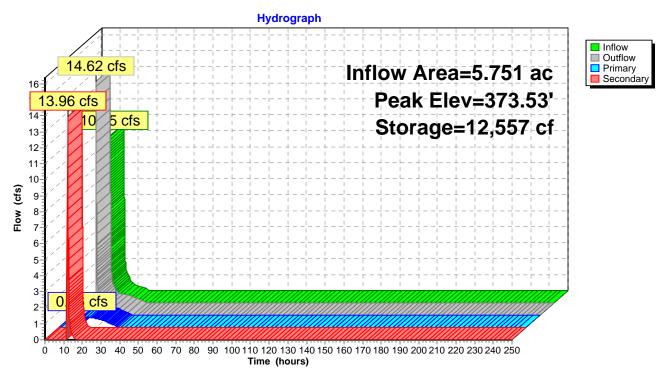
Primary OutFlow Max=0.66 cfs @ 12.35 hrs HW=373.53' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.66 cfs)

Secondary OutFlow Max=13.96 cfs @ 12.35 hrs HW=373.53' (Free Discharge)

-3=Custom Weir/Orifice (Orifice Controls 13.96 cfs @ 6.98 fps)

—2=Broad-Crested Rectangular Weir (Passes 13.96 cfs of 308.61 cfs potential flow)

Pond 4P: Detention Pond



Post-Development3

Type III 24-hr 10 Year Rainfall=5.00"

Prepared by Petruccelli Engineering

Printed 11/1/2010

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Page 108

Summary for Pond 6P: Drywells

Inflow Area =	0.231 ac,100.00% Impervious, Inflow D	epth = 4.76" for 10 Year event
Inflow =	0.65 cfs @ 12.39 hrs, Volume=	0.092 af
Outflow =	0.20 cfs @ 13.00 hrs, Volume=	0.092 af, Atten= 69%, Lag= 36.4 min
Primary =	0.20 cfs @ 13.00 hrs, Volume=	0.092 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 387.49' @ 13.00 hrs Surf.Area= 1,218 sf Storage= 951 cf

Plug-Flow detention time= 29.2 min calculated for 0.092 af (100% of inflow) Center-of-Mass det. time= 29.2 min (799.4 - 770.2)

Volume	Invert	Avail.Storage	Storage Description
#1A	386.00'	1,291 cf	27.68'W x 44.00'L x 4.83'H Field A
			5,883 cf Overall - 2,655 cf Embedded = 3,227 cf x 40.0% Voids
#2A	387.00'	2,058 cf	Dry_Well 1000 Gallon x 16 Inside #1
			Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf
			Outside= 68.0"W x 34.0"H => 15.81 sf x 10.50'L = 166.0 cf
		3,349 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	386.00'	6.000 in/hr Exfiltration over Wetted area
#2	Secondary	393.00'	24.0" Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=0.20 cfs @ 13.00 hrs HW=387.49' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.20 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=386.00' (Free Discharge) 2=Orifice/Grate (Controls 0.00 cfs)

Page 109

Pond 6P: Drywells - Chamber Wizard Field A

Chamber Model = Dry_Well 1000 Gallon

Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf Outside= 68.0"W x 34.0"H => 15.81 sf x 10.50'L = 166.0 cf

68.0" Wide + 12.0" Spacing = 80.0" C-C

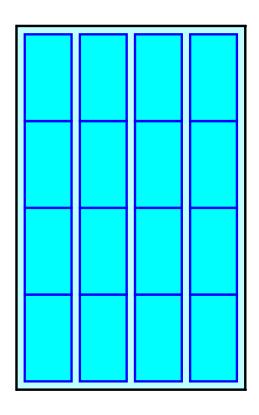
4 Chambers/Row x 10.50' Long = 42.00' + 12.0" End Stone x 2 = 44.00' Base Length 4 Rows x 68.0" Wide + 12.0" Spacing x 3 + 12.0" Side Stone x 2 = 27.68' Base Width 12.0" Base + 34.0" Chamber Height + 12.0" Cover = 4.83' Field Height

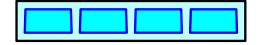
16 Chambers x 128.6 cf = 2,058.4 cf Chamber Storage 16 Chambers x 166.0 cf = 2,655.4 cf Displacement

5,882.6 cf Field - 2,655.4 cf Chambers = 3,227.2 cf Stone x 40.0% Voids = 1,290.9 cf Stone Storage

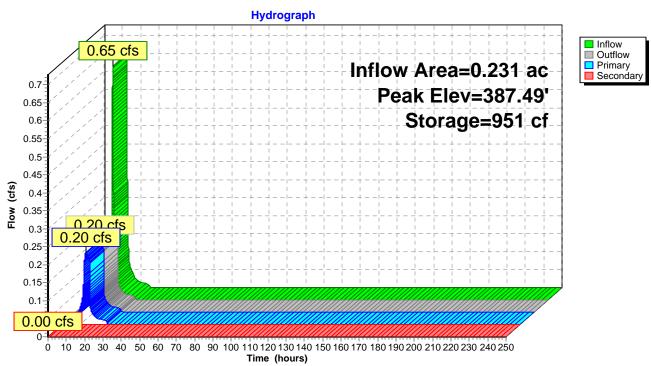
Stone + Chamber Storage = 3,349.3 cf = 0.077 af

16 Chambers 217.9 cy Field 119.5 cy Stone





Pond 6P: Drywells



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Page 111

Summary for Pond 7P: Stormwater Treatment Pond #2

Inflow Area =	2.937 ac, 37.76% Impervious, Inflow D	epth = 2.28" for 10 Year event
Inflow =	4.26 cfs @ 12.46 hrs, Volume=	0.558 af
Outflow =	4.23 cfs @ 12.49 hrs, Volume=	0.558 af, Atten= 1%, Lag= 2.0 min
Primary =	0.20 cfs @ 12.49 hrs, Volume=	0.185 af
Secondary =	4.03 cfs @ 12.49 hrs, Volume=	0.373 af
Tertiary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 364.22' @ 12.49 hrs Surf.Area= 846 sf Storage= 1,443 cf

Plug-Flow detention time= 32.0 min calculated for 0.558 af (100% of inflow)

Center-of-Mass det. time= 32.0 min (896.8 - 864.7)

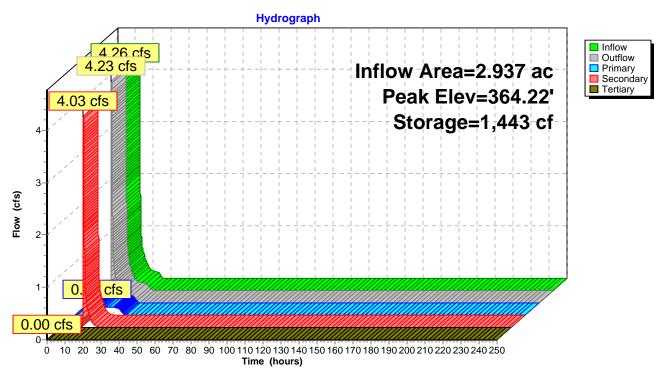
Volume	Invert	t Avail.Sto	rage	Storage	Description	
#1	361.00	2,60	05 cf	Custon	Stage Data (Pi	rismatic)Listed below (Recalc)
□laatia	0		المما	04	O Otana	
Elevation		urf.Area		Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic	-feet)	(cubic-feet)	
361.0	00	100		0	0	
362.0	00	300		200	200	
364.0	00	763	•	1,063	1,263	
365.0	00	1,132		948	2,211	
365.3	30	1,500		395	2,605	
Device	Routing	Invert	Outle	t Device	S	
#1	Primary	361.00'	10.00	0 in/hr	Exfiltration over	r Surface area
#2	Secondary	363.50'	Custo	om Wei	r/Orifice, Cv= 2.	62 (C= 3.28)
	_		Head	(feet) (0.00 1.50	,
			Width	ı (feet) 2	2.00 2.00	
#3	Tertiary	365.10'	93.0'	long x	3.0' breadth Bro	oad-Crested Rectangular Weir
	,			_		0.80 1.00 1.20 1.40 1.60 1.80 2.00
				` ,	50 4.00 4.50	
						68 2.67 2.65 2.64 2.64 2.68 2.68
					92 2.97 3.07 3	
			2.12	Z.O I Z.	92 2.91 3.01 3	.JZ

Primary OutFlow Max=0.20 cfs @ 12.49 hrs HW=364.22' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.20 cfs)

Secondary OutFlow Max=4.03 cfs @ 12.49 hrs HW=364.22' (Free Discharge) 2=Custom Weir/Orifice (Weir Controls 4.03 cfs @ 2.79 fps)

Tertiary OutFlow Max=0.00 cfs @ 0.00 hrs HW=361.00' (Free Discharge) -3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 7P: Stormwater Treatment Pond #2



Volume

Invert

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Page 113

Summary for Pond 8P: Stormwater Treatment Pond #1

Inflow 4.03 cfs @ 12.49 hrs, Volume= 0.373 af 3.37 cfs @ 12.68 hrs. Volume= 0.373 af, Atten= 17%, Lag= 11.3 min Outflow = 0.37 cfs @ 12.68 hrs, Volume= Primary 0.100 af 2.99 cfs @ 12.68 hrs, Volume= Secondary = 0.273 af

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 363.67' @ 12.68 hrs Surf.Area= 1,618 sf Storage= 2,623 cf

Plug-Flow detention time= 15.0 min calculated for 0.373 af (100% of inflow) Center-of-Mass det. time= 15.0 min (815.7 - 800.8)

Avail.Storage Storage Description

volume	IIIVEIL	Avaii.Siu	raye Sibraye	Description	
#1	361.00'	5,80	67 cf Custon	n Stage Data (P	rismatic)Listed below (Recalc)
	Elevation Surf.Area		Inc.Store	Cum.Store	
(fee	et)	(sq-ft)	(cubic-feet)	(cubic-feet)	
361.0	00	300	0	0	
362.00 842		842	571	571	
364.0	00	1,772	2,614	3,185	
365.0	00	2,163	1,968	5,153	
365.30 2,600		2,600	714	5,867	
Device	Routing	Invert	Outlet Device	es	
#1	Secondary	361.00'	24.0" Round	d Culvert	
					conforming to fill, Ke= 0.500
			Inlet / Outlet	Invert= 361.00' /	330.00' S= 0.1550 '/' Cc= 0.900
				rrugated PE, sm	
#2	Device 1	361.10'		ifice/Grate (0 yr	
#3	Device 1	361.70'		ifice/Grate (1yr)	
#4	Device 1	362.30'		ifice/Grate(2yr)	
#5	Device 1	363.40'		ifice/Grate(10yr	
#6	Device 1	364.70'			Riser (100yr) C= 0.600
				eir flow at low hea	
#7	Device 1	365.00'			oad-Crested Rectangular Weir (14.5)
			` '		0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.		
					61 2.60 2.66 2.70 2.77 2.89 2.88
			2.85 3.07 3.		
#8	Device 1	365.20'			road-Crested Rectangular Weir
					0.80 1.00 1.20 1.40 1.60 1.80 2.00
				.50 4.00 4.50	00 0 07 0 05 0 04 0 04 0 00 0 00
					68 2.67 2.65 2.64 2.64 2.68 2.68
40	Duine out	204.00		.92 2.97 3.07 3	
#9	Primary	361.00'	าบ.บบบ in/hr	Exfiltration ove	r Surrace area

Primary OutFlow Max=0.37 cfs @ 12.68 hrs HW=363.67' (Free Discharge) 9=Exfiltration (Exfiltration Controls 0.37 cfs)

Secondary OutFlow Max=2.99 cfs @ 12.68 hrs HW=363.67' (Free Discharge)

-1=Culvert (Passes 2.99 cfs of 19.54 cfs potential flow)

-2=Orifice/Grate (0 yr) (Orifice Controls 0.17 cfs @ 7.59 fps)

-3=Orifice/Grate (1yr) (Orifice Controls 1.24 cfs @ 6.31 fps)

-4=Orifice/Grate(2yr) (Orifice Controls 1.34 cfs @ 5.00 fps)

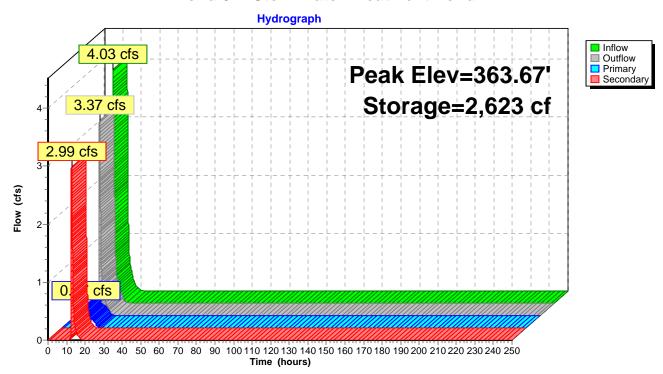
-5=Orifice/Grate(10yr) (Orifice Controls 0.25 cfs @ 1.76 fps)

-6=Top of Riser (100yr) (Controls 0.00 cfs)

-7=Broad-Crested Rectangular Weir (14.5) (Controls 0.00 cfs)

-8=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 8P: Stormwater Treatment Pond #1



Post-Development3

Type III 24-hr 10 Year Rainfall=5.00"

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Page 115

Summary for Pond 9P: Drywells

Inflow Area =	0.459 ac,100.00% Impervious, Inflow D	Depth = 4.76" for 10 Year event
Inflow =	1.56 cfs @ 12.25 hrs, Volume=	0.182 af
Outflow =	0.25 cfs @ 11.72 hrs, Volume=	0.182 af, Atten= 84%, Lag= 0.0 min
Primary =	0.25 cfs @ 11.72 hrs, Volume=	0.182 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 388.13' @ 12.99 hrs Surf.Area= 1,834 sf Storage= 2,457 cf

Plug-Flow detention time= 61.9 min calculated for 0.182 af (100% of inflow) Center-of-Mass det. time= 61.9 min (822.1 - 760.2)

Volume	Invert	Avail.Storage	Storage Description
#1A	386.00'	1,685 cf	41.69'W x 44.00'L x 4.83'H Field A
			8,860 cf Overall - 4,647 cf Embedded = 4,213 cf \times 40.0% Voids
#2A	387.00'	3,602 cf	Dry_Well 1000 Gallon x 28 Inside #1
			Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf
			Outside= 68.0"W x 34.0"H => 15.81 sf x 10.50'L = 166.0 cf
		5,287 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	386.00'	6.000 in/hr Exfiltration over Surface area
#2	Secondary	392.00'	24.0" Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=0.25 cfs @ 11.72 hrs HW=386.08' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.25 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=386.00' (Free Discharge) 2=Orifice/Grate (Controls 0.00 cfs)

Page 116

Pond 9P: Drywells - Chamber Wizard Field A

Chamber Model = Dry_Well 1000 Gallon

Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf Outside= 68.0"W x 34.0"H => 15.81 sf x 10.50'L = 166.0 cf

68.0" Wide + 0.0" Spacing = 68.0" C-C

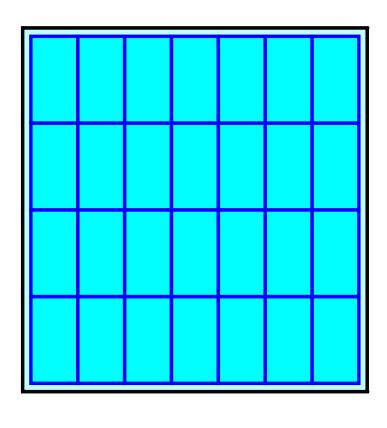
4 Chambers/Row x 10.50' Long = 42.00' + 12.0'' End Stone x 2 = 44.00' Base Length 7 Rows x 68.0'' Wide + 12.0'' Side Stone x 2 = 41.69' Base Width 12.0'' Base + 34.0'' Chamber Height + 12.0'' Cover = 4.83' Field Height

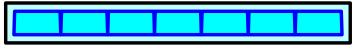
28 Chambers x 128.6 cf = 3,602.2 cf Chamber Storage 28 Chambers x 166.0 cf = 4,646.9 cf Displacement

8,860.0 cf Field - 4,646.9 cf Chambers = 4,213.1 cf Stone x 40.0% Voids = 1,685.2 cf Stone Storage

Stone + Chamber Storage = 5,287.4 cf = 0.121 af

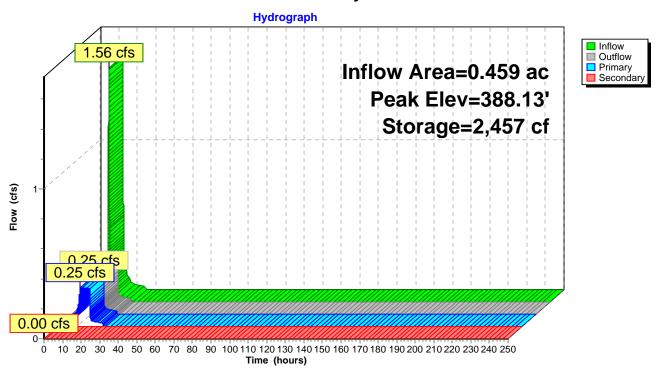
28 Chambers 328.1 cy Field 156.0 cy Stone





Page 117

Pond 9P: Drywells



Summary for Link DP-1: Design Point 1

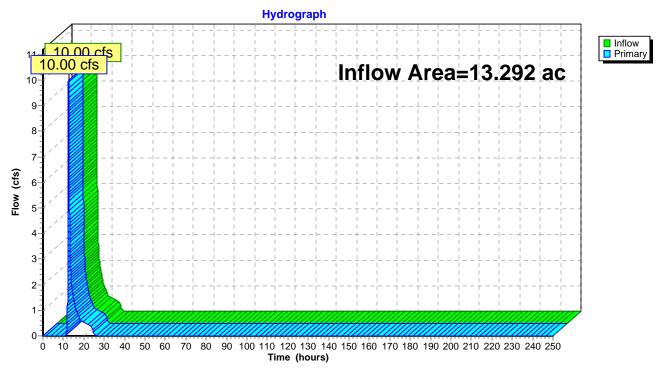
Inflow Area = 13.292 ac, 3.27% Impervious, Inflow Depth = 1.30" for 10 Year event

Inflow = 10.00 cfs @ 12.50 hrs, Volume= 1.441 af

Primary = 10.00 cfs @ 12.50 hrs, Volume= 1.441 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-1: Design Point 1



Summary for Link DP-2: Design Point 2

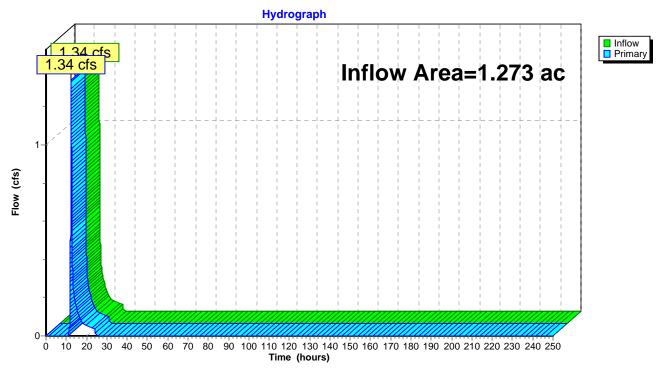
Inflow Area = 1.273 ac, 7.29% Impervious, Inflow Depth = 1.51" for 10 Year event

Inflow = 1.34 cfs @ 12.34 hrs, Volume= 0.160 af

Primary = 1.34 cfs @ 12.34 hrs, Volume= 0.160 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-2: Design Point 2



Summary for Link DP-3: Design Point 3

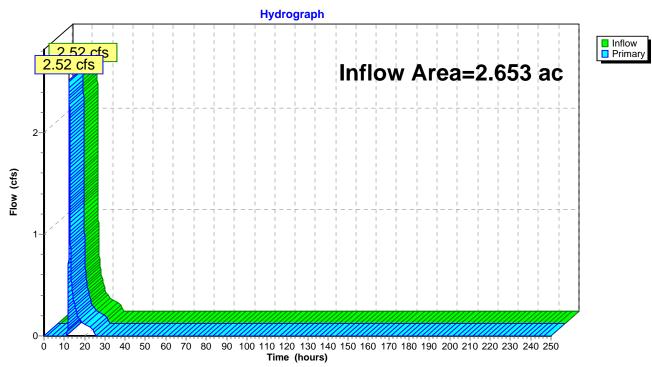
Inflow Area = 2.653 ac, 7.97% Impervious, Inflow Depth = 1.51" for 10 Year event

Inflow = 2.52 cfs @ 12.43 hrs, Volume= 0.334 af

Primary = 2.52 cfs @ 12.43 hrs, Volume= 0.334 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-3: Design Point 3



Summary for Link DP-4: Design Point 4

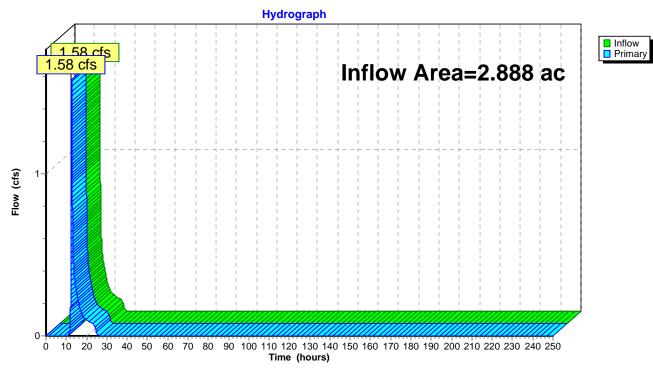
Inflow Area = 2.888 ac, 0.56% Impervious, Inflow Depth = 1.04" for 10 Year event

Inflow = 1.58 cfs @ 12.55 hrs, Volume= 0.251 af

Primary = 1.58 cfs @ 12.55 hrs, Volume= 0.251 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-4: Design Point 4



Summary for Link DP-5: Design Point 5

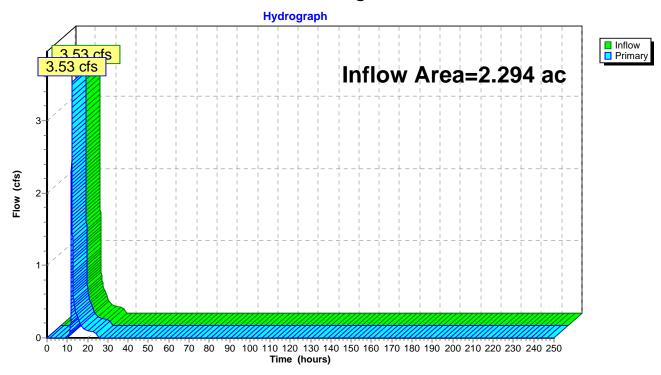
Inflow Area = 2.294 ac, 15.79% Impervious, Inflow Depth = 1.73" for 10 Year event

Inflow = 3.53 cfs @ 12.19 hrs, Volume= 0.330 af

Primary = 3.53 cfs @ 12.19 hrs, Volume= 0.330 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-5: Design Point 5



Summary for Link DP-6: Design Point 6

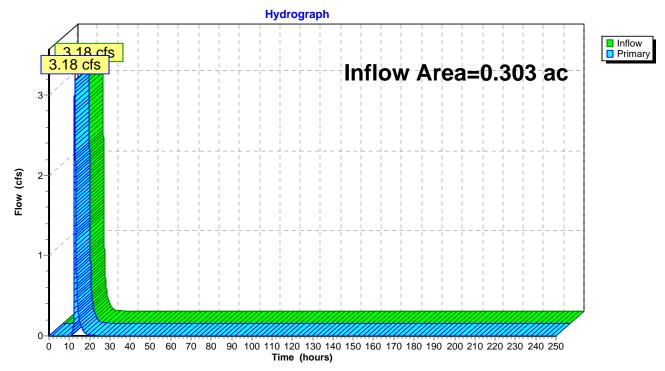
Inflow Area = 0.303 ac, 0.00% Impervious, Inflow Depth = 12.11" for 10 Year event

Inflow = 3.18 cfs @ 12.67 hrs, Volume= 0.306 af

Primary = 3.18 cfs @ 12.67 hrs, Volume= 0.306 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-6: Design Point 6



Summary for Link DP-7: Design Point 7

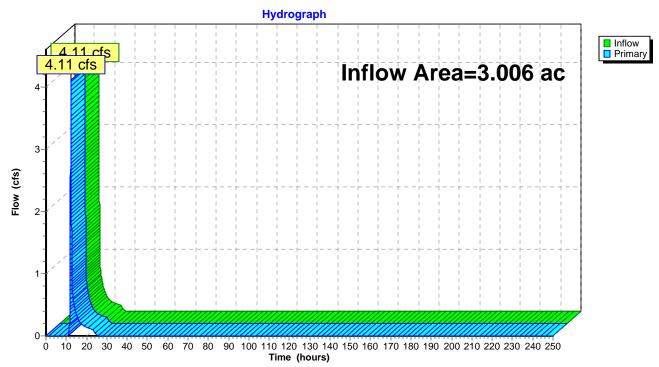
Inflow Area = 3.006 ac, 9.68% Impervious, Inflow Depth = 1.58" for 10 Year event

Inflow = 4.11 cfs @ 12.20 hrs, Volume= 0.396 af

Primary = 4.11 cfs @ 12.20 hrs, Volume= 0.396 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-7: Design Point 7



Printed 11/1/2010 Page 125

Time span=0.00-250.00 hrs, dt=0.01 hrs, 25001 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Stor-Ind method - Pond routing by Stor-Ind method

3 .,	3 3, 23
Subcatchment 1: Drainage Area 1	Runoff Area=13.292 ac 3.27% Impervious Runoff Depth=2.96" Flow Length=1,455' Tc=31.8 min CN=60 Runoff=24.82 cfs 3.282 af
Subcatchment 2A: Drainage Area 2A	Runoff Area=5.751 ac 33.25% Impervious Runoff Depth=4.37" Flow Length=999' Tc=19.1 min CN=73 Runoff=20.23 cfs 2.094 af
Subcatchment 2B: Drainage Area 2B	Runoff Area=19,994 sf 100.00% Impervious Runoff Depth=7.26" Flow Length=999' Tc=19.1 min CN=98 Runoff=2.35 cfs 0.278 af
Subcatchment3: Drainage Area 3	Runoff Area=115,560 sf 7.97% Impervious Runoff Depth=3.28" Flow Length=552' Tc=28.6 min CN=63 Runoff=5.81 cfs 0.725 af
Subcatchment 4: Drainage Area 4	Runoff Area=55,457 sf 7.29% Impervious Runoff Depth=3.28" Flow Length=284' Tc=22.5 min CN=63 Runoff=3.10 cfs 0.348 af
Subcatchment 5: Drainage Area 5	Runoff Area=125,820 sf 0.56% Impervious Runoff Depth=2.55" Flow Length=497' Tc=33.7 min CN=56 Runoff=4.41 cfs 0.614 af
Subcatchment 6A: Drainage Area 6	Runoff Area=2.937 ac 37.76% Impervious Runoff Depth=4.37" Flow Length=578' Tc=32.3 min CN=73 Runoff=8.24 cfs 1.069 af
Subcatchment 6B: Drainage Area 6	Runoff Area=10,050 sf 100.00% Impervious Runoff Depth=7.26" Flow Length=540' Tc=29.9 min CN=98 Runoff=0.98 cfs 0.140 af
Subcatchment 6c: Drainage Area 6	Runoff Area=13,208 sf 0.00% Impervious Runoff Depth=2.96" Flow Length=540' Tc=29.9 min CN=60 Runoff=0.58 cfs 0.075 af
Subcatchment 7: Drainage Area 7	Runoff Area=3.006 ac 9.68% Impervious Runoff Depth=3.39" Flow Length=527' Tc=13.4 min CN=64 Runoff=9.33 cfs 0.848 af
Subcatchment 8: Drainage Area 8	Runoff Area=99,910 sf 15.79% Impervious Runoff Depth=3.60" Flow Length=558' Tc=13.0 min CN=66 Runoff=7.69 cfs 0.688 af
Reach 1R: 18" 18.0" Round Pipe n=0.013	Avg. Flow Depth=0.60' Max Vel=12.52 fps Inflow=8.24 cfs 1.069 af L=132.0' S=0.0546 '/' Capacity=24.55 cfs Outflow=8.24 cfs 1.069 af
Pond 1P: Pond - D Primary=8.24	Peak Elev=369.05' Inflow=8.24 cfs 1.069 af cfs 1.069 af Secondary=0.00 cfs 0.000 af Outflow=8.24 cfs 1.069 af
Pond 3P: Detention Pond Primary=1.69	Peak Elev=372.43' Storage=33,702 cf Inflow=24.06 cfs 1.187 af cfs 1.187 af Secondary=0.00 cfs 0.000 af Outflow=1.69 cfs 1.187 af
Pond 4P: Detention Pond Primary=0.66 cf	Peak Elev=377.50' Storage=12,557 cf Inflow=20.23 cfs 2.094 af s 0.907 af Secondary=24.06 cfs 1.187 af Outflow=24.72 cfs 2.094 af
Pond 6P: Drywells Primary=0.22	Peak Elev=388.40' Storage=1,796 cf Inflow=0.98 cfs 0.140 af 2 cfs 0.140 af Secondary=0.00 cfs 0.000 af Outflow=0.22 cfs 0.140 af

Post-Devel	lopment3
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Type III 24-hr 100 Year Rainfall=7.50"

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Page 126

Pond 7P: Stormwater Treatment Pond #2 Peak Elev=364.64' Storage=1,826 cf Inflow=8.24 cfs 1.069 af Primary=0.23 cfs 0.219 af Secondary=7.96 cfs 0.851 af Tertiary=0.00 cfs 0.000 af Outflow=8.19 cfs 1.069 af

Pond 8P: Stormwater Treatment Pond #1 Peak Elev=364.77' Storage=4,670 cf Inflow=7.96 cfs 0.851 af Primary=0.48 cfs 0.183 af Secondary=7.05 cfs 0.668 af Outflow=7.53 cfs 0.851 af

Pond 9P: Drywells Peak Elev=389.44' Storage=4,437 cf Inflow=2.35 cfs 0.278 af

Primary=0.25 cfs 0.278 af Secondary=0.00 cfs 0.000 af Outflow=0.25 cfs 0.278 af

Link DP-1: Design Point 1 Inflow=24.82 cfs 3.282 af Primary=24.82 cfs 3.282 af

Link DP-2: Design Point 2 Inflow=3.10 cfs 0.348 af

Link DP-3: Design Point 3 Inflow=5.81 cfs 0.725 af

Link DP-3: Design Point 3 Inflow=5.81 cfs 0.725 af Primary=5.81 cfs 0.725 af

Link DP-4: Design Point 4 Inflow=4.41 cfs 0.614 af

Primary=4.41 cfs 0.614 af

Link DP-5: Design Point 5 Inflow=7.69 cfs 0.688 af

Primary=7.69 cfs 0.688 af

Primary=3.10 cfs 0.348 af

Link DP-6: Design Point 6 Inflow=7.58 cfs 0.743 af

Primary=7.58 cfs 0.743 af

Link DP-7: Design Point 7 Inflow=9.33 cfs 0.848 af

Primary=9.33 cfs 0.848 af

Total Runoff Area = 35.087 ac Runoff Volume = 10.162 af Average Runoff Depth = 3.48" 85.41% Pervious = 29.968 ac 14.59% Impervious = 5.119 ac Prepared by Petruccelli Engineering

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Page 127

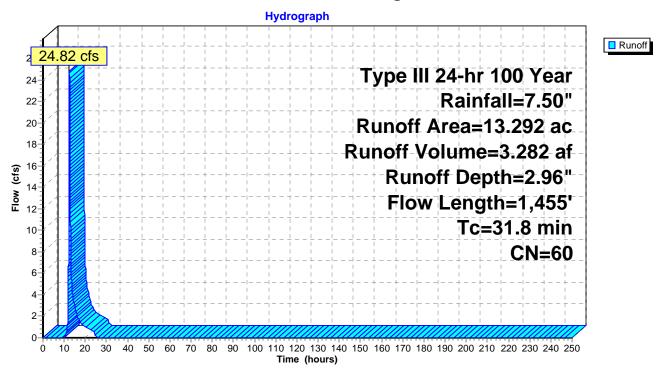
Summary for Subcatchment 1: Drainage Area 1

Runoff = 24.82 cfs @ 12.47 hrs, Volume= 3.282 af, Depth= 2.96"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 100 Year Rainfall=7.50"

Area	(ac) C	N Desc	cription					
2.770 69 50-75% Grass cover, Fair, HSG B								
0.435 98 Paved roads w/curbs & sewers								
9	9.824 55 Woods, Good, HSG B							
0.263 82 Dirt roads, HSG B								
13	.292 6	60 Weig	ghted Aver	age				
12	.857	96.7	3% Pervio	us Area				
0	.435	3.27	% Impervi	ous Area				
_								
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
18.9	100	0.0260	0.09		Sheet Flow, 1 to 2			
4 7	040	0.4000	0.44		Grass: Bermuda n= 0.410 P2= 3.50"			
1.7	216	0.1830	2.14		Shallow Concentrated Flow, 2 to 3			
0.3	78	0.0450	1 21		Woodland Kv= 5.0 fps			
0.3	10	0.0450	4.31		Shallow Concentrated Flow, 3 to 4 Paved Kv= 20.3 fps			
1.2	121	0.1150	1.70		Shallow Concentrated Flow, 4 to 5			
1.2	121	0.1150	1.70		Woodland Kv= 5.0 fps			
5.8	679	0.0770	1.94		Shallow Concentrated Flow, 5 to 6			
0.0	0.0	0.01.0			Short Grass Pasture Kv= 7.0 fps			
3.9	261	0.0500	1.12		Shallow Concentrated Flow, 6 to DP1			
					Woodland Kv= 5.0 fps			
31.8	1,455	Total						

Subcatchment 1: Drainage Area 1



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Page 129

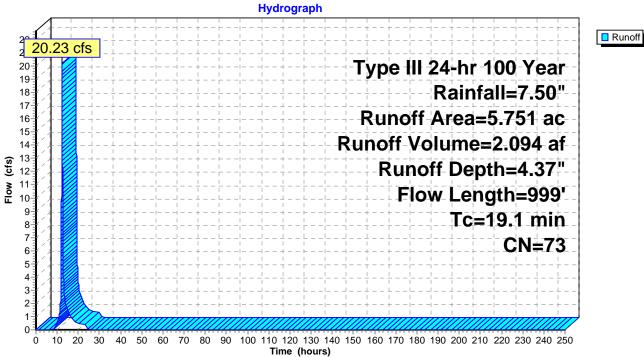
Summary for Subcatchment 2A: Drainage Area 2A

Runoff = 20.23 cfs @ 12.26 hrs, Volume= 2.094 af, Depth= 4.37"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 100 Year Rainfall=7.50"

Area	(ac) C	N Desc	cription		
	-			/curbs & se	
			% Grass co ds, Good,	over, Good, HSG B	, H2G B
_			ghted Aver		
	.839		5% Pervio		
1.	.912	33.2	5% Imper	ious Area	
То	Longth	Clone	\/olooity	Consoity	Description
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.5	100	0.0900	0.15	(0.0)	Sheet Flow, 1 to 2
_					Grass: Bermuda n= 0.410 P2= 3.50"
5.8	423	0.0300	1.21		Shallow Concentrated Flow, 2 to 3
0.5	7.4	0.04.40	0.40		Short Grass Pasture Kv= 7.0 fps
0.5	74	0.0140	2.40		Shallow Concentrated Flow, 3 to 4 (Road) Paved Kv= 20.3 fps
0.1	25	0.0100	4.54	3.56	
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
					n= 0.013 Corrugated PE, smooth interior
0.6	167	0.0120	4.97	3.90	
					12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25' n= 0.013 Corrugated PE, smooth interior
0.4	70	0.0090	2.60	3.19	
• • • • • • • • • • • • • • • • • • • •	. •	0.000		00	15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
					n= 0.025 Corrugated metal
0.2	140	0.1510	12.01	21.23	Pipe Channel, 7 to 8
					18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'
10.4	000	Total			n= 0.025 Corrugated metal
19.1	999	Total			

Subcatchment 2A: Drainage Area 2A





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Page 131

Summary for Subcatchment 2B: Drainage Area 2B

Runoff = 2.35 cfs @ 12.25 hrs, Volume= 0.278 af, Depth= 7.26"

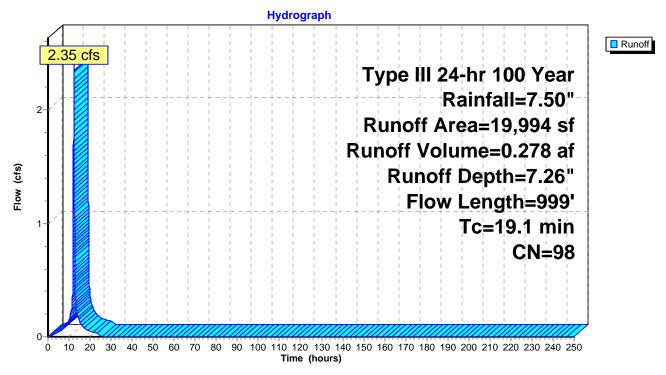
Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 100 Year Rainfall=7.50"

А	rea (sf)	CN E	Description		
	19,994	98 F	Paved road	s w/curbs 8	& sewers
	19,994	1	00.00% In	pervious A	ırea
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
11.5	100	0.0900	0.15	(013)	Sheet Flow, 1 to 2
5.8	423	0.0300	1.21		Grass: Bermuda n= 0.410 P2= 3.50" Shallow Concentrated Flow, 2 to 3
0.5	74	0.0140	2.40		Short Grass Pasture Kv= 7.0 fps Shallow Concentrated Flow, 3 to 4 (Road) Paved Kv= 20.3 fps
0.1	25	0.0100	4.54	3.56	Pipe Channel, 4 to 5 12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
0.6	167	0.0120	4.97	3.90	12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25'
0.4	70	0.0090	2.60	3.19	15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
0.2	140	0.1510	12.01	21.23	n= 0.025 Corrugated metal Pipe Channel, 7 to 8 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.025 Corrugated metal
19.1	999	Total			

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Page 132

Subcatchment 2B: Drainage Area 2B



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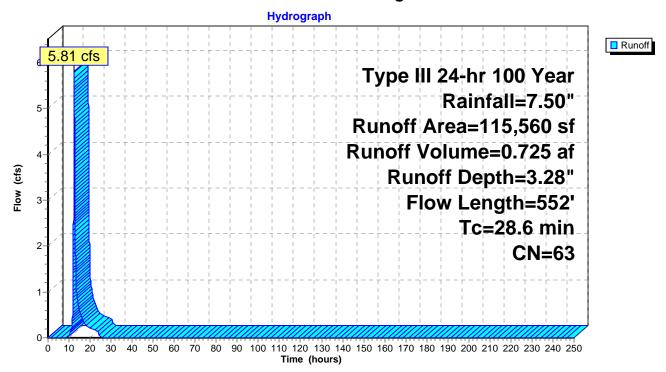
Summary for Subcatchment 3: Drainage Area 3

Runoff = 5.81 cfs @ 12.42 hrs, Volume= 0.725 af, Depth= 3.28"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 100 Year Rainfall=7.50"

A	rea (sf)	CN E	escription				
	49,852 61 >75% Grass cover, Good, HSG B						
	56,494	60 V	Voods, Fai	r, HSG B			
	9,214	98 F	aved road	s w/curbs &	R sewers		
1	15,560	63 V	Veighted A	verage			
1	06,346	9	2.03% Per	vious Area			
	9,214	7	.97% Impe	ervious Area	a		
Tc	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
19.4	118	0.0320	0.10		Sheet Flow, 1 to 2		
					Woods: Light underbrush n= 0.400 P2= 3.50"		
2.5	87	0.0140	0.59		Shallow Concentrated Flow, 2 to 3		
					Woodland Kv= 5.0 fps		
1.2	159	0.1940	2.20		Shallow Concentrated Flow, 3 to 4		
					Woodland Kv= 5.0 fps		
5.5	188	0.0130	0.57		Shallow Concentrated Flow, 4 to DP 3		
					Woodland Kv= 5.0 fps		
28.6	552	Total					

Subcatchment 3: Drainage Area 3



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Page 134

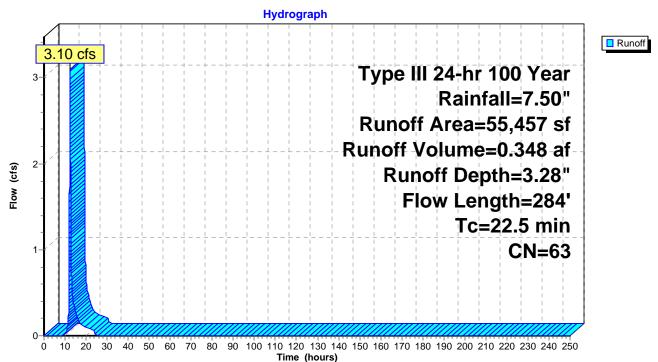
Summary for Subcatchment 4: Drainage Area 4

Runoff = 3.10 cfs @ 12.32 hrs, Volume= 0.348 af, Depth= 3.28"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 100 Year Rainfall=7.50"

	Α	rea (sf)	CN I	Description		
		31,781	60 '	Noods, Fai	r, HSG B	
		4,042	98	Paved road	s w/curbs &	& sewers
_		19,634	61 :	>75% Gras	s cover, Go	ood, HSG B
		55,457	63 \	Neighted A	verage	
		51,415	,	92.71% Pei	rvious Area	
		4,042	•	7.29% Impe	ervious Are	a
	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	20.9	207	0.0860	0.16		Sheet Flow, 1 to 2
						Grass: Bermuda n= 0.410 P2= 3.50"
	1.6	77	0.0130	0.80		Shallow Concentrated Flow, 2 to DP 4
_						Short Grass Pasture Kv= 7.0 fps
	22.5	284	Total			

Subcatchment 4: Drainage Area 4



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Page 135

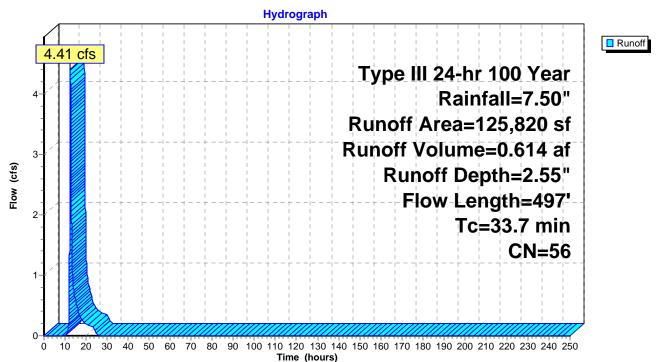
Summary for Subcatchment 5: Drainage Area 5

Runoff = 4.41 cfs @ 12.51 hrs, Volume= 0.614 af, Depth= 2.55"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 100 Year Rainfall=7.50"

	Α	rea (sf)	CN E	escription			
		23,735	61 >	75% Gras	s cover, Go	ood, HSG B	
	1	01,385	55 V	Voods, Go	od, HSG B		
		700	98 F	aved park	ing, HSG B		
	1	25,820	56 V	Veighted A	verage		
	1	25,120	9	9.44% Per	rvious Area		
		700	0	.56% Impe	ervious Area	a	
				-			
	Tc	Length	Slope	Velocity	Capacity	Description	
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
_		•		,		Description Sheet Flow, 1 to 2	
_	(min)	(feet)	(ft/ft)	(ft/sec)		·	
_	(min)	(feet)	(ft/ft)	(ft/sec)		Sheet Flow, 1 to 2	
_	(min) 29.9	(feet) 135	(ft/ft) 0.0150	(ft/sec) 0.08		Sheet Flow, 1 to 2 Grass: Bermuda n= 0.410 P2= 3.50"	

Subcatchment 5: Drainage Area 5



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Summary for Subcatchment 6A: Drainage Area 6

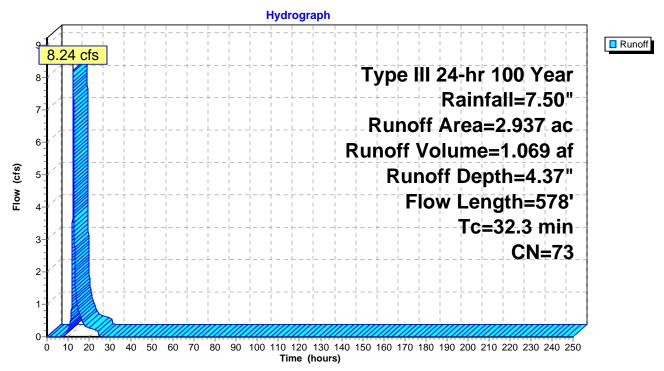
Runoff = 8.24 cfs @ 12.45 hrs, Volume= 1.069 af, Depth= 4.37"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 100 Year Rainfall=7.50"

Area	(ac) C	N Desc	cription					
0.893 55 Woods, Good, HSG B								
1.	.109 9	8 Pave	ed roads w	/curbs & se	ewers			
0	.935 6	31 >75°	% Grass co	over, Good	, HSG B			
2.	.937 7	'3 Weig	ghted Aver	age				
1.	.828	62.2	4% Pervio	us Area				
1.	.109	37.7	6% Imperv	∕ious Area				
Tc	Length	Slope	Velocity	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
21.6	185	0.0600	0.14		Sheet Flow, 1 to 2			
					Woods: Light underbrush n= 0.400 P2= 3.50"			
0.5	214	0.1180	6.97		Shallow Concentrated Flow, 2 to 3			
					Paved Kv= 20.3 fps			
0.0	17	0.0120	6.25	7.67	Pipe Channel, 3 to 4 (Catchbasins)			
					15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'			
0.4	400	0.0077	5.00	0.00	n= 0.012 HDPE			
0.4	132	0.0077	5.22	9.22	Pipe Channel, 4 to 5 (18" Culvert)			
					18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'			
0.0	20	0.0400	0.05		n= 0.013 HDPE			
9.8	30	0.0120	0.05		Sheet Flow, 5 to DP 6			
		T			Grass: Bermuda n= 0.410 P2= 3.50"			
32.3	578	Total						

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Subcatchment 6A: Drainage Area 6



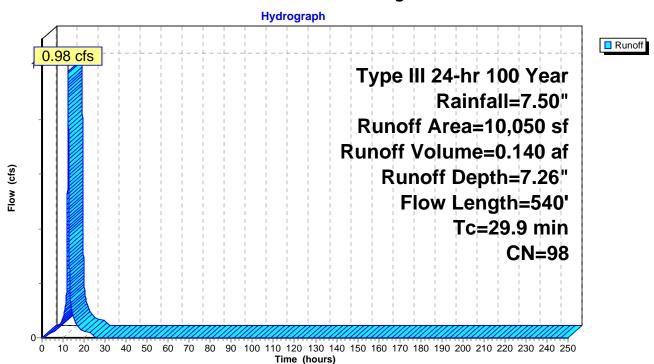
Summary for Subcatchment 6B: Drainage Area 6

Runoff = 0.98 cfs @ 12.39 hrs, Volume= 0.140 af, Depth= 7.26"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 100 Year Rainfall=7.50"

	^	"" (°t)	CN1 1			
_	A	rea (sf)	CN [Description		
		10,050	98 F	Paved road	s w/curbs 8	R sewers
		10,050	•	100.00% In	npervious A	ırea
	Tc (min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description
	21.6	185	0.0600	0.14		Sheet Flow, 1 to 2
						Woods: Light underbrush n= 0.400 P2= 3.50"
	0.5	214	0.1180	6.97		Shallow Concentrated Flow, 2 to 3
						Paved Kv= 20.3 fps
	0.0	17	0.0120	6.25	7.67	
						15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
						n= 0.012 HDPE
	0.1	101	0.1730	24.72	43.69	Pipe Channel, 4 to 5 (18" Culvert)
						18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'
						n= 0.013 HDPE
	7.7	23	0.0130	0.05		Sheet Flow, 5 to DP 6
						Grass: Bermuda n= 0.410 P2= 3.50"
	29.9	540	Total			

Subcatchment 6B: Drainage Area 6



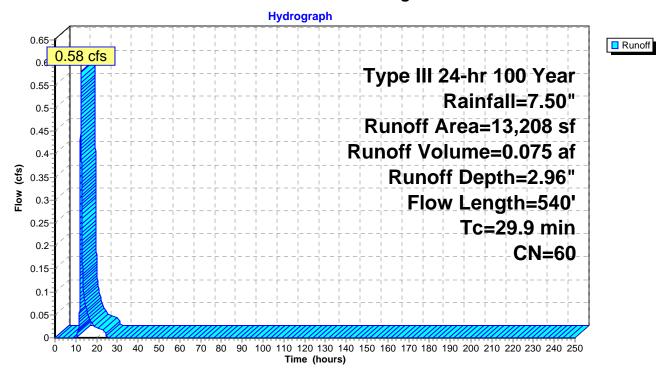
Summary for Subcatchment 6c: Drainage Area 6

Runoff = 0.58 cfs @ 12.45 hrs, Volume= 0.075 af, Depth= 2.96"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 100 Year Rainfall=7.50"

_	Α	rea (sf)	CN	Description		
Ī		13,208	60	Woods, Fai	r, HSG B	
		13,208		100.00% Pe	ervious Are	a
	Tc (min)	Length (feet)	Slope (ft/ft)	•	Capacity (cfs)	Description
	21.6	185	0.0600	0.14		Sheet Flow, 1 to 2 Woods: Light underbrush n= 0.400 P2= 3.50"
	0.5	214	0.1180	6.97		Shallow Concentrated Flow, 2 to 3 Paved Kv= 20.3 fps
	0.0	17	0.0120	6.25	7.67	Pipe Channel, 3 to 4 (Catchbasins) 15.0" Round Area= 1.2 sf Perim= 3.9' r= 0.31'
	0.1	101	0.1730	24.72	43.69	n= 0.012 HDPE Pipe Channel, 4 to 5 (18" Culvert) 18.0" Round Area= 1.8 sf Perim= 4.7' r= 0.38'
_	7.7	23	0.0130	0.05		n= 0.013 HDPE Sheet Flow, 5 to DP 6 Grass: Bermuda n= 0.410 P2= 3.50"
	29.9	540	Total			

Subcatchment 6c: Drainage Area 6



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Page 140

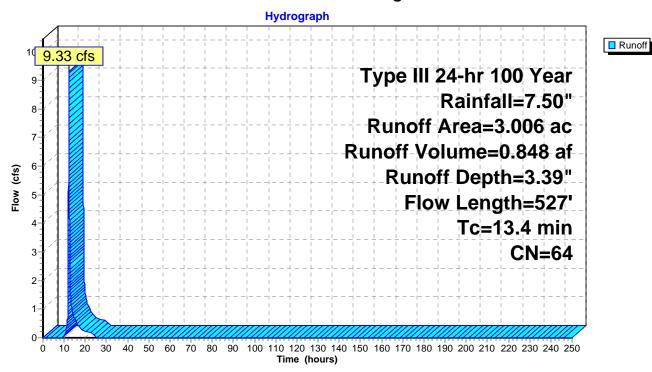
Summary for Subcatchment 7: Drainage Area 7

Runoff = 9.33 cfs @ 12.19 hrs, Volume= 0.848 af, Depth= 3.39"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 100 Year Rainfall=7.50"

Area	(ac) C	N Desc	cription		
-	` /		ds, Fair, H	ISG B	
				/curbs & se	DWArc
_				over, Good.	
					, 1100 D
		•	ghted Aver	•	
2.	715		2% Pervio		
0.	291	9.68	% Impervi	ous Area	
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	· ·
10.1	117	0.1620	0.19		Sheet Flow, 1 to 2
					Woods: Light underbrush n= 0.400 P2= 3.50"
0.7	100	0.2300	2.40		Shallow Concentrated Flow, 2 to 3
0.7	100	0.2000	2.10		Woodland Kv= 5.0 fps
1.2	164	0.2010	2.24		Shallow Concentrated Flow, 3 to 4
1.2	104	0.2010	2.24		•
4.4	4.40	0.4000	4 75		Woodland Kv= 5.0 fps
1.4	146	0.1230	1.75		Shallow Concentrated Flow, 3 to DP 7
					Woodland Kv= 5.0 fps
13.4	527	Total			

Subcatchment 7: Drainage Area 7



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Page 141

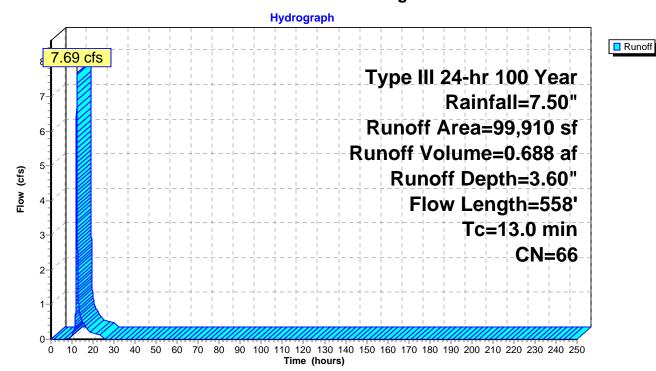
Summary for Subcatchment 8: Drainage Area 8

Runoff = 7.69 cfs @ 12.18 hrs, Volume= 0.688 af, Depth= 3.60"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Type III 24-hr 100 Year Rainfall=7.50"

A	rea (sf)	CN D	escription		
	41,570	60 V	Voods, Fai	r, HSG B	
	15,772	98 P	aved road	s w/curbs &	& sewers
	42,568	61 >	75% Gras	s cover, Go	ood, HSG B
	99,910	66 V	Veighted A	verage	
	84,138	8	4.21% Per	vious Area	
	15,772	1	5.79% lmp	pervious Ar	ea
Tc	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
9.7	100	0.1300	0.17		Sheet Flow, 1 to 2
					Woods: Light underbrush n= 0.400 P2= 3.50"
3.1	362	0.1550	1.97		Shallow Concentrated Flow, 2 to 3
					Woodland Kv= 5.0 fps
0.2	96	0.1150	6.88		Shallow Concentrated Flow, 3 to DP 8
					Paved Kv= 20.3 fps
13.0	558	Total			

Subcatchment 8: Drainage Area 8



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Page 142

Summary for Reach 1R: 18"

Inflow Area = 2.937 ac, 37.76% Impervious, Inflow Depth = 4.37" for 100 Year event

Inflow = 8.24 cfs @ 12.45 hrs, Volume= 1.069 af

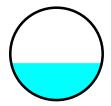
Outflow = 8.24 cfs @ 12.45 hrs, Volume= 1.069 af, Atten= 0%, Lag= 0.2 min

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

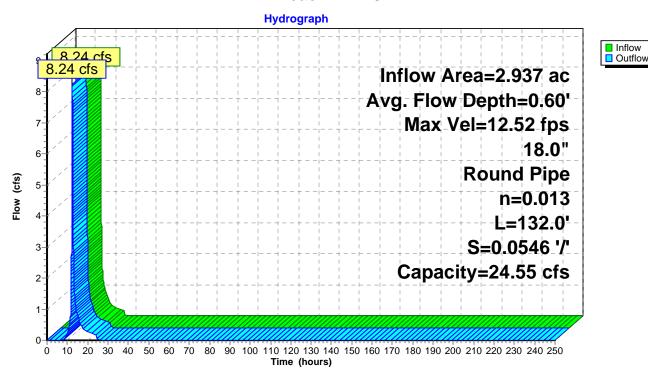
Max. Velocity= 12.52 fps, Min. Travel Time= 0.2 min Avg. Velocity = 5.05 fps, Avg. Travel Time= 0.4 min

Peak Storage= 87 cf @ 12.45 hrs Average Depth at Peak Storage= 0.60' Bank-Full Depth= 1.50', Capacity at Bank-Full= 24.55 cfs

18.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 132.0' Slope= 0.0546 '/' Inlet Invert= 374.01', Outlet Invert= 366.80'



Reach 1R: 18"



Summary for Pond 1P: Pond - D

Inflow Area =	2.937 ac, 37.76% Impervious, Inflow De	epth = 4.37" for 100 Year event
Inflow =	8.24 cfs @ 12.45 hrs, Volume=	1.069 af
Outflow =	8.24 cfs @ 12.45 hrs, Volume=	1.069 af, Atten= 0%, Lag= 0.0 min
Primary =	8.24 cfs @ 12.45 hrs, Volume=	1.069 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

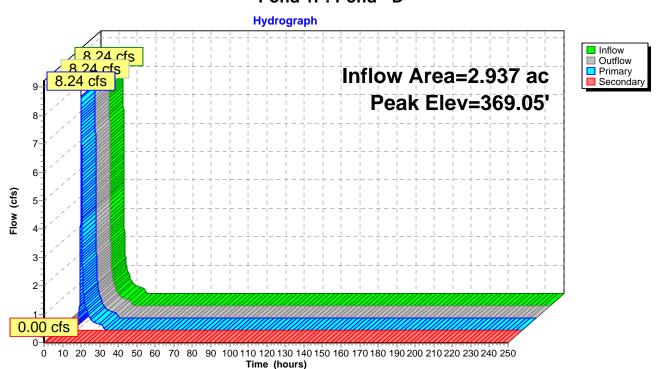
Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 369.05' @ 12.45 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	366.80'	18.0" Round Culvert
	•		L= 6.0' CMP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 366.80' / 361.00' S= 0.9667 '/' Cc= 0.900
			n= 0.025 Corrugated metal
#2	Secondary	371.19'	57.0" W x 57.0" H Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=8.23 cfs @ 12.45 hrs HW=369.05' (Free Discharge) 1=Culvert (Inlet Controls 8.23 cfs @ 4.66 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=366.80' (Free Discharge) 2=Orifice/Grate (Controls 0.00 cfs)

Pond 1P: Pond - D



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Page 144

Summary for Pond 3P: Detention Pond

Inflow = 24.06 cfs @ 12.26 hrs, Volume= 1.187 af

Outflow = 1.69 cfs @ 13.64 hrs, Volume= 1.187 af, Atten= 93%, Lag= 82.5 min

Primary = 1.69 cfs @ 13.64 hrs, Volume= 1.187 af Secondary = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 372.43' @ 13.64 hrs Surf.Area= 7,320 sf Storage= 33,702 cf

Plug-Flow detention time= 255.3 min calculated for 1.187 af (100% of inflow)

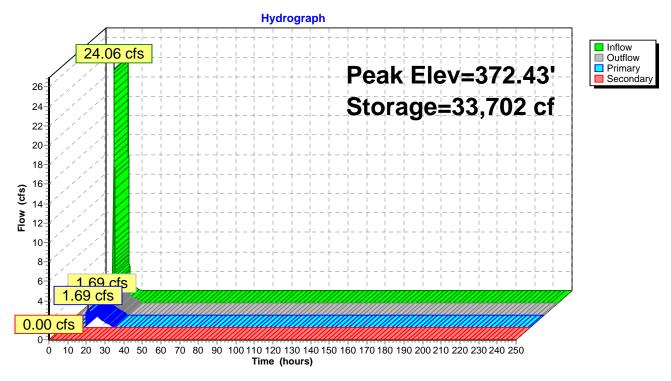
Center-of-Mass det. time= 255.3 min (1,033.8 - 778.5)

Volume	Invert	Avail.Sto	rage Storage	e Description	
#1	363.00'	39,4	43 cf Custor	n Stage Data (Pi	rismatic)Listed below (Recalc)
Elevatio (fee	_	urf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
363.0	0	0	0	0	
364.0	0	1,137	569	569	
366.0	0	2,172	3,309	3,878	
368.0	0	3,441	5,613	9,491	
370.0	0	5,233	8,674	18,165	
372.0	0	7,184	12,417	30,582	
373.0	0	7,500	7,342	37,924	
373.2	0	7,700	1,520	39,443	
Device	Routing	Invert	Outlet Device	es	
#1	Primary	363.00'		Exfiltration over	
#2	Secondary	373.00'	Head (feet)	0.20 0.40 0.60	Broad-Crested Rectangular Weir 0.80 1.00 1.20 1.40 1.60 70 2.69 2.68 2.69 2.67 2.64

Primary OutFlow Max=1.69 cfs @ 13.64 hrs HW=372.43' (Free Discharge) 1=Exfiltration (Exfiltration Controls 1.69 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=363.00' (Free Discharge) 2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 3P: Detention Pond



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Summary for Pond 4P: Detention Pond

Inflow Area = 5.751 ac, 33.25% Impervious, Inflow Depth = 4.37" for 100 Year event

Inflow 20.23 cfs @ 12.26 hrs. Volume= 2.094 af

24.72 cfs @ 12.26 hrs, Volume= Outflow 2.094 af, Atten= 0%, Lag= 0.1 min

0.66 cfs @ 12.08 hrs, Volume= Primary 0.907 af Secondary = 24.06 cfs @ 12.26 hrs, Volume= 1.187 af

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 377.50' @ 12.26 hrs Surf.Area= 2,850 sf Storage= 12,557 cf

Plug-Flow detention time= 119.1 min calculated for 2.094 af (100% of inflow)

Avail Ctorogo Ctorogo Description

Center-of-Mass det. time= 119.2 min (952.5 - 833.3)

los cort

volume	invert	Avaii.5	torage	Storage	Description	
#1	363.00'	12	,557 cf	Custon	n Stage Data (Pr	ismatic)Listed below (Recalc)
Elevation (feet)		Area sq-ft)	Inc.S (cubic-	Store feet)	Cum.Store (cubic-feet)	
363.00 364.00 366.00 368.00	1	0 448 893 ,459		0 224 ,341 2,352	0 224 1,565 3,917	
370.00 372.00 372.09	2	2,095 2,739 2,850	3	3,554 1,834 252	7,471 12,305 12,557	

Device	Routing	Invert	Outlet Devices
#1	Primary	363.00'	10.000 in/hr Exfiltration over Surface area
#2	Device 3	372.00'	54.0' long x 1.5' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00
			Coef. (English) 2.62 2.64 2.64 2.68 2.75 2.86 2.92 3.07 3.07
			3.03 3.28 3.32
#3	Secondary	371.00'	Custom Weir/Orifice, Cv= 2.62 (C= 3.28)
	-		Head (feet) 0.00 1.00
			Width (feet) 2.00 2.00

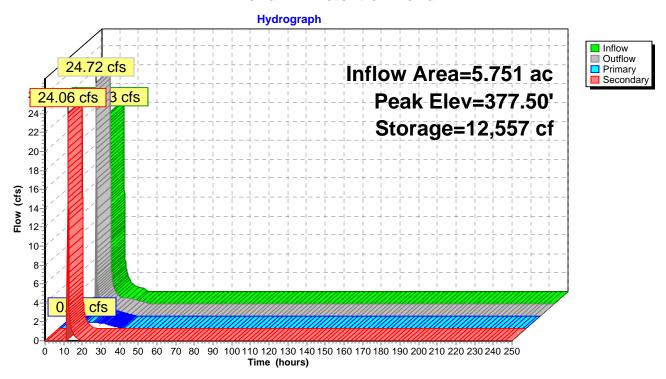
Primary OutFlow Max=0.66 cfs @ 12.08 hrs HW=373.88' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.66 cfs)

Secondary OutFlow Max=24.05 cfs @ 12.26 hrs HW=377.50' (Free Discharge)

-3=Custom Weir/Orifice (Orifice Controls 24.05 cfs @ 12.03 fps)

-2=Broad-Crested Rectangular Weir (Passes 24.05 cfs of 2,310.99 cfs potential flow)

Pond 4P: Detention Pond



Post-Development3

Type III 24-hr 100 Year Rainfall=7.50" Printed 11/1/2010

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Page 148

Summary for Pond 6P: Drywells

Inflow Area =	0.231 ac,100.00% Impervious, Inflow D	epth = 7.26" for 100 Year event
Inflow =	0.98 cfs @ 12.39 hrs, Volume=	0.140 af
Outflow =	0.22 cfs @ 13.17 hrs, Volume=	0.140 af, Atten= 78%, Lag= 46.6 min
Primary =	0.22 cfs @ 13.17 hrs, Volume=	0.140 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 388.40' @ 13.17 hrs Surf.Area= 1,218 sf Storage= 1,796 cf

Plug-Flow detention time= 55.7 min calculated for 0.140 af (100% of inflow) Center-of-Mass det. time= 55.7 min (819.9 - 764.2)

Volume	Invert	Avail.Storage	Storage Description
#1A	386.00'	1,291 cf	27.68'W x 44.00'L x 4.83'H Field A
			5,883 cf Overall - 2,655 cf Embedded = 3,227 cf x 40.0% Voids
#2A	387.00'	2,058 cf	Dry_Well 1000 Gallon x 16 Inside #1
			Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf
			Outside= 68.0"W x 34.0"H => 15.81 sf x 10.50'L = 166.0 cf
		3,349 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	386.00'	6.000 in/hr Exfiltration over Wetted area
#2	Secondary	393.00'	24.0" Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=0.22 cfs @ 13.17 hrs HW=388.40' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.22 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=386.00' (Free Discharge) 2=Orifice/Grate (Controls 0.00 cfs)

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Page 149

Pond 6P: Drywells - Chamber Wizard Field A

Chamber Model = Dry_Well 1000 Gallon

Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf Outside= 68.0"W x 34.0"H => 15.81 sf x 10.50'L = 166.0 cf

68.0" Wide + 12.0" Spacing = 80.0" C-C

4 Chambers/Row x 10.50' Long = 42.00' + 12.0" End Stone x 2 = 44.00' Base Length 4 Rows x 68.0" Wide + 12.0" Spacing x 3 + 12.0" Side Stone x 2 = 27.68' Base Width 12.0" Base + 34.0" Chamber Height + 12.0" Cover = 4.83' Field Height

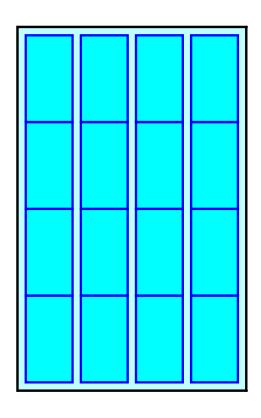
16 Chambers x 128.6 cf = 2,058.4 cf Chamber Storage

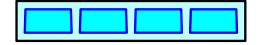
16 Chambers x 166.0 cf = 2,655.4 cf Displacement

5,882.6 cf Field - 2,655.4 cf Chambers = 3,227.2 cf Stone x 40.0% Voids = 1,290.9 cf Stone Storage

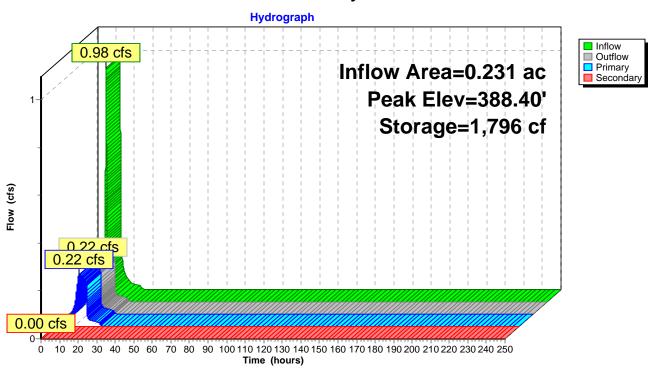
Stone + Chamber Storage = 3,349.3 cf = 0.077 af

16 Chambers 217.9 cy Field 119.5 cy Stone





Pond 6P: Drywells



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Page 151

Summary for Pond 7P: Stormwater Treatment Pond #2

Inflow Area = 2.937 ac, 37.76% Impervious, Inflow Depth = 4.37" for 100 Year event Inflow 8.24 cfs @ 12.45 hrs. Volume= 1.069 af 8.19 cfs @ 12.48 hrs, Volume= Outflow 1.069 af, Atten= 1%, Lag= 1.3 min 0.23 cfs @ 12.48 hrs, Volume= Primary 0.219 af Secondary = 7.96 cfs @ 12.48 hrs, Volume= 0.851 af 0.00 cfs @ 0.00 hrs, Volume= Tertiary 0.000 af

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 364.64' @ 12.48 hrs Surf.Area= 999 sf Storage= 1,826 cf

Plug-Flow detention time= 20.6 min calculated for 1.069 af (100% of inflow) Center-of-Mass det. time= 20.6 min (866.4 - 845.9)

<u>Volume</u>	Invert	Avail.Sto	rage Storage	Description	
#1	361.00'	2,60	05 cf Custom	Stage Data (Pr	rismatic)Listed below (Recalc)
Elevation (fee	_	urf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	
361.0	00	100	0	0	
362.0	00	300	200	200	
364.0	00	763	1,063	1,263	
365.0	00	1,132	948	2,211	
365.3	30	1,500	395	2,605	
Device	Routing	Invert	Outlet Devices	5	
#1	Primary	361.00'	10.000 in/hr E	xfiltration over	Surface area
#2	Cocondon	262 50'	Custom Wair	Orifica Cv- 2	62 (C_ 2 20)

#1	Primary	361.00'	10.000 in/hr Exfiltration over Surface area
#2	Secondary	363.50'	Custom Weir/Orifice, Cv= 2.62 (C= 3.28)
			Head (feet) 0.00 1.50
			Width (feet) 2.00 2.00
#3	Tertiary	365.10'	93.0' long x 3.0' breadth Broad-Crested Rectangular Weir
			Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00 3.50 4.00 4.50
			Coef. (English) 2.44 2.58 2.68 2.67 2.65 2.64 2.64 2.68 2.68
			2.72 2.81 2.92 2.97 3.07 3.32

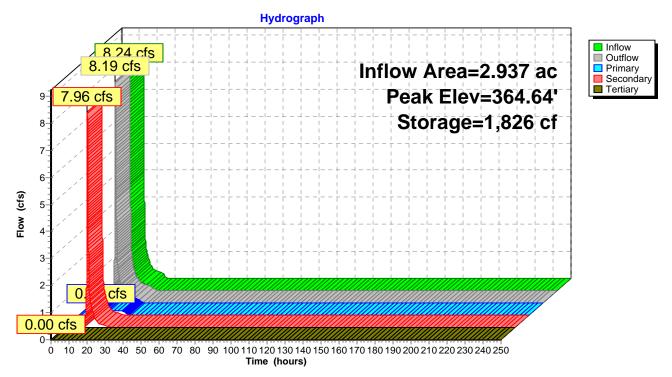
Primary OutFlow Max=0.23 cfs @ 12.48 hrs HW=364.64' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.23 cfs)

Secondary OutFlow Max=7.96 cfs @ 12.48 hrs HW=364.64' (Free Discharge) 2=Custom Weir/Orifice (Weir Controls 7.96 cfs @ 3.49 fps)

Tertiary OutFlow Max=0.00 cfs @ 0.00 hrs HW=361.00' (Free Discharge)

3=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 7P: Stormwater Treatment Pond #2



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Page 153

Summary for Pond 8P: Stormwater Treatment Pond #1

Inflow 7.96 cfs @ 12.48 hrs, Volume= 0.851 af 7.53 cfs @ 12.58 hrs. Volume= 0.851 af, Atten= 5%, Lag= 6.0 min Outflow = 0.48 cfs @ 12.58 hrs, Volume= Primary 0.183 af 7.05 cfs @ 12.58 hrs, Volume= 0.668 af Secondary =

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 364.77' @ 12.58 hrs Surf.Area= 2,074 sf Storage= 4,670 cf

Plug-Flow detention time= 13.7 min calculated for 0.851 af (100% of inflow) Center-of-Mass det. time= 13.7 min (829.6 - 815.9)

Volume	Invert	Avail.Sto	rage Storag	e Description		
#1	361.00'	5,86	67 cf Custo	m Stage Data (P	rismatic)Listed below (Recalc)	
Elevation (fee		rf.Area (sq-ft)	Inc.Store (cubic-feet)	Inc.Store Cum.Store		
361.0	•	300	0	0		
362.0		842	571	571		
364.0		1,772	2,614	3,185		
365.0		2,163	1,968	5,153		
365.3	30	2,600	714	5,867		
Dovice	Douting	Invert	Outlet Device	200		
Device	Routing	Invert				
#1	Secondary	361.00'	24.0" Roun		conforming to fill, Ke= 0.500	
					330.00' S= 0.1550 '/' Cc= 0.900	
#2	Device 1	361.10'	n= 0.013 Corrugated PE, smooth interior 2.0" Vert. Orifice/Grate (0 yr) C= 0.600			
#3	Device 1	361.70'				
#4	Device 1	362.30'	7.0" Vert. Orifice/Grate(2yr) C= 0.600			
#5	Device 1	363.40'	9.0" Vert. Orifice/Grate(10yr) C= 0.600			
#6	Device 1	364.70'	57.0" x 57.0" Horiz. Top of Riser (100yr) C= 0.600			
			Limited to weir flow at low heads			
#7	Device 1	365.00'			oad-Crested Rectangular Weir (14.5)	
			, ,		0.80 1.00 1.20 1.40 1.60 1.80 2.00	
			2.50 3.00 3			
					61 2.60 2.66 2.70 2.77 2.89 2.88	
" 0	D : 1 : 4	005 001	2.85 3.07 3		10 (15 () 17 (
#8	Device 1	365.20'			road-Crested Rectangular Weir	
Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60			0.80 1.00 1.20 1.40 1.60 1.80 2.00			
2.50 3.00 3.50 4.00 4.50 Coef. (English) 2.44 2.58 2.68 2.67 2.65		60 267 265 264 264 260 260				
				511) 2.44 2.56 2. 2.92 2.97 3.07 3		
#9	Primary	361.00'	_	r Exfiltration ove		
πΟ	i illiai y	301.00	10.000 111/111		i Juliace alea	

Primary OutFlow Max=0.48 cfs @ 12.58 hrs HW=364.77' (Free Discharge) 9=Exfiltration (Exfiltration Controls 0.48 cfs)

Secondary OutFlow Max=7.01 cfs @ 12.58 hrs HW=364.77' (Free Discharge)

-1=Culvert (Passes 7.01 cfs of 25.19 cfs potential flow)

-2=Orifice/Grate (0 yr) (Orifice Controls 0.20 cfs @ 9.12 fps)

-3=Orifice/Grate (1yr) (Orifice Controls 1.59 cfs @ 8.09 fps)

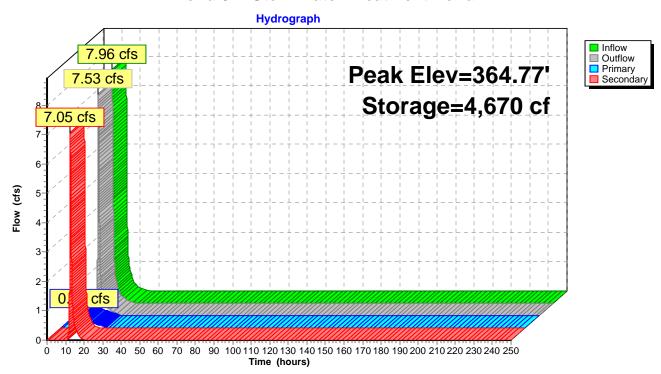
-4=Orifice/Grate(2yr) (Orifice Controls 1.90 cfs @ 7.11 fps)

-5=Orifice/Grate(10yr) (Orifice Controls 2.12 cfs @ 4.81 fps)

-6=Top of Riser (100yr) (Weir Controls 1.20 cfs @ 0.88 fps) -7=Broad-Crested Rectangular Weir (14.5)(Controls 0.00 cfs)

-8=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

Pond 8P: Stormwater Treatment Pond #1



Post-Development3

Type III 24-hr 100 Year Rainfall=7.50" Printed 11/1/2010

Prepared by Petruccelli Engineering

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Page 155

Summary for Pond 9P: Drywells

Inflow Area =	0.459 ac,100.00% Impervious, Inflow De	epth = 7.26" for 100 Year event
Inflow =	2.35 cfs @ 12.25 hrs, Volume=	0.278 af
Outflow =	0.25 cfs @ 11.31 hrs, Volume=	0.278 af, Atten= 89%, Lag= 0.0 min
Primary =	0.25 cfs @ 11.31 hrs, Volume=	0.278 af
Secondary =	0.00 cfs @ 0.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs Peak Elev= 389.44' @ 13.37 hrs Surf.Area= 1,834 sf Storage= 4,437 cf

Plug-Flow detention time= 125.5 min calculated for 0.278 af (100% of inflow) Center-of-Mass det. time= 125.5 min (879.7 - 754.2)

Volume	Invert	Avail.Storage	Storage Description
#1A	386.00'	1,685 cf	41.69'W x 44.00'L x 4.83'H Field A
			8,860 cf Overall - 4,647 cf Embedded = 4,213 cf \times 40.0% Voids
#2A	387.00'	3,602 cf	Dry_Well 1000 Gallon x 28 Inside #1
			Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf
			Outside= 68.0"W x 34.0"H => 15.81 sf x 10.50'L = 166.0 cf
		5,287 cf	Total Available Storage

Storage Group A created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	386.00'	6.000 in/hr Exfiltration over Surface area
#2	Secondary	392.00'	24.0" Vert. Orifice/Grate C= 0.600

Primary OutFlow Max=0.25 cfs @ 11.31 hrs HW=386.08' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.25 cfs)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=386.00' (Free Discharge) 2=Orifice/Grate (Controls 0.00 cfs)

Page 156

Pond 9P: Drywells - Chamber Wizard Field A

Chamber Model = Dry_Well 1000 Gallon

Inside= 62.0"W x 30.0"H => 12.86 sf x 10.00'L = 128.6 cf Outside= 68.0"W x 34.0"H => 15.81 sf x 10.50'L = 166.0 cf

68.0" Wide + 0.0" Spacing = 68.0" C-C

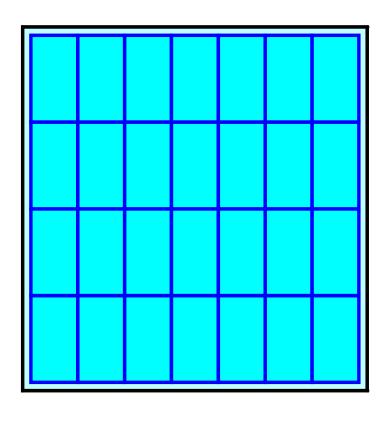
4 Chambers/Row x 10.50' Long = 42.00' + 12.0'' End Stone x 2 = 44.00' Base Length 7 Rows x 68.0'' Wide + 12.0'' Side Stone x 2 = 41.69' Base Width 12.0'' Base + 34.0'' Chamber Height + 12.0'' Cover = 4.83' Field Height

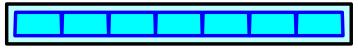
28 Chambers x 128.6 cf = 3,602.2 cf Chamber Storage 28 Chambers x 166.0 cf = 4,646.9 cf Displacement

8,860.0 cf Field - 4,646.9 cf Chambers = 4,213.1 cf Stone x 40.0% Voids = 1,685.2 cf Stone Storage

Stone + Chamber Storage = 5,287.4 cf = 0.121 af

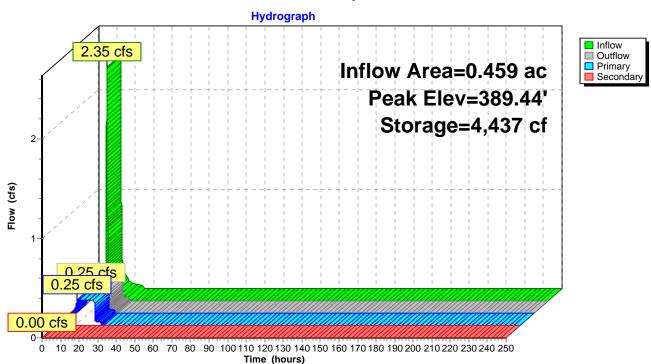
28 Chambers 328.1 cy Field 156.0 cy Stone





rage 13

Pond 9P: Drywells



Summary for Link DP-1: Design Point 1

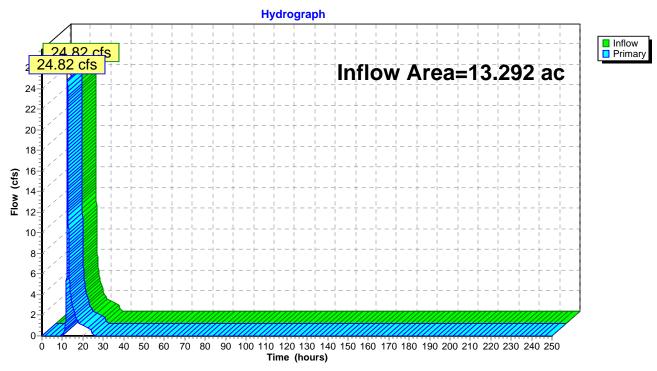
Inflow Area = 13.292 ac, 3.27% Impervious, Inflow Depth = 2.96" for 100 Year event

Inflow = 24.82 cfs @ 12.47 hrs, Volume= 3.282 af

Primary = 24.82 cfs @ 12.47 hrs, Volume= 3.282 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-1: Design Point 1



Page 159

Summary for Link DP-2: Design Point 2

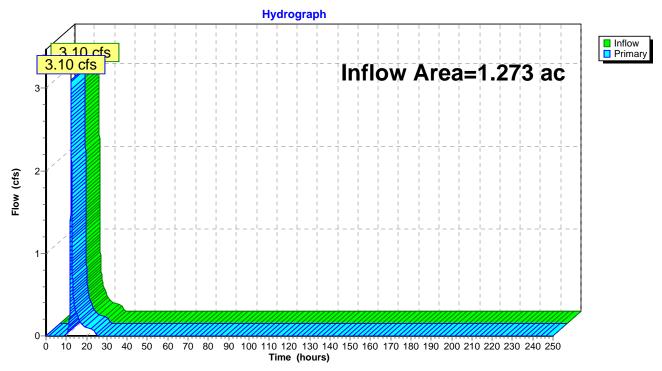
Inflow Area = 1.273 ac, 7.29% Impervious, Inflow Depth = 3.28" for 100 Year event

Inflow = 3.10 cfs @ 12.32 hrs, Volume= 0.348 af

Primary = 3.10 cfs @ 12.32 hrs, Volume= 0.348 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-2: Design Point 2



Summary for Link DP-3: Design Point 3

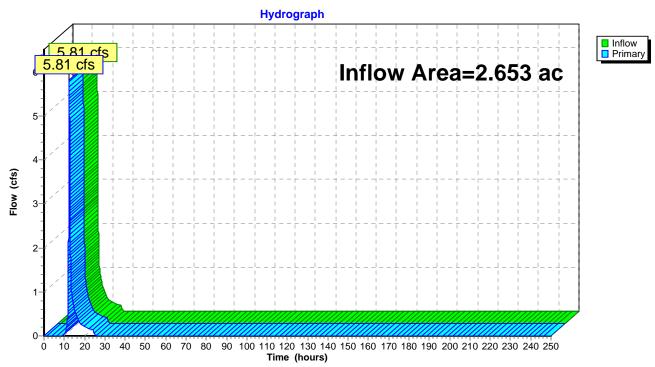
Inflow Area = 2.653 ac, 7.97% Impervious, Inflow Depth = 3.28" for 100 Year event

Inflow = 5.81 cfs @ 12.42 hrs, Volume= 0.725 af

Primary = 5.81 cfs @ 12.42 hrs, Volume= 0.725 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-3: Design Point 3



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Summary for Link DP-4: Design Point 4

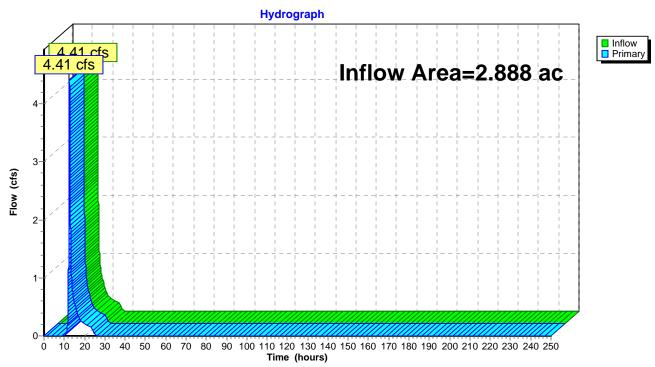
Inflow Area = 2.888 ac, 0.56% Impervious, Inflow Depth = 2.55" for 100 Year event

Inflow = 4.41 cfs @ 12.51 hrs, Volume= 0.614 af

Primary = 4.41 cfs @ 12.51 hrs, Volume= 0.614 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-4: Design Point 4



Summary for Link DP-5: Design Point 5

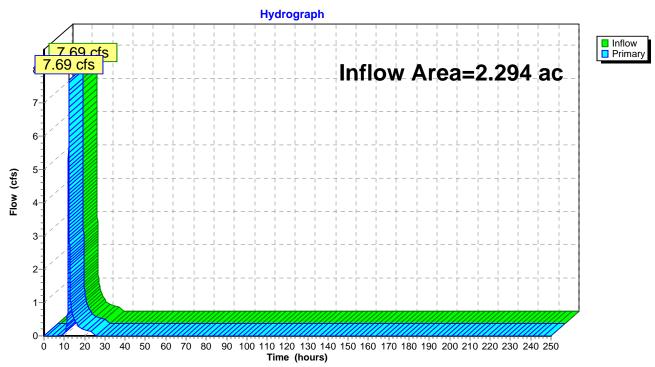
Inflow Area = 2.294 ac, 15.79% Impervious, Inflow Depth = 3.60" for 100 Year event

Inflow = 7.69 cfs @ 12.18 hrs, Volume= 0.688 af

Primary = 7.69 cfs @ 12.18 hrs, Volume= 0.688 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-5: Design Point 5



Page 163

Summary for Link DP-6: Design Point 6

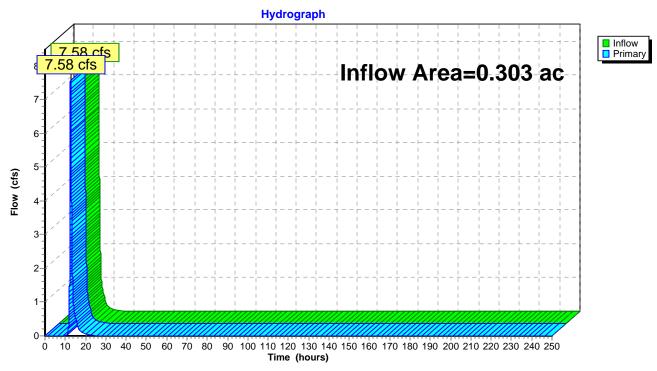
Inflow Area = 0.303 ac, 0.00% Impervious, Inflow Depth = 29.39" for 100 Year event

Inflow = 7.58 cfs @ 12.57 hrs, Volume= 0.743 af

Primary = 7.58 cfs @ 12.57 hrs, Volume= 0.743 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-6: Design Point 6



Page 164

Summary for Link DP-7: Design Point 7

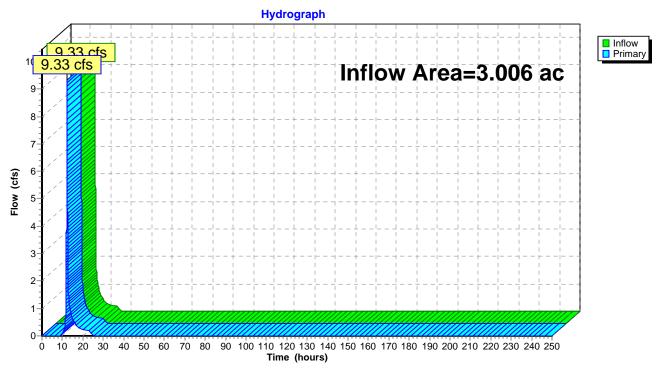
Inflow Area = 3.006 ac, 9.68% Impervious, Inflow Depth = 3.39" for 100 Year event

Inflow = 9.33 cfs @ 12.19 hrs, Volume= 0.848 af

Primary = 9.33 cfs @ 12.19 hrs, Volume= 0.848 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-250.00 hrs, dt= 0.01 hrs

Link DP-7: Design Point 7





<u>APPENDIX D</u>

- OPERATOR AND CONTRACTOR CERTIFICATIONS



TRIPI CONSERVATION SUBDIVISION HARRIS ROAD TOWN OF BEDFORD WESTCHESTER COUNTY, NEW YORK

OPERATOR CERTIFICATION STATEMENT

I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that false statements made herein are punishable as a class A misdemeanor pursuant to sections 210.45 of the Penal Law.

Operator	
g.	
Signature	
Print Name	
Title	
Date	
Address	
Telephone Number	



<u>APPENDIX E</u>

- MAINTENANCE AND INSPECTION REPORTS AND CONSTRUCTION CHECKLIST



Stormwater/Wetland Pond Construction Inspection Checklist

Due Comptunistica Martoniale and Familiano and		
CONSTRUCTION SEQUENCE	Satisfactory/ Unsatisfactory	COMMENTS
Inspector:		
Time:		
Date:		
Project: Location: Site Status:		
		•

Construction Sequence	Satisfactory/ Unsatisfactory	COMMENTS
Pre-Construction/Materials and Equipment		
Pre-construction meeting		
Pipe and appurtenances on-site prior to construction and dimensions checked		
Material (including protective coating, if specified)		
2. Diameter		
Dimensions of metal riser or pre-cast concrete outlet structure		
Required dimensions between water control structures (orifices, weirs, etc.) are in accordance with approved plans		
5. Barrel stub for prefabricated pipe structures at proper angle for design barrel slope		
Number and dimensions of prefabricated anti-seep collars		
7. Watertight connectors and gaskets		
8. Outlet drain valve		
Project benchmark near pond site		
Equipment for temporary de-watering		

CONSTRUCTION SEQUENCE	Satisfactory/ Unsatisfactory	COMMENTS			
2. Subgrade Preparation					
Area beneath embankment stripped of all vegetation, topsoil, and organic matter					
3. Pipe Spillway Installation					
Method of installation detailed on plans					
A. Bed preparation					
Installation trench excavated with specified side slopes					
Stable, uniform, dry subgrade of relatively impervious material (If subgrade is wet, contractor shall have defined steps before proceeding with installation)					
Invert at proper elevation and grade					
B. Pipe placement					
Metal / plastic pipe					
Watertight connectors and gaskets properly installed					
Anti-seep collars properly spaced and having watertight connections to pipe					
Backfill placed and tamped by hand under "haunches" of pipe					
4. Remaining backfill placed in max. 8 inch lifts using small power tamping equipment until 2 feet cover over pipe is reached					

CONSTRUCTION SEQUENCE	Satisfactory/ Unsatisfactory	Сомментѕ				
3. Pipe Spillway Installation						
Concrete pipe	T					
Pipe set on blocks or concrete slab for pouring of low cradle						
Pipe installed with rubber gasket joints with no spalling in gasket interface area						
Excavation for lower half of anti-seep collar(s) with reinforcing steel set						
Entire area where anti-seep collar(s) will come in contact with pipe coated with mastic or other approved waterproof sealant						
Low cradle and bottom half of anti-seep collar installed as monolithic pour and of an approved mix						
Upper half of anti-seep collar(s) formed with reinforcing steel set						
 Concrete for collar of an approved mix and vibrated into place (protected from freezing while curing, if necessary) 						
 Forms stripped and collar inspected for honeycomb prior to backfilling. Parge if necessary. 						
C. Backfilling						
Fill placed in maximum 8 inch lifts						
Backfill taken minimum 2 feet above top of anti- seep collar elevation before traversing with heavy equipment						

CONSTRUCTION SEQUENCE	Satisfactory/ Unsatisfactory	Сомментѕ
4. Riser / Outlet Structure Installation		
Riser located within embankment		
A. Metal riser		
Riser base excavated or formed on stable subgrade to design dimensions		
Set on blocks to design elevations and plumbed		
Reinforcing bars placed at right angles and projecting into sides of riser		
Concrete poured so as to fill inside of riser to invert of barrel		
B. Pre-cast concrete structure		
Dry and stable subgrade		
Riser base set to design elevation		
If more than one section, no spalling in gasket interface area; gasket or approved caulking material placed securely		
Watertight and structurally sound collar or gasket joint where structure connects to pipe spillway		
C. Poured concrete structure		
Footing excavated or formed on stable subgrade, to design dimensions with reinforcing steel set		
Structure formed to design dimensions, with reinforcing steel set as per plan		
Concrete of an approved mix and vibrated into place (protected from freezing while curing, if necessary)		
Forms stripped & inspected for "honeycomb" prior to backfilling; parge if necessary		

CONSTRUCTION SEQUENCE	Satisfactory/ Unsatisfactory	COMMENTS				
i. Embankment Construction						
Fill material						
Compaction						
Embankment						
Fill placed in specified lifts and compacted with appropriate equipment						
Constructed to design cross-section, side slopes and top width						
Constructed to design elevation plus allowance for settlement						
6. Impounded Area Construction						
Excavated / graded to design contours and side slopes						
Inlet pipes have adequate outfall protection						
Forebay(s)						
Pond benches						
7. Earth Emergency Spillway Construction						
Spillway located in cut or structurally stabilized with riprap, gabions, concrete, etc.						
Excavated to proper cross-section, side slopes and bottom width						
Entrance channel, crest, and exit channel constructed to design grades and elevations						

CONSTRUCTION SEQUENCE	Satisfactory / Unsatisfactory	COMMENTS				
8. Outlet Protection						
A. End section						
Securely in place and properly backfilled						
B. Endwall						
Footing excavated or formed on stable subgrade, to design dimensions and reinforcing steel set, if specified						
Endwall formed to design dimensions with reinforcing steel set as per plan						
Concrete of an approved mix and vibrated into place (protected from freezing, if necessary)						
Forms stripped and structure inspected for "honeycomb" prior to backfilling; parge if necessary						
C. Riprap apron / channel						
Apron / channel excavated to design cross- section with proper transition to existing ground						
Filter fabric in place						
Stone sized as per plan and uniformly place at the thickness specified						
9. Vegetative Stabilization						
Approved seed mixture or sod						
Proper surface preparation and required soil amendments						
Excelsior mat or other stabilization, as per plan						

CONSTRUCTION SEQUENCE	Satisfactory/ Unsatisfactory	COMMENTS
10. Miscellaneous		
Drain for ponds having a permanent pool		
Trash rack / anti-vortex device secured to outlet structure		
Trash protection for low flow pipes, orifices, etc.		
Fencing (when required)		
Access road		
Set aside for clean-out maintenance		
11. Stormwater Wetlands		
Adequate water balance		
Variety of depth zones present		
Approved pondscaping plan in place Reinforcement budget for additional plantings		
Plants and materials ordered 6 months prior to construction		
Construction planned to allow for adequate planting and establishment of plant community (April-June planting window)		
Wetland buffer area preserved to maximum extent possible		
Comments:		

Actions to be Taken:					

Infiltration Trench Construction Inspection Checklist

Project: Location:			
Site Status:			
Date:			
Time:			
Inspector:			

CONSTRUCTION SEQUENCE	Satisfactory/ Unsatisfactory	COMMENTS
1. Pre-Construction		
Pre-construction meeting		
Runoff diverted		
Soil permeability tested		
Groundwater / bedrock sufficient at depth		
2. Excavation		
Size and location		
Side slopes stable		
Excavation does not compact subsoils		
3. Filter Fabric Placement		
Fabric specifications		
Placed on bottom, sides, and top		

CONSTRUCTION SEQUENCE	SATISFACTORY / UNSATISFACTORY	COMMENTS			
4. Aggregate Material					
Size as specified					
Clean / washed material					
Placed properly					
5. Observation Well					
Pipe size					
Removable cap / footplate					
Initial depth =feet					
6. Final Inspection					
Pretreatment facility in place					
Contributing watershed stabilized prior to flow diversion					
Outlet					
Comments:					

Actions to be Taken:					

Infiltration Basin Construction Inspection Checklist

Project: Location: Site Status:			
Date:			
Time:			
Inspector:			

CONSTRUCTION SEQUENCE	SATISFACTORY/ UNSATISFACTORY	COMMENTS
1. Pre-Construction		
Runoff diverted		
Soil permeability tested		
Groundwater / bedrock depth		
2. Excavation		
Size and location		
Side slopes stable		
Excavation does not compact subsoils		
3. Embankment		
Barrel		
Anti-seep collar or Filter diaphragm		
Fill material		

CONSTRUCTION SEQUENCE	Satisfactory/ Unsatisfactory	COMMENTS
4. Final Excavation		
Drainage area stabilized		
Sediment removed from facility		
Basin floor tilled		
Facility stabilized		
5. Final Inspection		
Pretreatment facility in place		
Inlets / outlets		
Contributing watershed stabilized before flow is routed to the factility		
Comments:		
Actions to be Taken:		